

Appendix U:

Traffic Impact Analysis (Revised Appendix) part 2

Year 2025 With Project – Proposed Action “A”

Horseshoe Grande Property - Proposed Action "A"
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 Sanderson Avenue (NS) at Ramona Expressway (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.857

Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 51.5

Optimal Cycle: OPTIMIZED Level Of Service: D

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control (Protected), Rights (Ovl), Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing traffic volumes and adjustments. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module:

Table with 12 columns representing saturation flow and adjustments. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis metrics. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 Sanderson Avenue (NS) at Ramona Expressway (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.849
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 52.9
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Ovl			Ovl			Ovl		
Min. Green:	10	28	28	10	28	28	10	28	28	10	28	28
Lanes:	2	0	2	0	2	0	1	2	0	2	0	2
	0	1			1			1		0	1	

Volume Module:

Base Vol:	95	726	18	757	1359	853	457	685	119	9	478	491
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	95	726	18	757	1359	853	457	685	119	9	478	491
Added Vol:	0	0	0	47	0	0	0	47	0	0	42	42
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	95	726	18	804	1359	853	457	732	119	9	520	533
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	100	764	19	846	1431	898	481	771	125	9	547	561
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	100	764	19	846	1431	898	481	771	125	9	547	561
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	100	764	19	846	1431	898	481	771	125	9	547	561

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.95	0.85	0.92	0.95	0.85	0.92	0.95	0.85	0.92	0.95	0.85
Lanes:	2.00	2.00	1.00	2.00	2.00	1.00	2.00	2.00	1.00	2.00	2.00	1.00
Final Sat.:	3502	3610	1615	3502	3610	1615	3502	3610	1615	3502	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.03	0.21	0.01	0.24	0.40	0.56	0.14	0.21	0.08	0.00	0.15	0.35
Crit Moves:	****			****			****			****		
Green/Cycle:	0.08	0.24	0.34	0.25	0.41	0.55	0.14	0.27	0.36	0.10	0.23	0.49
Volume/Cap:	0.34	0.87	0.03	0.95	0.96	1.01	1.01	0.78	0.22	0.03	0.65	0.71
Delay/Veh:	52.6	52.5	26.4	64.2	48.4	59.9	95.8	44.6	27.2	49.1	43.4	27.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	52.6	52.5	26.4	64.2	48.4	59.9	95.8	44.6	27.2	49.1	43.4	27.4
LOS by Move:	D	D	C	E	D	E	F	D	C	D	D	C
HCM2kAvgQ:	2	17	0	21	32	41	14	16	3	0	10	17

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	0	0	0	1	0	1	0

Volume Module:

Base Vol:	1	693	15	148	252	1	1	1	1	15	1	366
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	1	693	15	148	252	1	1	1	1	15	1	366
Added Vol:	0	0	0	213	0	0	0	0	0	0	0	96
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	1	693	15	361	252	1	1	1	1	15	1	462
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	1	729	16	380	265	1	1	1	1	16	1	486
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	1	729	16	380	265	1	1	1	1	16	1	486

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	4.1	xxxx	xxxxxx	7.1	6.5	6.2	7.1	6.5	6.2
FollowUpTim:	2.2	xxxx	xxxxxx	2.2	xxxx	xxxxxx	3.5	4.0	3.3	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	266	xxxx	xxxxxx	745	xxxx	xxxxxx	2009	1773	266	1766	1766	737
Potent Cap.:	1309	xxxx	xxxxxx	872	xxxx	xxxxxx	45	84	778	66	85	422
Move Cap.:	1309	xxxx	xxxxxx	872	xxxx	xxxxxx	0	47	778	43	48	422
Volume/Cap:	0.00	xxxx	xxxx	0.44	xxxx	xxxx	xxxx	0.02	0.00	0.37	0.02	1.15

Level Of Service Module:

2Way95thQ:	0.0	xxxx	xxxxxx	2.2	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	18.1
Control Del:	7.8	xxxx	xxxxxx	12.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	123.2
LOS by Move:	A	*	*	B	*	*	*	*	*	*	*	F
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	0	xxxxxx	43	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	1.4	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	134.5	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	F	*	*
ApproachDel:	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	123.5	xxxxxx	
ApproachLOS:	*	*	*	*	*	*	F	F	F	F	F	

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

Average Delay (sec/veh): 40.2 Worst Case Level Of Service: F[4125.1]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume metrics. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with 12 columns for gap metrics. Rows include Critical Gp and FollowUpTim.

Capacity Module: Table with 12 columns for capacity metrics. Rows include Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with 12 columns for LOS metrics. Rows include 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

 Cycle (sec): 105 Critical Vol./Cap.(X): 0.694
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 33.3
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Ovl		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	1	0	0	1	0	0	1	0	0	0	1	0

Volume Module:

Base Vol:	1	693	15	148	252	1	1	1	1	15	1	366
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	1	693	15	148	252	1	1	1	1	15	1	366
Added Vol:	0	0	0	213	0	0	0	0	0	0	0	96
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	1	693	15	361	252	1	1	1	1	15	1	462
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	1	729	16	380	265	1	1	1	1	16	1	486
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	1	729	16	380	265	1	1	1	1	16	1	486
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	1	729	16	380	265	1	1	1	1	16	1	486

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	0.95	1.00	1.00	0.92	0.92	0.92	0.83	0.83	0.85
Lanes:	1.00	0.98	0.02	1.00	0.99	0.01	0.34	0.33	0.33	0.94	0.06	1.00
Final Sat.:	1805	1854	40	1805	1891	8	581	581	581	1477	98	1615

Capacity Analysis Module:

Vol/Sat:	0.00	0.39	0.39	0.21	0.14	0.14	0.00	0.00	0.00	0.01	0.01	0.30
Crit Moves:	****			****						****		
Green/Cycle:	0.26	0.47	0.47	0.25	0.46	0.46	0.17	0.17	0.17	0.17	0.17	0.42
Volume/Cap:	0.00	0.85	0.85	0.85	0.31	0.31	0.01	0.01	0.01	0.06	0.06	0.72
Delay/Veh:	29.1	32.3	32.3	51.3	18.1	18.1	36.1	36.1	36.1	36.5	36.5	28.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	29.1	32.3	32.3	51.3	18.1	18.1	36.1	36.1	36.1	36.5	36.5	28.9
LOS by Move:	C	C	C	D	B	B	D	D	D	D	D	C
HCM2kAvgQ:	0	24	24	14	5	5	0	0	0	0	0	14

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 0.745
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 30.7
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Ovl		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	1	0	0	1	0	0	1	0	0	0	1	0

Volume Module:

Base Vol:	1	466	14	380	663	1	1	1	2	21	2	241
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	1	466	14	380	663	1	1	1	2	21	2	241
Added Vol:	0	0	0	285	0	0	0	0	0	0	0	251
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	1	466	14	665	663	1	1	1	2	21	2	492
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	1	491	15	700	698	1	1	1	2	22	2	518
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	1	491	15	700	698	1	1	1	2	22	2	518
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	1	491	15	700	698	1	1	1	2	22	2	518

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	0.95	1.00	1.00	0.90	0.90	0.90	0.80	0.80	0.85
Lanes:	1.00	0.97	0.03	1.00	0.99	0.01	0.25	0.25	0.50	0.91	0.09	1.00
Final Sat.:	1805	1837	55	1805	1897	3	429	429	858	1393	133	1615

Capacity Analysis Module:

Vol/Sat:	0.00	0.27	0.27	0.39	0.37	0.37	0.00	0.00	0.00	0.02	0.02	0.32
Crit Moves:	****			****						****		
Green/Cycle:	0.14	0.31	0.31	0.44	0.61	0.61	0.15	0.15	0.15	0.15	0.15	0.59
Volume/Cap:	0.00	0.87	0.87	0.87	0.60	0.60	0.02	0.02	0.02	0.11	0.11	0.54
Delay/Veh:	44.6	53.2	53.2	40.7	15.2	15.2	43.5	43.5	43.5	44.3	44.3	15.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	44.6	53.2	53.2	40.7	15.2	15.2	43.5	43.5	43.5	44.3	44.3	15.2
LOS by Move:	D	D	D	D	B	B	D	D	D	D	D	B
HCM2kAvgQ:	0	21	21	26	16	16	0	0	0	1	1	11

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 State Street (NS) at Ramona Expressway (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 0.894
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 56.0
 Optimal Cycle: OPTIMIZED Level Of Service: E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	28	28	10	28	28	10	24	24	10	24	24
Lanes:	1	0	1	1	0	1	1	0	1	1	0	1

Volume Module:

Base Vol:	379	683	101	109	453	56	159	279	422	252	502	307
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	379	683	101	109	453	56	159	279	422	252	502	307
Added Vol:	0	0	0	0	0	0	0	71	0	0	32	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	379	683	101	109	453	56	159	350	422	252	534	307
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	399	719	106	115	477	59	167	368	444	265	562	323
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	399	719	106	115	477	59	167	368	444	265	562	323
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	399	719	106	115	477	59	167	368	444	265	562	323

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.93	0.93	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85
Lanes:	1.00	1.74	0.26	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	1805	3085	456	1805	3610	1615	1805	3610	1615	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.22	0.23	0.23	0.06	0.13	0.04	0.09	0.10	0.28	0.15	0.16	0.20
Crit Moves:	****			****			****			****		
Green/Cycle:	0.22	0.33	0.33	0.12	0.23	0.23	0.13	0.27	0.27	0.14	0.28	0.28
Volume/Cap:	1.02	0.70	0.70	0.54	0.57	0.16	0.70	0.38	1.02	1.02	0.55	0.70
Delay/Veh:	96.3	36.8	36.8	52.4	41.5	36.8	59.1	35.8	90.7	111.0	37.1	43.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	96.3	36.8	36.8	52.4	41.5	36.8	59.1	35.8	90.7	111.0	37.1	43.4
LOS by Move:	F	D	D	D	D	D	E	D	F	F	D	D
HCM2kAvgQ:	21	15	15	5	9	2	7	6	23	15	10	12

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 State Street (NS) at Ramona Expressway (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 1.122
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 80.5
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
	L	T	R	L	T	R	L	T	R	L	T	R
Movement:												
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	28	28	10	28	28	10	24	24	10	24	24
Lanes:	1	0	1	1	0	2	1	0	2	1	0	2

Volume Module:

Base Vol:	422	750	112	288	744	71	125	699	574	232	388	154
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	422	750	112	288	744	71	125	699	574	232	388	154
Added Vol:	0	0	0	0	0	0	0	95	0	0	84	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	422	750	112	288	744	71	125	794	574	232	472	154
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	444	789	118	303	783	75	132	836	604	244	497	162
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	444	789	118	303	783	75	132	836	604	244	497	162
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	444	789	118	303	783	75	132	836	604	244	497	162

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.93	0.93	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85
Lanes:	1.00	1.74	0.26	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	1805	3081	460	1805	3610	1615	1805	3610	1615	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.25	0.26	0.26	0.17	0.22	0.05	0.07	0.23	0.37	0.14	0.14	0.10
Crit Moves:	****			****			****			****		
Green/Cycle:	0.21	0.27	0.27	0.17	0.23	0.23	0.13	0.31	0.31	0.11	0.30	0.30
Volume/Cap:	1.19	0.96	0.96	0.96	0.93	0.20	0.58	0.74	1.19	1.19	0.46	0.33
Delay/Veh:	158.0	64.7	64.7	90.4	61.5	37.2	53.2	39.4	146.2	177.9	34.3	33.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	158.0	64.7	64.7	90.4	61.5	37.2	53.2	39.4	146.2	177.9	34.3	33.0
LOS by Move:	F	E	E	F	E	D	D	D	F	F	C	C
HCM2kAvgQ:	28	22	22	16	19	2	5	16	37	17	8	5

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 State Street (NS) at Ramona Expressway (EW)

Cycle (sec): 115 Critical Vol./Cap.(X): 0.699
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 42.6
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Ovl			Include		
Min. Green:	10	28	28	10	28	28	10	24	24	10	24	24
Lanes:	1	0	2	0	1	1	1	0	2	0	1	1

Volume Module:

Base Vol:	379	683	101	109	453	56	159	279	422	252	502	307
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	379	683	101	109	453	56	159	279	422	252	502	307
Added Vol:	0	0	0	0	0	0	0	71	0	0	32	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	379	683	101	109	453	56	159	350	422	252	534	307
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	399	719	106	115	477	59	167	368	444	265	562	323
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	399	719	106	115	477	59	167	368	444	265	562	323
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	399	719	106	115	477	59	167	368	444	265	562	323

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	1805	3610	1615	1805	3610	1615	1805	3610	1615	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.22	0.20	0.07	0.06	0.13	0.04	0.09	0.10	0.28	0.15	0.16	0.20
Crit Moves:	****			****			****			****		
Green/Cycle:	0.25	0.36	0.36	0.13	0.24	0.24	0.11	0.21	0.45	0.16	0.26	0.26
Volume/Cap:	0.90	0.55	0.18	0.49	0.54	0.15	0.81	0.49	0.61	0.90	0.60	0.78
Delay/Veh:	63.0	29.9	25.3	48.3	38.6	34.3	70.6	40.6	25.1	75.6	38.7	48.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	63.0	29.9	25.3	48.3	38.6	34.3	70.6	40.6	25.1	75.6	38.7	48.6
LOS by Move:	E	C	C	D	D	C	E	D	C	E	D	D
HCM2kAvgQ:	17	11	3	4	8	2	8	6	12	13	10	12

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
Year 2025 With Project
Evening Peak Hour - With Improvements

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 State Street (NS) at Ramona Expressway (EW)

Cycle (sec): 115 Critical Vol./Cap.(X): 0.964
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 54.4
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns representing different traffic flows. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with 12 columns. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with 12 columns. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.860
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 32.3
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control (Permitted/Protected), Rights (Include), Min. Green, and Lanes.

Volume Module: Table with 12 columns for different traffic movements. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with 12 columns for different traffic movements. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with 12 columns for different traffic movements. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 1.277
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 93.0
Optimal Cycle: OPTIMIZED Level Of Service: F

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns representing different traffic movements and 11 rows of volume-related metrics like Base Vol, Growth Adj, etc.

Saturation Flow Module: Table with 12 columns and 4 rows showing saturation flow rates and adjustments.

Capacity Analysis Module: Table with 12 columns and 11 rows showing capacity analysis metrics like Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.823
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 29.5
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	28	28	28	28	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	1	0	2	1	0	2

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	103	698	118	130	537	239	220	627	120	73	729	97
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	103	698	118	130	537	239	220	627	120	73	729	97
Added Vol:	0	0	36	0	0	0	0	169	0	16	71	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	103	698	154	130	537	239	220	796	120	89	800	97
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	108	735	162	137	565	252	232	838	126	94	842	102
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	108	735	162	137	565	252	232	838	126	94	842	102
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	108	735	162	137	565	252	232	838	126	94	842	102

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.17	0.92	0.92	0.21	1.00	0.85	0.95	0.89	0.89	0.95	0.95	0.85
Lanes:	1.00	1.64	0.36	1.00	1.00	1.00	1.00	2.61	0.39	1.00	2.00	1.00
Final Sat.:	323	2878	635	390	1900	1615	1805	4417	666	1805	3610	1615

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.34	0.26	0.26	0.35	0.30	0.16	0.13	0.19	0.19	0.05	0.23	0.06
Crit Moves:				****				****				
Green/Cycle:	0.41	0.41	0.41	0.41	0.41	0.41	0.15	0.34	0.34	0.12	0.31	0.31
Volume/Cap:	0.82	0.63	0.63	0.86	0.73	0.38	0.86	0.56	0.56	0.43	0.75	0.20
Delay/Veh:	56.6	22.1	22.1	59.9	26.1	19.1	61.3	24.7	24.7	38.0	30.7	23.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	56.6	22.1	22.1	59.9	26.1	19.1	61.3	24.7	24.7	38.0	30.7	23.0
LOS by Move:	E	C	C	E	C	B	E	C	C	D	C	C
HCM2kAvgQ:	5	11	11	6	14	5	9	8	8	3	13	2

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.997
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 49.6
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	28	28	28	28	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	1	0	2	1	0	2

Volume Module:

Base Vol:	138	581	125	184	712	392	195	1075	172	144	1039	126
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	138	581	125	184	712	392	195	1075	172	144	1039	126
Added Vol:	0	0	47	0	0	0	0	225	0	42	198	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	138	581	172	184	712	392	195	1300	172	186	1237	126
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	145	612	181	194	749	413	205	1368	181	196	1302	133
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	145	612	181	194	749	413	205	1368	181	196	1302	133
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	145	612	181	194	749	413	205	1368	181	196	1302	133

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.18	0.92	0.92	0.31	1.00	0.85	0.95	0.89	0.89	0.95	0.95	0.85
Lanes:	1.00	1.54	0.46	1.00	1.00	1.00	1.00	2.65	0.35	1.00	2.00	1.00
Final Sat.:	343	2691	797	584	1900	1615	1805	4498	595	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.42	0.23	0.23	0.33	0.39	0.26	0.11	0.30	0.30	0.11	0.36	0.08
Crit Moves:	****						****			****		
Green/Cycle:	0.42	0.42	0.42	0.42	0.42	0.42	0.11	0.35	0.35	0.13	0.36	0.36
Volume/Cap:	1.00	0.54	0.54	0.78	0.93	0.60	1.00	0.87	0.87	0.87	1.00	0.23
Delay/Veh:	108.2	26.1	26.1	44.5	49.9	28.2	115.1	41.2	41.2	79.5	62.5	26.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	108.2	26.1	26.1	44.5	49.9	28.2	115.1	41.2	41.2	79.5	62.5	26.8
LOS by Move:	F	C	C	D	D	C	F	D	D	E	E	C
HCM2kAvgQ:	9	11	11	8	31	12	12	22	22	10	32	3

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 1.415
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 163.2
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	18	18	18	18	21	21	21	21	21	21
Lanes:	1	0	1	0	0	1	0	1	0	0	1	0

Volume Module:

Base Vol:	337	375	84	30	541	29	18	231	348	49	168	22
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	337	375	84	30	541	29	18	231	348	49	168	22
Added Vol:	0	0	275	71	0	0	0	0	0	119	0	32
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	337	375	359	101	541	29	18	231	348	168	168	54
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	355	395	378	106	569	31	19	243	366	177	177	57
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	355	395	378	106	569	31	19	243	366	177	177	57
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	355	395	378	106	569	31	19	243	366	177	177	57

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.93	0.93	0.93	0.95	0.99	0.99	0.96	0.96	0.85	0.51	0.51	0.51
Lanes:	1.19	0.41	0.40	1.00	0.95	0.05	0.07	0.93	1.00	0.43	0.43	0.14
Final Sat.:	2108	738	707	1805	1789	96	132	1692	1615	421	421	135

Capacity Analysis Module:

Vol/Sat:	0.17	0.53	0.53	0.06	0.32	0.32	0.14	0.14	0.23	0.42	0.42	0.42
Crit Moves:	****			****						****		
Green/Cycle:	0.38	0.38	0.38	0.22	0.22	0.22	0.30	0.30	0.30	0.30	0.30	0.30
Volume/Cap:	0.45	1.42	1.42	0.26	1.42	1.42	0.48	0.48	0.76	1.42	1.42	1.42
Delay/Veh:	28.0	231	231.5	38.6	247	246.8	35.3	35.3	45.5	248.1	248	248.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	28.0	231	231.5	38.6	247	246.8	35.3	35.3	45.5	248.1	248	248.1
LOS by Move:	C	F	F	D	F	F	D	D	D	F	F	F
HCM2kAvgQ:	8	70	70	3	45	45	8	8	14	32	32	32

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 2.482
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 453.3
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	18	18	18	18	21	21	21	21	21	21
Lanes:	1	0	1	0	0	1	0	1	0	0	1	0

Volume Module:

Base Vol:	452	436	171	81	522	49	50	356	452	179	304	54
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	452	436	171	81	522	49	50	356	452	179	304	54
Added Vol:	0	0	368	95	0	0	0	0	0	323	0	84
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	452	436	539	176	522	49	50	356	452	502	304	138
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	476	459	567	185	549	52	53	375	476	528	320	145
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	476	459	567	185	549	52	53	375	476	528	320	145
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	476	459	567	185	549	52	53	375	476	528	320	145

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.93	0.93	0.93	0.95	0.99	0.99	0.86	0.86	0.85	0.44	0.44	0.44
Lanes:	1.19	0.36	0.45	1.00	0.91	0.09	0.12	0.88	1.00	0.53	0.32	0.15
Final Sat.:	2095	640	791	1805	1714	161	201	1433	1615	442	267	121

Capacity Analysis Module:

Vol/Sat:	0.23	0.72	0.72	0.10	0.32	0.32	0.26	0.26	0.29	1.20	1.20	1.20
Crit Moves:	****			****						****		
Green/Cycle:	0.28	0.28	0.28	0.15	0.15	0.15	0.47	0.47	0.47	0.47	0.47	0.47
Volume/Cap:	0.81	2.55	2.55	0.68	2.14	2.14	0.56	0.56	0.63	2.55	2.55	2.55
Delay/Veh:	42.9	746	746.3	55.4	574	574.3	23.8	23.8	25.7	737.6	738	737.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	42.9	746	746.3	55.4	574	574.3	23.8	23.8	25.7	737.6	738	737.6
LOS by Move:	D	F	F	E	F	F	C	C	C	F	F	F
HCM2kAvgQ:	16	140	140	8	62	62	11	11	14	110	110	110

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

 Cycle (sec): 90 Critical Vol./Cap.(X): 0.674
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 32.1
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ovl			Include			Ovl			Include		
Min. Green:	18	18	18	18	18	18	10	21	21	10	21	21
Lanes:	1	0	1	0	1	0	1	0	1	0	1	0

Volume Module:

Base Vol:	337	375	84	30	541	29	18	231	348	49	168	22
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	337	375	84	30	541	29	18	231	348	49	168	22
Added Vol:	0	0	275	71	0	0	0	0	0	119	0	32
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	337	375	359	101	541	29	18	231	348	168	168	54
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	355	395	378	106	569	31	19	243	366	177	177	57
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	355	395	378	106	569	31	19	243	366	177	177	57
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	355	395	378	106	569	31	19	243	366	177	177	57

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	0.85	0.95	0.94	0.94	0.95	1.00	0.85	0.92	0.96	0.96
Lanes:	1.00	1.00	1.00	1.00	1.90	0.10	1.00	1.00	1.00	2.00	0.76	0.24
Final Sat.:	1805	1900	1615	1805	3399	182	1805	1900	1615	3502	1386	446

Capacity Analysis Module:

Vol/Sat:	0.20	0.21	0.23	0.06	0.17	0.17	0.01	0.13	0.23	0.05	0.13	0.13
Crit Moves:	****			****			****			****		
Green/Cycle:	0.26	0.26	0.38	0.21	0.21	0.21	0.11	0.23	0.50	0.11	0.23	0.23
Volume/Cap:	0.74	0.79	0.62	0.28	0.79	0.79	0.09	0.55	0.46	0.45	0.55	0.55
Delay/Veh:	36.5	38.7	24.9	30.0	38.8	38.8	36.1	31.8	15.1	38.3	31.8	31.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	36.5	38.7	24.9	30.0	38.8	38.8	36.1	31.8	15.1	38.3	31.8	31.8
LOS by Move:	D	D	C	C	D	D	D	C	B	D	C	C
HCM2kAvgQ:	11	12	9	3	10	10	1	7	7	3	6	6

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

 Cycle (sec): 105 Critical Vol./Cap.(X): 0.848
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 49.6
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ovl			Include			Ovl			Include		
Min. Green:	18	18	18	18	18	18	10	21	21	10	21	21
Lanes:	1	0	1	0	1	0	1	0	1	0	1	0

Volume Module:

Base Vol:	452	436	171	81	522	49	50	356	452	179	304	54
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	452	436	171	81	522	49	50	356	452	179	304	54
Added Vol:	0	0	368	95	0	0	0	0	0	323	0	84
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	452	436	539	176	522	49	50	356	452	502	304	138
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	476	459	567	185	549	52	53	375	476	528	320	145
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	476	459	567	185	549	52	53	375	476	528	320	145
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	476	459	567	185	549	52	53	375	476	528	320	145

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	0.85	0.95	0.94	0.94	0.95	1.00	0.85	0.92	0.95	0.95
Lanes:	1.00	1.00	1.00	1.00	1.83	0.17	1.00	1.00	1.00	2.00	0.69	0.31
Final Sat.:	1805	1900	1615	1805	3257	306	1805	1900	1615	3502	1245	565

Capacity Analysis Module:

Vol/Sat:	0.26	0.24	0.35	0.10	0.17	0.17	0.03	0.20	0.29	0.15	0.26	0.26
Crit Moves:	****			****			****			****		
Green/Cycle:	0.29	0.29	0.45	0.18	0.18	0.18	0.10	0.21	0.50	0.16	0.28	0.28
Volume/Cap:	0.92	0.84	0.78	0.56	0.92	0.92	0.31	0.92	0.59	0.93	0.92	0.92
Delay/Veh:	57.2	46.2	30.1	41.0	59.7	59.7	45.3	66.4	19.6	66.2	58.0	58.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	57.2	46.2	30.1	41.0	59.7	59.7	45.3	66.4	19.6	66.2	58.0	58.0
LOS by Move:	E	D	C	D	E	E	D	E	B	E	E	E
HCM2kAvgQ:	19	16	17	6	14	14	2	16	11	13	19	19

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #6 San Jacinto Street (NS) at Esplanade Avenue (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.656
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 31.7
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different traffic movements. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module:

Table with 12 columns representing different traffic movements. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns representing different traffic movements. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #6 San Jacinto Street (NS) at Esplanade Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.904
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 53.6
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	28	28	10	28	28	10	28	28	10	28	28
Lanes:	1	0	2	0	1	1	0	1	1	0	1	1

Volume Module:

Base Vol:	229	686	158	232	632	134	181	647	225	181	535	106
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	229	686	158	232	632	134	181	647	225	181	535	106
Added Vol:	0	320	0	0	281	42	47	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	229	1006	158	232	913	176	228	647	225	181	535	106
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	241	1059	166	244	961	185	240	681	237	191	563	112
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	241	1059	166	244	961	185	240	681	237	191	563	112
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	241	1059	166	244	961	185	240	681	237	191	563	112

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.93	0.93	0.95	0.95	0.85	0.95	0.93	0.93
Lanes:	1.00	2.00	1.00	1.00	1.68	0.32	1.00	2.00	1.00	1.00	1.67	0.33
Final Sat.:	1805	3610	1615	1805	2954	569	1805	3610	1615	1805	2938	582

Capacity Analysis Module:

Vol/Sat:	0.13	0.29	0.10	0.14	0.33	0.33	0.13	0.19	0.15	0.11	0.19	0.19
Crit Moves:	****			****			****			****		
Green/Cycle:	0.14	0.34	0.34	0.15	0.35	0.35	0.14	0.26	0.26	0.12	0.23	0.23
Volume/Cap:	0.93	0.87	0.31	0.87	0.93	0.93	0.93	0.73	0.57	0.90	0.82	0.82
Delay/Veh:	89.5	44.6	29.8	74.2	50.8	50.8	89.6	43.6	40.5	88.6	50.3	50.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	89.5	44.6	29.8	74.2	50.8	50.8	89.6	43.6	40.5	88.6	50.3	50.3
LOS by Move:	F	D	C	E	D	D	F	D	D	F	D	D
HCM2kAvgQ:	12	22	5	12	26	26	12	13	8	10	15	15

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #7 San Jacinto Street (NS) at Menlo Avenue (EW)

Cycle (sec): 70 Critical Vol./Cap.(X): 0.654
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 23.9
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	18	10	18	18	28	28	28	28	28	28
Lanes:	1	0	1	1	0	1	0	0	1	0	1	0

Volume Module:

Base Vol:	87	463	58	41	533	101	82	363	110	68	473	84
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	87	463	58	41	533	101	82	363	110	68	473	84
Added Vol:	0	240	0	0	103	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	87	703	58	41	636	101	82	363	110	68	473	84
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	92	740	61	43	669	106	86	382	116	72	498	88
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	92	740	61	43	669	106	86	382	116	72	498	88
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	92	740	61	43	669	106	86	382	116	72	498	88

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.94	0.94	0.95	0.93	0.93	0.27	0.97	0.97	0.27	1.00	0.85
Lanes:	1.00	1.85	0.15	1.00	1.73	0.27	1.00	0.77	0.23	1.00	1.00	1.00
Final Sat.:	1805	3298	272	1805	3050	484	517	1407	426	517	1900	1615

Capacity Analysis Module:

Vol/Sat:	0.05	0.22	0.22	0.02	0.22	0.22	0.17	0.27	0.27	0.14	0.26	0.05
Crit Moves:	****			****			****					
Green/Cycle:	0.14	0.28	0.28	0.15	0.29	0.29	0.40	0.40	0.40	0.40	0.40	0.40
Volume/Cap:	0.36	0.81	0.81	0.16	0.77	0.77	0.42	0.68	0.68	0.35	0.66	0.14
Delay/Veh:	27.9	29.0	29.0	26.0	26.5	26.5	16.5	19.9	19.9	15.6	19.2	13.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	27.9	29.0	29.0	26.0	26.5	26.5	16.5	19.9	19.9	15.6	19.2	13.4
LOS by Move:	C	C	C	C	C	C	B	B	B	B	B	B
HCM2kAvgQ:	2	11	11	1	10	10	2	10	10	1	10	1

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #7 San Jacinto Street (NS) at Menlo Avenue (EW)

 Cycle (sec): 105 Critical Vol./Cap.(X): 0.864
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 37.8
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	18	10	18	18	28	28	28	28	28	28
Lanes:	1	0	1	1	0	1	0	0	1	0	1	0

Volume Module:

Base Vol:	128	776	111	74	652	123	164	517	123	86	374	86
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	128	776	111	74	652	123	164	517	123	86	374	86
Added Vol:	0	320	0	0	281	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	128	1096	111	74	933	123	164	517	123	86	374	86
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	135	1154	117	78	982	129	173	544	129	91	394	91
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	135	1154	117	78	982	129	173	544	129	91	394	91
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	135	1154	117	78	982	129	173	544	129	91	394	91

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.94	0.94	0.95	0.93	0.93	0.40	0.97	0.97	0.15	1.00	0.85
Lanes:	1.00	1.82	0.18	1.00	1.77	0.23	1.00	0.81	0.19	1.00	1.00	1.00
Final Sat.:	1805	3232	327	1805	3135	413	758	1490	355	293	1900	1615

Capacity Analysis Module:

Vol/Sat:	0.07	0.36	0.36	0.04	0.31	0.31	0.23	0.37	0.37	0.31	0.21	0.06
Crit Moves:	****			****			****					
Green/Cycle:	0.11	0.39	0.39	0.10	0.37	0.37	0.40	0.40	0.40	0.40	0.40	0.40
Volume/Cap:	0.66	0.91	0.91	0.45	0.84	0.84	0.57	0.91	0.91	0.77	0.52	0.14
Delay/Veh:	52.3	39.8	39.8	46.8	35.1	35.1	27.1	45.6	45.6	54.2	24.5	20.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	52.3	39.8	39.8	46.8	35.1	35.1	27.1	45.6	45.6	54.2	24.5	20.1
LOS by Move:	D	D	D	D	D	D	C	D	D	D	C	C
HCM2kAvgQ:	5	24	24	3	20	20	5	25	25	4	10	2

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #8 San Jacinto Street (NS) at Devonshire Avenue (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.727
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 26.0
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	18	10	18	18	28	28	28	28	28	28
Lanes:	1	0	1	1	0	1	1	0	1	1	0	1

Volume Module:

Base Vol:	134	471	90	81	554	145	100	137	97	135	415	97
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	134	471	90	81	554	145	100	137	97	135	415	97
Added Vol:	0	240	0	0	103	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	134	711	90	81	657	145	100	137	97	135	415	97
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	141	748	95	85	692	153	105	144	102	142	437	102
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	141	748	95	85	692	153	105	144	102	142	437	102
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	141	748	95	85	692	153	105	144	102	142	437	102

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.93	0.93	0.95	0.92	0.92	0.21	1.00	0.85	0.66	0.97	0.97
Lanes:	1.00	1.78	0.22	1.00	1.64	0.36	1.00	1.00	1.00	1.00	0.81	0.19
Final Sat.:	1805	3150	399	1805	2877	635	403	1900	1615	1254	1497	350

Capacity Analysis Module:

Vol/Sat:	0.08	0.24	0.24	0.05	0.24	0.24	0.26	0.08	0.06	0.11	0.29	0.29
Crit Moves:	****			****						****		
Green/Cycle:	0.13	0.29	0.29	0.16	0.32	0.32	0.39	0.39	0.39	0.39	0.39	0.39
Volume/Cap:	0.59	0.82	0.82	0.29	0.75	0.75	0.67	0.20	0.16	0.29	0.75	0.75
Delay/Veh:	34.3	29.9	29.9	28.2	25.8	25.8	30.1	15.4	15.1	16.2	24.4	24.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	34.3	29.9	29.9	28.2	25.8	25.8	30.1	15.4	15.1	16.2	24.4	24.4
LOS by Move:	C	C	C	C	C	C	C	B	B	B	C	C
HCM2kAvgQ:	4	12	12	2	11	11	3	2	2	2	12	12

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #8 San Jacinto Street (NS) at Devonshire Avenue (EW)

Cycle (sec): 95 Critical Vol./Cap.(X): 0.875
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 32.6
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	18	10	18	18	28	28	28	28	28	28
Lanes:	1	0	1	1	0	1	1	0	1	1	0	1

Volume Module:

Base Vol:	102	801	223	115	709	98	181	358	107	129	220	117
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	102	801	223	115	709	98	181	358	107	129	220	117
Added Vol:	0	320	0	0	281	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	102	1121	223	115	990	98	181	358	107	129	220	117
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	107	1180	235	121	1042	103	191	377	113	136	232	123
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	107	1180	235	121	1042	103	191	377	113	136	232	123
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	107	1180	235	121	1042	103	191	377	113	136	232	123

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.93	0.93	0.95	0.94	0.94	0.34	1.00	0.85	0.31	0.95	0.95
Lanes:	1.00	1.67	0.33	1.00	1.82	0.18	1.00	1.00	1.00	1.00	0.65	0.35
Final Sat.:	1805	2936	584	1805	3242	321	644	1900	1615	591	1176	625

Capacity Analysis Module:

Vol/Sat:	0.06	0.40	0.40	0.07	0.32	0.32	0.30	0.20	0.07	0.23	0.20	0.20
Crit Moves:	****			****			****					
Green/Cycle:	0.14	0.44	0.44	0.11	0.41	0.41	0.33	0.33	0.33	0.33	0.33	0.33
Volume/Cap:	0.44	0.91	0.91	0.64	0.78	0.78	0.91	0.61	0.21	0.71	0.60	0.60
Delay/Veh:	39.0	32.8	32.8	47.8	26.9	26.9	68.4	28.7	23.4	39.3	28.7	28.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	39.0	32.8	32.8	47.8	26.9	26.9	68.4	28.7	23.4	39.3	28.7	28.7
LOS by Move:	D	C	C	D	C	C	E	C	C	D	C	C
HCM2kAvgQ:	3	24	24	5	17	17	9	10	2	5	9	9

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.943
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 122.5
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	28	28	28	28	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	1	0	1	0	1	1	0	2

Volume Module:

Base Vol:	81	259	77	163	241	149	200	702	43	97	1114	202
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	81	259	77	163	241	149	200	702	43	97	1114	202
Added Vol:	0	36	0	0	16	87	204	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	81	295	77	163	257	236	404	702	43	97	1114	202
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	85	311	81	172	271	248	425	739	45	102	1173	213
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	85	311	81	172	271	248	425	739	45	102	1173	213
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	85	311	81	172	271	248	425	739	45	102	1173	213

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.92	0.92	0.98	0.98	0.85	0.95	0.94	0.94	0.95	0.95	0.85
Lanes:	1.00	1.59	0.41	1.00	1.00	1.00	1.00	1.88	0.12	1.00	2.00	1.00
Final Sat.:	1805	2774	724	1864	1864	1615	1805	3371	206	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.05	0.11	0.11	0.09	0.15	0.15	0.24	0.22	0.22	0.06	0.32	0.13
Crit Moves:	****			****			****			****		
Green/Cycle:	0.23	0.23	0.23	0.23	0.23	0.23	0.17	0.29	0.29	0.11	0.23	0.23
Volume/Cap:	0.20	0.48	0.48	0.39	0.62	0.66	1.41	0.74	0.74	0.54	1.39	0.56
Delay/Veh:	37.3	40.2	40.2	39.1	43.0	45.9	254.7	41.1	41.1	53.9	230	42.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	37.3	40.2	40.2	39.1	43.0	45.9	254.7	41.1	41.1	53.9	230	42.6
LOS by Move:	D	D	D	D	D	D	F	D	D	D	F	D
HCM2kAvgQ:	3	7	7	5	10	9	33	15	15	4	45	7

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.261
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 177.3
Optimal Cycle: OPTIMIZED Level Of Service: F

Table with 4 main columns: North Bound, South Bound, East Bound, West Bound. Sub-columns: L, T, R. Rows: Approach, Movement, Control, Rights, Min. Green, Lanes.

Volume Module:
Base Vol: 79 349 144 457 338 208 223 1167 108 172 1193 75
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 79 349 144 457 338 208 223 1167 108 172 1193 75
Added Vol: 0 47 0 0 42 239 273 0 0 0 0 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 79 396 144 457 380 447 496 1167 108 172 1193 75
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95
PHF Volume: 83 417 152 481 400 471 522 1228 114 181 1256 79
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 83 417 152 481 400 471 522 1228 114 181 1256 79
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Final Volume: 83 417 152 481 400 471 522 1228 114 181 1256 79

Saturation Flow Module:
Sat/Lane: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900
Adjustment: 0.95 0.91 0.91 0.97 0.97 0.85 0.95 0.94 0.94 0.95 0.95 0.85
Lanes: 1.00 1.47 0.53 1.09 0.91 1.00 1.00 1.83 0.17 1.00 2.00 1.00
Final Sat.: 1805 2541 924 2019 1679 1615 1805 3261 302 1805 3610 1615

Capacity Analysis Module:
Vol/Sat: 0.05 0.16 0.16 0.24 0.24 0.29 0.29 0.38 0.38 0.10 0.35 0.05
Crit Moves: **** **** ****
Green/Cycle: 0.23 0.23 0.23 0.23 0.23 0.23 0.17 0.32 0.32 0.08 0.23 0.23
Volume/Cap: 0.20 0.70 0.70 1.02 1.02 1.25 1.74 1.19 1.19 1.19 1.49 0.21
Delay/Veh: 37.2 45.0 45.0 82.1 82.1 178.2 394.6 137 136.6 189.0 273 37.4
User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
AdjDel/Veh: 37.2 45.0 45.0 82.1 82.1 178.2 394.6 137 136.6 189.0 273 37.4
LOS by Move: D D D F F F F F F F F D
HCM2kAvgQ: 2 11 11 23 23 31 47 42 42 13 52 2

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

Cycle (sec): 105 Critical Vol./Cap.(X): 0.689
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 35.4
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	10	28	28	10	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	0	1	1	0	1	0

Volume Module:

Base Vol:	81	259	77	163	241	149	200	702	43	97	1114	202
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	81	259	77	163	241	149	200	702	43	97	1114	202
Added Vol:	0	36	0	0	16	87	204	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	81	295	77	163	257	236	404	702	43	97	1114	202
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	85	311	81	172	271	248	425	739	45	102	1173	213
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	85	311	81	172	271	248	425	739	45	102	1173	213
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	85	311	81	172	271	248	425	739	45	102	1173	213

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.92	0.92	0.92	1.00	0.85	0.92	0.94	0.94	0.95	0.89	0.89
Lanes:	1.00	1.59	0.41	2.00	1.00	1.00	2.00	1.88	0.12	1.00	2.54	0.46
Final Sat.:	1805	2774	724	3502	1900	1615	3502	3371	206	1805	4290	778

Capacity Analysis Module:

Vol/Sat:	0.05	0.11	0.11	0.05	0.14	0.15	0.12	0.22	0.22	0.06	0.27	0.27
Crit Moves:	****			****			****			****		
Green/Cycle:	0.10	0.27	0.27	0.10	0.27	0.42	0.15	0.36	0.36	0.13	0.34	0.34
Volume/Cap:	0.50	0.42	0.42	0.51	0.53	0.37	0.81	0.61	0.61	0.44	0.81	0.81
Delay/Veh:	47.4	32.1	32.1	46.6	34.0	21.5	52.6	28.6	28.6	43.7	34.9	34.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	47.4	32.1	32.1	46.6	34.0	21.5	52.6	28.6	28.6	43.7	34.9	34.9
LOS by Move:	D	C	C	D	C	C	D	C	C	D	C	C
HCM2kAvgQ:	3	6	6	3	8	6	9	11	11	4	17	17

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.898
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 51.9
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	10	28	28	10	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	0	1	1	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	79	349	144	457	338	208	223	1167	108	172	1193	75
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	79	349	144	457	338	208	223	1167	108	172	1193	75
Added Vol:	0	47	0	0	42	239	273	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	79	396	144	457	380	447	496	1167	108	172	1193	75
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	83	417	152	481	400	471	522	1228	114	181	1256	79
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	83	417	152	481	400	471	522	1228	114	181	1256	79
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	83	417	152	481	400	471	522	1228	114	181	1256	79

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	0.91	0.92	1.00	0.85	0.92	0.94	0.94	0.95	0.90	0.90
Lanes:	1.00	1.47	0.53	2.00	1.00	1.00	2.00	1.83	0.17	1.00	2.82	0.18
Final Sat.:	1805	2541	924	3502	1900	1615	3502	3261	302	1805	4836	304

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.05	0.16	0.16	0.14	0.21	0.29	0.15	0.38	0.38	0.10	0.26	0.26
Crit Moves:	****			****			****			****		
Green/Cycle:	0.10	0.23	0.23	0.14	0.28	0.46	0.18	0.39	0.39	0.10	0.31	0.31
Volume/Cap:	0.47	0.70	0.70	0.97	0.76	0.64	0.83	0.97	0.97	0.97	0.83	0.83
Delay/Veh:	53.0	45.0	45.0	83.9	46.3	27.0	56.7	53.5	53.5	110.5	42.2	42.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	53.0	45.0	45.0	83.9	46.3	27.0	56.7	53.5	53.5	110.5	42.2	42.2
LOS by Move:	D	D	D	F	D	C	E	D	D	F	D	D
HCM2kAvgQ:	3	11	11	13	15	14	12	31	31	10	19	19

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

Cycle (sec): 110 Critical Vol./Cap.(X): 0.870

Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 44.4

Optimal Cycle: OPTIMIZED Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 sub-columns for movements (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume) across 4 approaches.

Saturation Flow Module:

Table with 12 columns representing saturation flow metrics (Sat/Lane, Adjustment, Lanes, Final Sat.) across 4 approaches.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis metrics (Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ) across 4 approaches.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 1.289
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 111.2
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	24	24	10	24	24	10	28	28	10	28	28
Lanes:	1	0	2	0	1	1	0	1	1	0	1	1

Volume Module:

Base Vol:	86	406	375	182	702	23	23	344	86	560	378	157
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	86	406	375	182	702	23	23	344	86	560	378	157
Added Vol:	0	0	103	95	0	0	0	462	0	92	407	84
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	86	406	478	277	702	23	23	806	86	652	785	241
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	91	427	503	292	739	24	24	848	91	686	826	254
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	91	427	503	292	739	24	24	848	91	686	826	254
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	91	427	503	292	739	24	24	848	91	686	826	254

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.95	0.95	0.95	0.94	0.94	0.95	0.92	0.92
Lanes:	1.00	2.00	1.00	1.00	1.94	0.06	1.00	1.81	0.19	1.00	1.53	0.47
Final Sat.:	1805	3610	1615	1805	3478	114	1805	3216	343	1805	2665	818

Capacity Analysis Module:

Vol/Sat:	0.05	0.12	0.31	0.16	0.21	0.21	0.01	0.26	0.26	0.38	0.31	0.31
Crit Moves:	****			****			****			****		
Green/Cycle:	0.10	0.23	0.23	0.12	0.25	0.25	0.11	0.23	0.23	0.28	0.41	0.41
Volume/Cap:	0.51	0.51	1.35	1.35	0.84	0.84	0.12	1.13	1.13	1.35	0.76	0.76
Delay/Veh:	53.7	40.8	219.3	236.6	49.8	49.8	48.5	120	119.7	212.1	33.2	33.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	53.7	40.8	219.3	236.6	49.8	49.8	48.5	120	119.7	212.1	33.2	33.2
LOS by Move:	D	D	F	F	D	D	D	F	F	F	C	C
HCM2kAvgQ:	4	8	36	22	17	17	1	29	29	48	19	19

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

Cycle (sec): 95 Critical Vol./Cap.(X): 0.697
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 31.5
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Include		
Min. Green:	10	24	24	10	24	24	10	28	28	10	28	28
Lanes:	1	0	2	0	1	1	0	1	1	0	2	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	64	848	367	98	436	37	42	238	95	225	171	40
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	64	848	367	98	436	37	42	238	95	225	171	40
Added Vol:	0	0	77	71	0	0	0	346	0	38	152	32
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	64	848	444	169	436	37	42	584	95	263	323	72
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	67	893	467	178	459	39	44	615	100	277	340	76
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	67	893	467	178	459	39	44	615	100	277	340	76
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	67	893	467	178	459	39	44	615	100	277	340	76

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.92	0.94	0.94	0.95	0.93	0.93	0.92	0.92	0.92
Lanes:	1.00	2.00	1.00	2.00	1.84	0.16	1.00	1.72	0.28	2.00	1.64	0.36
Final Sat.:	1805	3610	1615	3502	3288	279	1805	3040	494	3502	2872	640

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.04	0.25	0.29	0.05	0.14	0.14	0.02	0.20	0.20	0.08	0.12	0.12
Crit Moves:	****			****			****			****		
Green/Cycle:	0.13	0.33	0.43	0.11	0.30	0.30	0.11	0.29	0.29	0.11	0.29	0.29
Volume/Cap:	0.29	0.76	0.67	0.48	0.46	0.46	0.23	0.69	0.69	0.75	0.40	0.40
Delay/Veh:	38.3	31.5	24.1	41.1	27.0	27.0	39.6	31.5	31.5	49.7	27.1	27.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	38.3	31.5	24.1	41.1	27.0	27.0	39.6	31.5	31.5	49.7	27.1	27.1
LOS by Move:	D	C	C	D	C	C	D	C	C	D	C	C
HCM2kAvgQ:	2	14	12	3	6	6	1	11	11	6	5	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

 Cycle (sec): 100 Critical Vol./Cap.(X): 0.860
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 42.1
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Include		
Min. Green:	10	24	24	10	24	24	10	28	28	10	28	28
Lanes:	1	0	2	0	1	1	0	1	1	0	2	0

Volume Module:

Base Vol:	86	406	375	182	702	23	23	344	86	560	378	157
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	86	406	375	182	702	23	23	344	86	560	378	157
Added Vol:	0	0	103	95	0	0	0	462	0	92	407	84
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	86	406	478	277	702	23	23	806	86	652	785	241
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	91	427	503	292	739	24	24	848	91	686	826	254
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	91	427	503	292	739	24	24	848	91	686	826	254
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	91	427	503	292	739	24	24	848	91	686	826	254

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.92	0.95	0.95	0.95	0.94	0.94	0.92	0.92	0.92
Lanes:	1.00	2.00	1.00	2.00	1.94	0.06	1.00	1.81	0.19	2.00	1.53	0.47
Final Sat.:	1805	3610	1615	3502	3478	114	1805	3216	343	3502	2665	818

Capacity Analysis Module:

Vol/Sat:	0.05	0.12	0.31	0.08	0.21	0.21	0.01	0.26	0.26	0.20	0.31	0.31
Crit Moves:	****			****			****			****		
Green/Cycle:	0.10	0.24	0.45	0.10	0.24	0.24	0.12	0.29	0.29	0.21	0.38	0.38
Volume/Cap:	0.50	0.49	0.69	0.83	0.89	0.89	0.11	0.92	0.92	0.92	0.82	0.82
Delay/Veh:	44.8	33.2	24.5	59.7	47.5	47.5	39.3	47.5	47.5	55.0	32.2	32.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	44.8	33.2	24.5	59.7	47.5	47.5	39.3	47.5	47.5	55.0	32.2	32.2
LOS by Move:	D	C	C	E	D	D	D	D	D	E	C	C
HCM2kAvgQ:	3	6	13	7	15	15	1	19	19	15	18	18

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

Average Delay (sec/veh): 21.9 Worst Case Level Of Service: F[253.5]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns representing different traffic volumes and adjustment factors.

Critical Gap Module: Table with 12 columns showing critical gap and follow-up time values.

Capacity Module: Table with 12 columns showing capacity, potential capacity, and volume/capacity ratios.

Level Of Service Module: Table with 12 columns showing level of service, control delay, and approach delay.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

Average Delay (sec/veh): 109.0 Worst Case Level Of Service: F[644.8]

Approach:	North Bound			South Bound			East Bound			West Bound										
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign										
Rights:	Include			Include			Include			Include										
Lanes:	1	0	2	0	1	1	0	2	0	1	0	1	0	0	1	0	0	1	0	0

Volume Module:

Base Vol:	109	649	1	61	1180	69	109	1	331	4	2	6
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	109	649	1	61	1180	69	109	1	331	4	2	6
Added Vol:	0	99	0	0	88	4	4	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	109	748	1	61	1268	73	113	1	331	4	2	6
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	115	787	1	64	1335	77	119	1	348	4	2	6
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	115	787	1	64	1335	77	119	1	348	4	2	6

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	4.1	xxxx	xxxxxx	7.5	6.5	6.9	7.5	6.5	6.9
FollowUpTim:	2.2	xxxx	xxxxxx	2.2	xxxx	xxxxxx	3.5	4.0	3.3	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	1412	xxxx	xxxxxx	788	xxxx	xxxxxx	2087	2481	667	1813	2557	394
Potent Cap.:	489	xxxx	xxxxxx	840	xxxx	xxxxxx	31	30	406	50	27	611
Move Cap.:	489	xxxx	xxxxxx	840	xxxx	xxxxxx	22	21	406	5	19	611
Volume/Cap:	0.23	xxxx	xxxx	0.08	xxxx	xxxx	5.44	0.05	0.86	0.80	0.11	0.01

Level Of Service Module:

2Way95thQ:	0.9	xxxx	xxxxxx	0.2	xxxx	xxxxxx	xxxx	xxxx	8.4	xxxx	xxxx	xxxxxx
Control Del:	14.6	xxxx	xxxxxx	9.6	xxxx	xxxxxx	xxxxxx	xxxx	48.7	xxxxxx	xxxx	xxxxxx
LOS by Move:	B	*	*	A	*	*	*	*	E	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	22	xxxx	xxxxxx	xxxx	14	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	15.2	xxxx	xxxxxx	xxxxxx	2.1	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	2376	xxxx	xxxxxx	xxxxxx	578	xxxxxx
Shared LOS:	*	*	*	*	*	*	F	*	*	*	F	*
ApproachDel:	xxxxxx			xxxxxx			644.8			577.6		
ApproachLOS:	*			*			F			F		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.459
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 10.3
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	18	18	18	18	28	28	28	28	28	28
Lanes:	1	0	2	0	1	0	0	1	0	0	0	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	136	1143	8	34	614	106	71	2	113	3	5	1
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	136	1143	8	34	614	106	71	2	113	3	5	1
Added Vol:	0	74	0	0	35	3	3	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	136	1217	8	34	649	109	74	2	113	3	5	1
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	143	1281	8	36	683	115	78	2	119	3	5	1
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	143	1281	8	36	683	115	78	2	119	3	5	1
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	143	1281	8	36	683	115	78	2	119	3	5	1

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.36	0.95	0.85	0.17	0.95	0.85	0.72	0.72	0.85	0.94	0.94	0.94
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	0.97	0.03	1.00	0.33	0.56	0.11
Final Sat.:	688	3610	1615	325	3610	1615	1338	36	1615	594	990	198

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.21	0.35	0.01	0.11	0.19	0.07	0.06	0.06	0.07	0.01	0.01	0.01
Crit Moves:	****			****			****			****		
Green/Cycle:	0.70	0.70	0.70	0.70	0.70	0.70	0.23	0.23	0.23	0.23	0.23	0.23
Volume/Cap:	0.30	0.51	0.01	0.16	0.27	0.10	0.25	0.25	0.32	0.02	0.02	0.02
Delay/Veh:	7.2	8.5	5.4	6.4	6.7	5.9	37.9	37.9	38.6	35.5	35.5	35.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	7.2	8.5	5.4	6.4	6.7	5.9	37.9	37.9	38.6	35.5	35.5	35.5
LOS by Move:	A	A	A	A	A	A	D	D	D	D	D	D
HCM2kAvgQ:	2	11	0	1	5	1	2	2	4	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.735
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 14.8
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	18	18	18	18	28	28	28	28	28	28
Lanes:	1	0	2	0	1	0	1	0	0	0	1	0

Volume Module:

Base Vol:	109	649	1	61	1180	69	109	1	331	4	2	6
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	109	649	1	61	1180	69	109	1	331	4	2	6
Added Vol:	0	99	0	0	88	4	4	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	109	748	1	61	1268	73	113	1	331	4	2	6
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	115	787	1	64	1335	77	119	1	348	4	2	6
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	115	787	1	64	1335	77	119	1	348	4	2	6
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	115	787	1	64	1335	77	119	1	348	4	2	6

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.13	0.95	0.85	0.31	0.95	0.85	0.71	0.71	0.85	0.88	0.88	0.88
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	0.99	0.01	1.00	0.33	0.17	0.50
Final Sat.:	253	3610	1615	580	3610	1615	1332	12	1615	560	280	839

Capacity Analysis Module:

Vol/Sat:	0.45	0.22	0.00	0.11	0.37	0.05	0.09	0.09	0.22	0.01	0.01	0.01
Crit Moves:	****						****					
Green/Cycle:	0.60	0.60	0.60	0.60	0.60	0.60	0.31	0.31	0.31	0.31	0.31	0.31
Volume/Cap:	0.76	0.36	0.00	0.18	0.62	0.08	0.29	0.29	0.69	0.02	0.02	0.02
Delay/Veh:	32.6	9.3	7.2	8.4	12.0	7.6	23.8	23.8	31.4	21.5	21.5	21.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	32.6	9.3	7.2	8.4	12.0	7.6	23.8	23.8	31.4	21.5	21.5	21.5
LOS by Move:	C	A	A	A	B	A	C	C	C	C	C	C
HCM2kAvgQ:	4	6	0	1	13	1	3	3	10	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #12 Mountain Avenue (NS) at Esplanade Avenue (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.901
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 31.7
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	T	R	L	T	R	L	T	R	L	T	R			
Control:	Protected			Permitted			Permitted			Permitted					
Rights:	Include			Include			Include			Include					
Min. Green:	10	24	0	0	24	24	21	0	21	0	0	0			
Lanes:	1	0	1	0	0	1	0	0	0	1	0	0	0	0	0

Volume Module:

Base Vol:	232	890	0	0	464	139	309	0	122	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	232	890	0	0	464	139	309	0	122	0	0	0
Added Vol:	0	71	0	0	32	3	3	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	232	961	0	0	496	142	312	0	122	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	244	1012	0	0	522	149	328	0	128	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	244	1012	0	0	522	149	328	0	128	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	244	1012	0	0	522	149	328	0	128	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	1.00	0.85	0.77	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	1900	1615	1461	0	1615	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.14	0.53	0.00	0.00	0.27	0.09	0.22	0.00	0.08	0.00	0.00	0.00
Crit Moves:	****						****					
Green/Cycle:	0.24	0.56	0.00	0.00	0.32	0.32	0.28	0.00	0.28	0.00	0.00	0.00
Volume/Cap:	0.56	0.95	0.00	0.00	0.86	0.29	0.80	0.00	0.28	0.00	0.00	0.00
Delay/Veh:	26.8	32.5	0.0	0.0	35.7	19.4	36.0	0.0	21.5	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	26.8	32.5	0.0	0.0	35.7	19.4	36.0	0.0	21.5	0.0	0.0	0.0
LOS by Move:	C	C	A	A	D	B	D	A	C	A	A	A
HCM2kAvgQ:	6	28	0	0	15	3	9	0	2	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #12 Mountain Avenue (NS) at Esplanade Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.088
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 66.5
Optimal Cycle: OPTIMIZED Level Of Service: E

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns for volume and growth factors. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with 12 columns for saturation flow factors. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with 12 columns for capacity analysis metrics. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #12 Mountain Avenue (NS) at Esplanade Avenue (EW)

Cycle (sec): 80 Critical Vol./Cap.(X): 0.891
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 27.1
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	24	0	0	24	24	21	0	21	0	0	0
Lanes:	1	0	1	0	0	2	0	0	1	0	0	0

Volume Module:

Base Vol:	232	890	0	0	464	139	309	0	122	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	232	890	0	0	464	139	309	0	122	0	0	0
Added Vol:	0	71	0	0	32	3	3	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	232	961	0	0	496	142	312	0	122	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	244	1012	0	0	522	149	328	0	128	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	244	1012	0	0	522	149	328	0	128	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	244	1012	0	0	522	149	328	0	128	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	0.95	0.85	0.77	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	3610	1615	1461	0	1615	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.14	0.53	0.00	0.00	0.14	0.09	0.22	0.00	0.08	0.00	0.00	0.00
Crit Moves:	****						****					
Green/Cycle:	0.29	0.59	0.00	0.00	0.30	0.30	0.26	0.00	0.26	0.00	0.00	0.00
Volume/Cap:	0.47	0.91	0.00	0.00	0.48	0.31	0.86	0.00	0.30	0.00	0.00	0.00
Delay/Veh:	24.2	25.1	0.0	0.0	23.3	22.0	45.1	0.0	24.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	24.2	25.1	0.0	0.0	23.3	22.0	45.1	0.0	24.0	0.0	0.0	0.0
LOS by Move:	C	C	A	A	C	C	D	A	C	A	A	A
HCM2kAvgQ:	5	26	0	0	6	3	11	0	3	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

 Intersection #12 Mountain Avenue (NS) at Esplanade Avenue (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.826
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 25.5
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	24	0	0	24	24	21	0	21	0	0	0
Lanes:	1	0	1	0	0	2	0	0	1	0	0	0

Volume Module:

Base Vol:	244	462	0	0	1003	401	249	0	360	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	244	462	0	0	1003	401	249	0	360	0	0	0
Added Vol:	0	95	0	0	84	4	4	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	244	557	0	0	1087	405	253	0	360	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	257	586	0	0	1144	426	266	0	379	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	257	586	0	0	1144	426	266	0	379	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	257	586	0	0	1144	426	266	0	379	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	0.95	0.85	0.77	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	3610	1615	1461	0	1615	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.14	0.31	0.00	0.00	0.32	0.26	0.18	0.00	0.23	0.00	0.00	0.00
Crit Moves:	****			****					****			
Green/Cycle:	0.17	0.56	0.00	0.00	0.38	0.38	0.28	0.00	0.28	0.00	0.00	0.00
Volume/Cap:	0.83	0.56	0.00	0.00	0.83	0.69	0.64	0.00	0.83	0.00	0.00	0.00
Delay/Veh:	46.3	11.3	0.0	0.0	25.1	22.6	26.9	0.0	36.8	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	46.3	11.3	0.0	0.0	25.1	22.6	26.9	0.0	36.8	0.0	0.0	0.0
LOS by Move:	D	B	A	A	C	C	C	A	D	A	A	A
HCM2kAvgQ:	8	9	0	0	15	9	7	0	11	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

Average Delay (sec/veh): 36.6 Worst Case Level Of Service: F[200.8]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	0	0	0	0	0	0	0	1	0

Volume Module:

Base Vol:	194	0	24	0	0	0	0	359	109	75	548	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	194	0	24	0	0	0	0	359	109	75	548	0
Added Vol:	36	0	0	0	0	0	0	16	16	0	36	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	230	0	24	0	0	0	0	375	125	75	584	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	242	0	25	0	0	0	0	395	132	79	615	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	242	0	25	0	0	0	0	395	132	79	615	0

Critical Gap Module:

Critical Gp:	6.4	xxxx	6.2	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	4.1	xxxx	xxxxx
FollowUpTim:	3.5	xxxx	3.3	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	2.2	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	1233	xxxx	461	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	526	xxxx	xxxxx
Potent Cap.:	197	xxxx	605	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	1051	xxxx	xxxxx
Move Cap.:	185	xxxx	605	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	1051	xxxx	xxxxx
Volume/Cap:	1.31	xxxx	0.04	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.08	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	13.7	xxxx	0.1	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.2	xxxx	xxxxx
Control Del:	220.6	xxxx	11.2	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	8.7	xxxx	xxxxx
LOS by Move:	F	*	B	*	*	*	*	*	*	A	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	0.2	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	8.7	xxxx	xxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	A	*	*
ApproachDel:	200.8			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	F			*			*			*		

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

Average Delay (sec/veh): 53.4 Worst Case Level Of Service: F[428.2]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume components (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume) and 4 rows for North, South, East, West bounds.

Critical Gap Module: Table with 4 columns for North, South, East, West bounds. Rows include Critical Gp and FollowUpTim.

Capacity Module: Table with 4 columns for North, South, East, West bounds. Rows include Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with 4 columns for North, South, East, West bounds. Rows include 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

 Cycle (sec): 65 Critical Vol./Cap.(X): 0.653
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 11.7
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	1	0	0	0	0	0	0	0	1	0	1	0

Volume Module:

Base Vol:	194	0	24	0	0	0	0	359	109	75	548	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	194	0	24	0	0	0	0	359	109	75	548	0
Added Vol:	36	0	0	0	0	0	0	16	16	0	36	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	230	0	24	0	0	0	0	375	125	75	584	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	242	0	25	0	0	0	0	395	132	79	615	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	242	0	25	0	0	0	0	395	132	79	615	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	242	0	25	0	0	0	0	395	132	79	615	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.77	1.00	0.85	1.00	1.00	1.00	1.00	0.97	0.97	0.90	0.90	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.75	0.25	0.11	0.89	0.00
Final Sat.:	1461	0	1615	0	0	0	0	1377	459	194	1512	0

Capacity Analysis Module:

Vol/Sat:	0.17	0.00	0.02	0.00	0.00	0.00	0.00	0.29	0.29	0.41	0.41	0.00
Crit Moves:	****			****								
Green/Cycle:	0.28	0.00	0.28	0.00	0.00	0.00	0.00	0.60	0.60	0.60	0.60	0.00
Volume/Cap:	0.60	0.00	0.06	0.00	0.00	0.00	0.00	0.48	0.48	0.68	0.68	0.00
Delay/Veh:	22.8	0.0	17.3	0.0	0.0	0.0	0.0	7.6	7.6	10.6	10.6	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	22.8	0.0	17.3	0.0	0.0	0.0	0.0	7.6	7.6	10.6	10.6	0.0
LOS by Move:	C	A	B	A	A	A	A	A	A	B	B	A
HCM2kAvgQ:	5	0	0	0	0	0	0	6	6	10	10	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

 Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

Cycle (sec): 105 Critical Vol./Cap.(X): 0.854
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 17.2
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	1	0	0	0	0	0	0	0	1	0	1	0

Volume Module:

Base Vol:	138	0	56	0	0	0	0	818	242	44	465	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	138	0	56	0	0	0	0	818	242	44	465	0
Added Vol:	47	0	0	0	0	0	0	42	42	0	47	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	185	0	56	0	0	0	0	860	284	44	512	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	195	0	59	0	0	0	0	905	299	46	539	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	195	0	59	0	0	0	0	905	299	46	539	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	195	0	59	0	0	0	0	905	299	46	539	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.77	1.00	0.85	1.00	1.00	1.00	1.00	0.97	0.97	0.85	0.85	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.75	0.25	0.08	0.92	0.00
Final Sat.:	1461	0	1615	0	0	0	0	1381	456	128	1484	0

Capacity Analysis Module:

Vol/Sat:	0.13	0.00	0.04	0.00	0.00	0.00	0.00	0.66	0.66	0.36	0.36	0.00
Crit Moves:	****			****								
Green/Cycle:	0.17	0.00	0.17	0.00	0.00	0.00	0.00	0.75	0.75	0.75	0.75	0.00
Volume/Cap:	0.78	0.00	0.21	0.00	0.00	0.00	0.00	0.87	0.87	0.48	0.48	0.00
Delay/Veh:	55.8	0.0	37.8	0.0	0.0	0.0	0.0	15.6	15.6	5.4	5.4	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	55.8	0.0	37.8	0.0	0.0	0.0	0.0	15.6	15.6	5.4	5.4	0.0
LOS by Move:	E	A	D	A	A	A	A	B	B	A	A	A
HCM2kAvgQ:	8	0	2	0	0	0	0	31	31	8	8	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 3.6 Worst Case Level Of Service: F[210.3]

Approach:	North Bound			South Bound			East Bound			West Bound						
Movement:	L	T	R	L	T	R	L	T	R	L	T	R				
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled						
Rights:	Include			Include			Include			Include						
Lanes:	1	0	0	0	0	0	0	0	1	0	1	1	0	1	0	0

Volume Module:

Base Vol:	25	0	17	0	0	0	0	1089	13	9	597	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	25	0	17	0	0	0	0	1089	13	9	597	0
Added Vol:	0	0	0	0	0	0	0	494	0	0	222	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	25	0	17	0	0	0	0	1583	13	9	819	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	26	0	18	0	0	0	0	1666	14	9	862	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	26	0	18	0	0	0	0	1666	14	9	862	0

Critical Gap Module:

Critical Gp:	6.4	xxxx	6.2	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	4.1	xxxx	xxxxx
FollowUpTim:	3.5	xxxx	3.3	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	2.2	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	2547	xxxx	1666	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	1680	xxxx	xxxxx
Potent Cap.:	30	xxxx	121	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	386	xxxx	xxxxx
Move Cap.:	29	xxxx	121	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	386	xxxx	xxxxx
Volume/Cap:	0.89	xxxx	0.15	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.02	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	3.0	xxxx	0.5	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.1	xxxx	xxxxx
Control Del:	326.1	xxxx	39.9	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	14.6	xxxx	xxxxx
LOS by Move:	F	*	E	*	*	*	*	*	*	B	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	210.3			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	F			*			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 7.7 Worst Case Level Of Service: F[646.3]

Approach:	North Bound			South Bound			East Bound			West Bound						
Movement:	L	T	R	L	T	R	L	T	R	L	T	R				
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled						
Rights:	Include			Include			Include			Include						
Lanes:	1	0	0	0	0	0	0	0	1	0	1	1	0	1	0	0

Volume Module:

Base Vol:	22	0	14	0	0	0	0	654	66	22	1027	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	22	0	14	0	0	0	0	654	66	22	1027	0
Added Vol:	0	0	0	0	0	0	0	660	0	0	583	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	22	0	14	0	0	0	0	1314	66	22	1610	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	23	0	15	0	0	0	0	1383	69	23	1695	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	23	0	15	0	0	0	0	1383	69	23	1695	0

Critical Gap Module:

Critical Gp:	6.4	xxxx	6.2	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	4.1	xxxx	xxxxx
FollowUpTim:	3.5	xxxx	3.3	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	2.2	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	3124	xxxx	1383	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	1453	xxxx	xxxxx
Potent Cap.:	13	xxxx	178	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	472	xxxx	xxxxx
Move Cap.:	12	xxxx	178	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	472	xxxx	xxxxx
Volume/Cap:	1.89	xxxx	0.08	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.05	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	3.7	xxxx	0.3	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.2	xxxx	xxxxx			
Control Del:	1040	xxxx	27.1	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	13.0	xxxx	xxxxx			
LOS by Move:	F	*	D	*	*	*	*	*	*	B	*	*			
Movement:	LT	-	LTR	-	RT	LT	-	LTR	-	RT	LT	-	LTR	-	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx			
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx			
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx			
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*			
ApproachDel:	646.3			xxxxxxx			xxxxxxx			xxxxxxx					
ApproachLOS:	F			*			*			*					

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
Year 2025 With Project
Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: C [18.2]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control, Rights, and Lanes.

Volume Module table with 12 columns for volume components and 4 columns for bound types. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module table with 4 columns for bound types. Rows include Critical Gp and FollowUpTim.

Capacity Module table with 4 columns for bound types. Rows include Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module table with 4 columns for bound types. Rows include 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
Year 2025 With Project
Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 0.3 Worst Case Level Of Service: C [15.2]

Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 0.513
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 5.6
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	1	0	0	0	0	0	0	2	0	1	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	25	0	17	0	0	0	0	1089	13	9	597	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	25	0	17	0	0	0	0	1089	13	9	597	0
Added Vol:	0	0	0	0	0	0	0	494	0	0	222	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	25	0	17	0	0	0	0	1583	13	9	819	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	26	0	18	0	0	0	0	1666	14	9	862	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	26	0	18	0	0	0	0	1666	14	9	862	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	26	0	18	0	0	0	0	1666	14	9	862	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.82	1.00	0.85	1.00	1.00	1.00	1.00	0.95	0.85	0.11	0.95	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	2.00	1.00	1.00	2.00	0.00
Final Sat.:	1560	0	1615	0	0	0	0	3610	1615	217	3610	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.02	0.00	0.01	0.00	0.00	0.00	0.00	0.46	0.01	0.04	0.24	0.00
Crit Moves:	****						****					
Green/Cycle:	0.15	0.00	0.15	0.00	0.00	0.00	0.00	0.78	0.78	0.78	0.78	0.00
Volume/Cap:	0.11	0.00	0.07	0.00	0.00	0.00	0.00	0.59	0.01	0.06	0.30	0.00
Delay/Veh:	44.3	0.0	44.0	0.0	0.0	0.0	0.0	5.6	2.8	3.1	3.8	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	44.3	0.0	44.0	0.0	0.0	0.0	0.0	5.6	2.8	3.1	3.8	0.0
LOS by Move:	D	A	D	A	A	A	A	A	A	A	A	A
HCM2kAvgQ:	1	0	1	0	0	0	0	13	0	0	5	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.519
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 5.6
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	1	0	0	0	0	0	0	0	2	0	1	0

Volume Module:

Base Vol:	22	0	14	0	0	0	0	654	66	22	1027	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	22	0	14	0	0	0	0	654	66	22	1027	0
Added Vol:	0	0	0	0	0	0	0	660	0	0	583	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	22	0	14	0	0	0	0	1314	66	22	1610	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	23	0	15	0	0	0	0	1383	69	23	1695	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	23	0	15	0	0	0	0	1383	69	23	1695	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	23	0	15	0	0	0	0	1383	69	23	1695	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.83	1.00	0.85	1.00	1.00	1.00	1.00	0.95	0.85	0.17	0.95	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	2.00	1.00	1.00	2.00	0.00
Final Sat.:	1575	0	1615	0	0	0	0	3610	1615	317	3610	0

Capacity Analysis Module:

Vol/Sat:	0.01	0.00	0.01	0.00	0.00	0.00	0.00	0.38	0.04	0.07	0.47	0.00
Crit Moves:	****											
Green/Cycle:	0.15	0.00	0.15	0.00	0.00	0.00	0.00	0.78	0.78	0.78	0.78	0.00
Volume/Cap:	0.10	0.00	0.06	0.00	0.00	0.00	0.00	0.49	0.05	0.09	0.60	0.00
Delay/Veh:	44.2	0.0	43.9	0.0	0.0	0.0	0.0	4.7	3.0	3.2	5.7	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	44.2	0.0	43.9	0.0	0.0	0.0	0.0	4.7	3.0	3.2	5.7	0.0
LOS by Move:	D	A	D	A	A	A	A	A	A	A	A	A
HCM2kAvgQ:	1	0	0	0	0	0	0	9	1	0	14	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 1.9 Worst Case Level Of Service: E [45.2]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	1	0	1	0	0	1	0

Volume Module:

Base Vol:	16	627	4	7	424	13	10	1	63	17	1	9
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	16	627	4	7	424	13	10	1	63	17	1	9
Added Vol:	0	96	0	0	213	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	16	723	4	7	637	13	10	1	63	17	1	9
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	17	761	4	7	671	14	11	1	66	18	1	9
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	17	761	4	7	671	14	11	1	66	18	1	9

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	4.1	xxxx	xxxxxx	7.1	6.5	6.2	7.1	6.5	6.2
FollowUpTim:	2.2	xxxx	xxxxxx	2.2	xxxx	xxxxxx	3.5	4.0	3.3	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	684	xxxx	xxxxxx	765	xxxx	xxxxxx	1487	1484	671	1523	1496	763
Potent Cap.:	919	xxxx	xxxxxx	857	xxxx	xxxxxx	104	126	460	98	124	407
Move Cap.:	919	xxxx	xxxxxx	857	xxxx	xxxxxx	98	123	460	82	121	407
Volume/Cap:	0.02	xxxx	xxxxxx	0.01	xxxx	xxxxxx	0.11	0.01	0.14	0.22	0.01	0.02

Level Of Service Module:

2Way95thQ:	0.1	xxxx	xxxxxx	0.0	xxxx	xxxxxx	xxxx	xxxx	0.5	xxxx	xxxx	0.1
Control Del:	9.0	xxxx	xxxxxx	9.2	xxxx	xxxxxx	xxxxxx	xxxxxx	14.1	xxxxxx	xxxxxx	14.0
LOS by Move:	A	*	*	A	*	*	*	*	B	*	*	B
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	100	xxxx	xxxxxx	83	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	0.4	xxxx	xxxxxx	0.8	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	45.6	xxxx	xxxxxx	60.8	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	E	*	*	F	*	*
ApproachDel:	xxxxxx			xxxxxx			18.8			45.2		
ApproachLOS:		*			*		C			E		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 1.4 Worst Case Level Of Service: F [75.6]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	1	0	1	0	0	1	0

Volume Module:

Base Vol:	52	454	17	9	729	15	5	1	26	6	1	4
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	52	454	17	9	729	15	5	1	26	6	1	4
Added Vol:	0	251	0	0	285	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	52	705	17	9	1014	15	5	1	26	6	1	4
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	55	742	18	9	1067	16	5	1	27	6	1	4
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	55	742	18	9	1067	16	5	1	27	6	1	4

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	4.1	xxxx	xxxxxx	7.1	6.5	6.2	7.1	6.5	6.2
FollowUpTim:	2.2	xxxx	xxxxxx	2.2	xxxx	xxxxxx	3.5	4.0	3.3	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	1083	xxxx	xxxxxx	760	xxxx	xxxxxx	1949	1956	1067	1969	1963	751
Potent Cap.:	652	xxxx	xxxxxx	861	xxxx	xxxxxx	49	65	272	48	64	414
Move Cap.:	652	xxxx	xxxxxx	861	xxxx	xxxxxx	44	59	272	39	58	414
Volume/Cap:	0.08	xxxx	xxxxxx	0.01	xxxx	xxxxxx	0.12	0.02	0.10	0.16	0.02	0.01

Level Of Service Module:

2Way95thQ:	0.3	xxxx	xxxxxx	0.0	xxxx	xxxxxx	xxxx	xxxx	0.3	xxxx	xxxx	0.0
Control Del:	11.0	xxxx	xxxxxx	9.2	xxxx	xxxxxx	xxxxxx	xxxx	19.7	xxxxxx	xxxx	13.8
LOS by Move:	B	*	*	A	*	*	*	*	C	*	*	B
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	46	xxxx	xxxxxx	41	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	0.4	xxxx	xxxxxx	0.6	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	94.6	xxxx	xxxxxx	111.0	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	F	*	*	F	*	*
ApproachDel:	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	33.8	xxxxxx	xxxxxx	75.6	xxxxxx	
ApproachLOS:	*	*	*	*	*	*	D	*	*	F	*	

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 1.3 Worst Case Level Of Service: C [24.9]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	0	1	0	0	1	0	0

Volume Module:

Base Vol:	16	627	4	7	424	13	10	0	63	17	0	9
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	16	627	4	7	424	13	10	0	63	17	0	9
Added Vol:	0	96	0	0	213	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	16	723	4	7	637	13	10	0	63	17	0	9
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	17	761	4	7	671	14	11	0	66	18	0	9
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	17	761	4	7	671	14	11	0	66	18	0	9

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	4.1	xxxx	xxxxxx	7.1	xxxx	6.2	7.1	xxxx	6.2
FollowUpTim:	2.2	xxxx	xxxxxx	2.2	xxxx	xxxxxx	3.5	xxxx	3.3	3.5	xxxx	3.3

Capacity Module:

Cnflct Vol:	684	xxxx	xxxxxx	765	xxxx	xxxxxx	1487	xxxx	335	1147	xxxx	763
Potent Cap.:	919	xxxx	xxxxxx	857	xxxx	xxxxxx	104	xxxx	711	178	xxxx	407
Move Cap.:	919	xxxx	xxxxxx	857	xxxx	xxxxxx	99	xxxx	711	158	xxxx	407
Volume/Cap:	0.02	xxxx	xxxx	0.01	xxxx	xxxx	0.11	xxxx	0.09	0.11	xxxx	0.02

Level Of Service Module:

2Way95thQ:	0.1	xxxx	xxxxxx	0.0	xxxx	xxxxxx	0.3	xxxx	0.3	0.4	xxxx	0.1
Control Del:	9.0	xxxx	xxxxxx	9.2	xxxx	xxxxxx	45.6	xxxx	10.6	30.7	xxxx	14.0
LOS by Move:	A	*	*	A	*	*	E	*	B	D	*	B
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxxx			xxxxxxx			15.4			24.9		
ApproachLOS:	*			*			C			C		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 0.9 Worst Case Level Of Service: D [30.9]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	2	0	0	0	1	0	0

Volume Module:

Base Vol:	52	454	17	9	729	15	5	0	26	6	0	4
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	52	454	17	9	729	15	5	0	26	6	0	4
Added Vol:	0	251	0	0	285	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	52	705	17	9	1014	15	5	0	26	6	0	4
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	55	742	18	9	1067	16	5	0	27	6	0	4
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	55	742	18	9	1067	16	5	0	27	6	0	4

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	4.1	xxxx	xxxxxx	7.1	xxxx	6.2	7.1	xxxx	6.2
FollowUpTim:	2.2	xxxx	xxxxxx	2.2	xxxx	xxxxxx	3.5	xxxx	3.3	3.5	xxxx	3.3

Capacity Module:

CnFlict Vol:	1083	xxxx	xxxxxx	760	xxxx	xxxxxx	1949	xxxx	534	1413	xxxx	751
Potent Cap.:	652	xxxx	xxxxxx	861	xxxx	xxxxxx	49	xxxx	550	117	xxxx	414
Move Cap.:	652	xxxx	xxxxxx	861	xxxx	xxxxxx	45	xxxx	550	103	xxxx	414
Volume/Cap:	0.08	xxxx	xxxx	0.01	xxxx	xxxx	0.12	xxxx	0.05	0.06	xxxx	0.01

Level Of Service Module:

2Way95thQ:	0.3	xxxx	xxxxxx	0.0	xxxx	xxxxxx	0.4	xxxx	0.2	0.2	xxxx	0.0
Control Del:	11.0	xxxx	xxxxxx	9.2	xxxx	xxxxxx	95.1	xxxx	11.9	42.3	xxxx	13.8
LOS by Move:	B	*	*	A	*	*	F	*	B	E	*	B
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			25.3			30.9		
ApproachLOS:	*			*			D			D		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #16 Soboba Road (NS) at Project North Entrance (NS)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.401
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 17.3
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	18	0	18	0	0	0
Lanes:	1	0	1	0	0	2	0	0	1	0	0	0

Volume Module:

Base Vol:	0	238	0	0	345	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	238	0	0	345	0	0	0	0	0	0	0
Added Vol:	242	31	0	0	52	161	66	0	98	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	242	269	0	0	397	161	66	0	98	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	255	283	0	0	418	169	69	0	103	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	255	283	0	0	418	169	69	0	103	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	255	283	0	0	418	169	69	0	103	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	0.95	0.85	0.81	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	3610	1615	1530	0	1615	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.14	0.15	0.00	0.00	0.12	0.10	0.05	0.00	0.06	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.20	0.50	0.00	0.00	0.30	0.30	0.30	0.00	0.30	0.00	0.00	0.00
Volume/Cap:	0.71	0.30	0.00	0.00	0.39	0.35	0.15	0.00	0.21	0.00	0.00	0.00
Delay/Veh:	28.6	9.0	0.0	0.0	16.9	16.9	15.6	0.0	15.9	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	28.6	9.0	0.0	0.0	16.9	16.9	15.6	0.0	15.9	0.0	0.0	0.0
LOS by Move:	C	A	A	A	B	B	B	A	B	A	A	A
HCM2kAvgQ:	6	3	0	0	4	3	1	0	2	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #16 Soboba Road (NS) at Project North Entrance (NS)

Cycle (sec): 65 Critical Vol./Cap.(X): 0.723
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 21.1
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	18	0	18	0	0	0
Lanes:	1	0	1	0	0	2	0	0	1	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	437	0	0	683	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	437	0	0	683	0	0	0	0	0	0	0
Added Vol:	320	65	0	0	72	213	187	0	280	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	320	502	0	0	755	213	187	0	280	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	337	528	0	0	795	224	197	0	295	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	337	528	0	0	795	224	197	0	295	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	337	528	0	0	795	224	197	0	295	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	0.95	0.85	0.77	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	3610	1615	1461	0	1615	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.19	0.28	0.00	0.00	0.22	0.14	0.13	0.00	0.18	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.25	0.54	0.00	0.00	0.29	0.29	0.28	0.00	0.28	0.00	0.00	0.00
Volume/Cap:	0.76	0.52	0.00	0.00	0.76	0.48	0.49	0.00	0.66	0.00	0.00	0.00
Delay/Veh:	29.9	10.1	0.0	0.0	24.1	19.7	20.6	0.0	24.4	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	29.9	10.1	0.0	0.0	24.1	19.7	20.6	0.0	24.4	0.0	0.0	0.0
LOS by Move:	C	B	A	A	C	B	C	A	C	A	A	A
HCM2kAvgQ:	8	7	0	0	10	4	4	0	6	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #17 Soboba Road (NS) at Project South Entrance (EW)

Cycle (sec): 70 Critical Vol./Cap.(X): 0.476
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 17.7
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	18	0	18	0	0	0
Lanes:	1	0	1	0	0	2	0	0	1	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	238	0	0	345	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	238	0	0	345	0	0	0	0	0	0	0
Added Vol:	322	239	0	0	70	81	33	0	131	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	322	477	0	0	415	81	33	0	131	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	339	502	0	0	437	85	35	0	138	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	339	502	0	0	437	85	35	0	138	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	339	502	0	0	437	85	35	0	138	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	0.95	0.85	0.84	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	3610	1615	1590	0	1615	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.19	0.26	0.00	0.00	0.12	0.05	0.02	0.00	0.09	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.31	0.57	0.00	0.00	0.26	0.26	0.26	0.00	0.26	0.00	0.00	0.00
Volume/Cap:	0.60	0.46	0.00	0.00	0.47	0.21	0.08	0.00	0.33	0.00	0.00	0.00
Delay/Veh:	22.0	9.1	0.0	0.0	22.4	20.6	19.8	0.0	21.6	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	22.0	9.1	0.0	0.0	22.4	20.6	19.8	0.0	21.6	0.0	0.0	0.0
LOS by Move:	C	A	A	A	C	C	B	A	C	A	A	A
HCM2kAvgQ:	7	7	0	0	5	2	1	0	3	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #17 Soboba Road (NS) at Project South Entrance (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.907
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 32.4
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	18	0	18	0	0	0
Lanes:	1	0	1	0	0	2	0	0	1	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	437	0	0	683	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	437	0	0	683	0	0	0	0	0	0	0
Added Vol:	426	291	0	0	245	107	93	0	373	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	426	728	0	0	928	107	93	0	373	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	448	766	0	0	977	113	98	0	393	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	448	766	0	0	977	113	98	0	393	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	448	766	0	0	977	113	98	0	393	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	0.95	0.85	0.78	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	3610	1615	1474	0	1615	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.25	0.40	0.00	0.00	0.27	0.07	0.07	0.00	0.24	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.27	0.57	0.00	0.00	0.30	0.30	0.27	0.00	0.27	0.00	0.00	0.00
Volume/Cap:	0.91	0.71	0.00	0.00	0.91	0.23	0.25	0.00	0.91	0.00	0.00	0.00
Delay/Veh:	46.7	13.6	0.0	0.0	36.4	20.1	21.9	0.0	49.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	46.7	13.6	0.0	0.0	36.4	20.1	21.9	0.0	49.0	0.0	0.0	0.0
LOS by Move:	D	B	A	A	D	C	C	A	D	A	A	A
HCM2kAvgQ:	14	14	0	0	16	2	2	0	13	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Future Volume Alternative)

Intersection #18 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 1.677
 Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 147.0
 Optimal Cycle: 0 Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0	0	1! 0	0	0	1! 0	0	1	0	0	1	0

Volume Module:

Base Vol:	298	45	1	2	164	179	191	2	461	1	2	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	298	45	1	2	164	179	191	2	461	1	2	2
Added Vol:	-8	-2	0	0	-29	230	564	0	-70	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	290	43	1	2	135	409	755	2	391	1	2	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	305	45	1	2	142	431	795	2	412	1	2	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	305	45	1	2	142	431	795	2	412	1	2	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	305	45	1	2	142	431	795	2	412	1	2	2

Saturation Flow Module:

Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.86	0.13	0.01	0.01	0.24	0.75	0.99	0.01	1.00	0.20	0.40	0.40
Final Sat.:	443	66	2	2	146	444	474	1	565	82	163	163

Capacity Analysis Module:

Vol/Sat:	0.69	0.69	0.69	0.97	0.97	0.97	1.68	1.68	0.73	0.01	0.01	0.01
Crit Moves:	****			****			****			****		
Delay/Veh:	23.9	23.9	23.9	54.3	54.3	54.3	332.6	333	24.3	11.3	11.3	11.3
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	23.9	23.9	23.9	54.3	54.3	54.3	332.6	333	24.3	11.3	11.3	11.3
LOS by Move:	C	C	C	F	F	F	F	F	C	B	B	B
ApproachDel:	23.9			54.3			227.6			11.3		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	23.9			54.3			227.6			11.3		
LOS by Appr:	C			F			F			B		
AllWayAvgQ:	2.0	2.0	2.0	7.4	7.4	7.4	42.5	42.5	2.4	0.0	0.0	0.0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Future Volume Alternative)

 Intersection #18 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 2.446
 Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 470.3
 Optimal Cycle: 0 Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0	0	1! 0	0	0	1! 0	0	1	0	0	1	0

Volume Module:

Base Vol:	530	196	1	2	288	400	239	2	599	1	2	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	530	196	1	2	288	400	239	2	599	1	2	2
Added Vol:	-71	-29	0	0	-35	653	746	0	-86	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	459	167	1	2	253	1053	985	2	513	1	2	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	483	176	1	2	266	1108	1037	2	540	1	2	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	483	176	1	2	266	1108	1037	2	540	1	2	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	483	176	1	2	266	1108	1037	2	540	1	2	2

Saturation Flow Module:

Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.73	0.26	0.01	0.01	0.19	0.80	0.99	0.01	1.00	0.20	0.40	0.40
Final Sat.:	375	137	1	1	109	453	451	1	532	80	160	160

Capacity Analysis Module:

Vol/Sat:	1.29	1.29	1.29	2.45	2.45	2.45	2.30	2.30	1.02	0.01	0.01	0.01
Crit Moves:	****			****			****			****		
Delay/Veh:	165.7	166	165.7	670.9	671	670.9	609.0	609	68.6	12.1	12.1	12.1
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	165.7	166	165.7	670.9	671	670.9	609.0	609	68.6	12.1	12.1	12.1
LOS by Move:	F	F	F	F	F	F	F	F	F	B	B	B
ApproachDel:	165.7			670.9			424.1			12.1		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	165.7			670.9			424.1			12.1		
LOS by Appr:	F			F			F			B		
AllWayAvgQ:	22.2	22.2	22.2	103	103	103.4	75.1	75.1	8.7	0.0	0.0	0.0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #18 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 110 Critical Vol./Cap.(X): 0.546
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 31.3
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ovl			Ovl			Include		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	1	0	0	1	0	0	1	1	0	0	0	1

Volume Module:

Base Vol:	298	45	1	2	164	179	191	2	461	1	2	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	298	45	1	2	164	179	191	2	461	1	2	2
Added Vol:	-8	-2	0	0	-29	230	564	0	-70	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	290	43	1	2	135	409	755	2	391	1	2	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	305	45	1	2	142	431	795	2	412	1	2	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	305	45	1	2	142	431	795	2	412	1	2	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	305	45	1	2	142	431	795	2	412	1	2	2

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	0.95	1.00	0.75	0.95	0.95	0.85	0.94	0.94	0.94
Lanes:	1.00	0.98	0.02	1.00	1.00	2.00	1.99	0.01	1.00	0.20	0.40	0.40
Final Sat.:	1805	1851	43	1805	1900	2842	3612	10	1615	356	712	712

Capacity Analysis Module:

Vol/Sat:	0.17	0.02	0.02	0.00	0.07	0.15	0.22	0.22	0.25	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.23	0.25	0.25	0.14	0.16	0.46	0.30	0.30	0.53	0.16	0.16	0.16
Volume/Cap:	0.74	0.10	0.10	0.01	0.46	0.33	0.74	0.74	0.48	0.02	0.02	0.02
Delay/Veh:	46.2	31.6	31.6	40.7	42.7	18.9	37.5	37.5	16.9	38.6	38.6	38.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	46.2	31.6	31.6	40.7	42.7	18.9	37.5	37.5	16.9	38.6	38.6	38.6
LOS by Move:	D	C	C	D	D	B	D	D	B	D	D	D
HCM2kAvgQ:	11	1	1	0	5	5	13	13	9	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "A"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #18 Soboba Road (NS) at Lake Park Drive (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 0.805
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 51.5
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ovl			Ovl			Include		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	1	0	0	1	0	0	1	1	0	0	0	1

Volume Module:

Base Vol:	530	196	1	2	288	400	239	2	599	1	2	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	530	196	1	2	288	400	239	2	599	1	2	2
Added Vol:	-71	-29	0	0	-35	653	746	0	-86	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	459	167	1	2	253	1053	985	2	513	1	2	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	483	176	1	2	266	1108	1037	2	540	1	2	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	483	176	1	2	266	1108	1037	2	540	1	2	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	483	176	1	2	266	1108	1037	2	540	1	2	2

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	0.95	1.00	0.75	0.95	0.95	0.85	0.94	0.94	0.94
Lanes:	1.00	0.99	0.01	1.00	1.00	2.00	1.99	0.01	1.00	0.20	0.40	0.40
Final Sat.:	1805	1887	11	1805	1900	2842	3610	7	1615	356	712	712

Capacity Analysis Module:

Vol/Sat:	0.27	0.09	0.09	0.00	0.14	0.39	0.29	0.29	0.33	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.27	0.27	0.27	0.15	0.15	0.44	0.29	0.29	0.57	0.15	0.15	0.15
Volume/Cap:	0.98	0.34	0.34	0.01	0.93	0.88	0.98	0.98	0.59	0.02	0.02	0.02
Delay/Veh:	78.3	35.4	35.4	43.3	86.6	37.9	64.6	64.6	18.0	43.5	43.5	43.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	78.3	35.4	35.4	43.3	86.6	37.9	64.6	64.6	18.0	43.5	43.5	43.5
LOS by Move:	E	D	D	D	F	D	E	E	B	D	D	D
HCM2kAvgQ:	23	5	5	0	13	24	25	25	13	0	0	0

Note: Queue reported is the number of cars per lane.

Year 2025 With Project – Proposed Action “B”

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 Sanderson Avenue (NS) at Ramona Expressway (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.857
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 51.5
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Ovl			Ovl			Ovl		
Min. Green:	10	28	28	10	28	28	10	28	28	10	28	28
Lanes:	2	0	2	0	2	0	1	2	0	2	0	2
	0	1			1			1		0	1	

Volume Module:

Base Vol:	76	917	14	414	614	315	663	413	47	7	468	529
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	76	917	14	414	614	315	663	413	47	7	468	529
Added Vol:	0	0	0	35	0	0	0	35	0	0	16	16
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	76	917	14	449	614	315	663	448	47	7	484	545
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	80	965	15	473	646	332	698	472	49	7	509	574
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	80	965	15	473	646	332	698	472	49	7	509	574
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	80	965	15	473	646	332	698	472	49	7	509	574

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.95	0.85	0.92	0.95	0.85	0.92	0.95	0.85	0.92	0.95	0.85
Lanes:	2.00	2.00	1.00	2.00	2.00	1.00	2.00	2.00	1.00	2.00	2.00	1.00
Final Sat.:	3502	3610	1615	3502	3610	1615	3502	3610	1615	3502	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.02	0.27	0.01	0.13	0.18	0.21	0.20	0.13	0.03	0.00	0.14	0.36
Crit Moves:	****			****			****			****		
Green/Cycle:	0.11	0.28	0.40	0.14	0.31	0.52	0.21	0.33	0.44	0.12	0.23	0.38
Volume/Cap:	0.20	0.95	0.02	0.95	0.57	0.39	0.95	0.40	0.07	0.02	0.60	0.95
Delay/Veh:	48.7	59.7	22.0	79.2	35.3	17.6	68.6	31.5	19.6	46.9	42.3	60.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	48.7	59.7	22.0	79.2	35.3	17.6	68.6	31.5	19.6	46.9	42.3	60.3
LOS by Move:	D	E	C	E	D	B	E	C	B	D	D	E
HCM2kAvgQ:	2	23	0	13	11	7	17	7	1	0	9	25

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 Sanderson Avenue (NS) at Ramona Expressway (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.849
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 52.9
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Approach, Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different volume metrics and 4 columns for North, South, East, and West bounds.

Saturation Flow Module:

Table with 12 columns representing saturation flow metrics and 4 columns for North, South, East, and West bounds.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis metrics and 4 columns for North, South, East, and West bounds.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	0	1	0	0	0	1	0

Volume Module:

Base Vol:	1	693	15	148	252	1	1	1	1	15	1	366
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	1	693	15	148	252	1	1	1	1	15	1	366
Added Vol:	0	0	0	207	0	0	0	0	0	0	0	94
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	1	693	15	355	252	1	1	1	1	15	1	460
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	1	729	16	374	265	1	1	1	1	16	1	484
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	1	729	16	374	265	1	1	1	1	16	1	484

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	4.1	xxxx	xxxxxx	7.1	6.5	6.2	7.1	6.5	6.2
FollowUpTim:	2.2	xxxx	xxxxxx	2.2	xxxx	xxxxxx	3.5	4.0	3.3	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	266	xxxx	xxxxxx	745	xxxx	xxxxxx	1995	1761	266	1754	1753	737
Potent Cap.:	1309	xxxx	xxxxxx	872	xxxx	xxxxxx	46	85	778	67	86	422
Move Cap.:	1309	xxxx	xxxxxx	872	xxxx	xxxxxx	0	49	778	44	49	422
Volume/Cap:	0.00	xxxx	xxxxxx	0.43	xxxx	xxxxxx	xxxx	0.02	0.00	0.36	0.02	1.15

Level Of Service Module:

2Way95thQ:	0.0	xxxx	xxxxxx	2.2	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	18.0
Control Del:	7.8	xxxx	xxxxxx	12.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	121.4
LOS by Move:	A	*	*	B	*	*	*	*	*	*	*	F
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	0	xxxxxx	44	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	1.3	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	129.1	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	F	*	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			121.7		
ApproachLOS:	*			*			F			F		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

Average Delay (sec/veh): 37.4 Worst Case Level Of Service: F[3416.3]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns representing different traffic volumes and adjustment factors.

Critical Gap Module: Table with 12 columns showing critical gap and follow-up time values.

Capacity Module: Table with 12 columns showing capacity-related metrics like Cnflct Vol, Potent Cap., etc.

Level Of Service Module: Table with 12 columns showing Level of Service (LOS) and other performance indicators.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

 Cycle (sec): 105 Critical Vol./Cap.(X): 0.690
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 33.0
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Ovl		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	1	0	0	1	0	0	0	0	1	0	1	0

Volume Module:

Base Vol:	1	693	15	148	252	1	1	1	1	15	1	366
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	1	693	15	148	252	1	1	1	1	15	1	366
Added Vol:	0	0	0	207	0	0	0	0	0	0	0	94
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	1	693	15	355	252	1	1	1	1	15	1	460
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	1	729	16	374	265	1	1	1	1	16	1	484
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	1	729	16	374	265	1	1	1	1	16	1	484
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	1	729	16	374	265	1	1	1	1	16	1	484

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	0.95	1.00	1.00	0.92	0.92	0.92	0.83	0.83	0.85
Lanes:	1.00	0.98	0.02	1.00	0.99	0.01	0.34	0.33	0.33	0.94	0.06	1.00
Final Sat.:	1805	1854	40	1805	1891	8	581	581	581	1477	98	1615

Capacity Analysis Module:

Vol/Sat:	0.00	0.39	0.39	0.21	0.14	0.14	0.00	0.00	0.00	0.01	0.01	0.30
Crit Moves:	****			****						****		
Green/Cycle:	0.26	0.47	0.47	0.25	0.46	0.46	0.17	0.17	0.17	0.17	0.17	0.42
Volume/Cap:	0.00	0.84	0.84	0.84	0.31	0.31	0.01	0.01	0.01	0.06	0.06	0.72
Delay/Veh:	29.1	31.7	31.7	51.0	18.1	18.1	36.1	36.1	36.1	36.5	36.5	29.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	29.1	31.7	31.7	51.0	18.1	18.1	36.1	36.1	36.1	36.5	36.5	29.1
LOS by Move:	C	C	C	D	B	B	D	D	D	D	D	C
HCM2kAvgQ:	0	23	23	14	5	5	0	0	0	0	0	14

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 0.739
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 30.3
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Ovl		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	1	0	0	1	0	0	1	0	0	0	1	0

Volume Module:

Base Vol:	1	466	14	380	663	1	1	1	2	21	2	241
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	1	466	14	380	663	1	1	1	2	21	2	241
Added Vol:	0	0	0	276	0	0	0	0	0	0	0	244
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	1	466	14	656	663	1	1	1	2	21	2	485
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	1	491	15	691	698	1	1	1	2	22	2	511
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	1	491	15	691	698	1	1	1	2	22	2	511
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	1	491	15	691	698	1	1	1	2	22	2	511

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	0.95	1.00	1.00	0.90	0.90	0.90	0.80	0.80	0.85
Lanes:	1.00	0.97	0.03	1.00	0.99	0.01	0.25	0.25	0.50	0.91	0.09	1.00
Final Sat.:	1805	1837	55	1805	1897	3	429	429	858	1393	133	1615

Capacity Analysis Module:

Vol/Sat:	0.00	0.27	0.27	0.38	0.37	0.37	0.00	0.00	0.00	0.02	0.02	0.32
Crit Moves:	****			****						****		
Green/Cycle:	0.14	0.31	0.31	0.44	0.61	0.61	0.15	0.15	0.15	0.15	0.15	0.59
Volume/Cap:	0.00	0.87	0.87	0.87	0.60	0.60	0.02	0.02	0.02	0.11	0.11	0.53
Delay/Veh:	44.6	52.1	52.1	40.1	15.2	15.2	43.5	43.5	43.5	44.3	44.3	15.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	44.6	52.1	52.1	40.1	15.2	15.2	43.5	43.5	43.5	44.3	44.3	15.2
LOS by Move:	D	D	D	D	B	B	D	D	D	D	D	B
HCM2kAvgQ:	0	20	20	26	16	16	0	0	0	1	1	11

Note: Queue reported is the number of cars per lane.

 Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 State Street (NS) at Ramona Expressway (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 0.894
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 56.0
 Optimal Cycle: OPTIMIZED Level Of Service: E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	28	28	10	28	28	10	24	24	10	24	24
Lanes:	1	0	1	1	0	1	1	0	1	1	0	1

Volume Module:

Base Vol:	379	683	101	109	453	56	159	279	422	252	502	307
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	379	683	101	109	453	56	159	279	422	252	502	307
Added Vol:	0	0	0	0	0	0	0	69	0	0	31	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	379	683	101	109	453	56	159	348	422	252	533	307
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	399	719	106	115	477	59	167	366	444	265	561	323
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	399	719	106	115	477	59	167	366	444	265	561	323
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	399	719	106	115	477	59	167	366	444	265	561	323

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.93	0.93	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85
Lanes:	1.00	1.74	0.26	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	1805	3085	456	1805	3610	1615	1805	3610	1615	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.22	0.23	0.23	0.06	0.13	0.04	0.09	0.10	0.28	0.15	0.16	0.20
Crit Moves:	****			****			****			****		
Green/Cycle:	0.22	0.33	0.33	0.12	0.23	0.23	0.13	0.27	0.27	0.14	0.28	0.28
Volume/Cap:	1.02	0.70	0.70	0.54	0.57	0.16	0.70	0.37	1.02	1.02	0.55	0.70
Delay/Veh:	96.3	36.8	36.8	52.4	41.5	36.8	59.1	35.7	90.7	111.0	37.0	43.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	96.3	36.8	36.8	52.4	41.5	36.8	59.1	35.7	90.7	111.0	37.0	43.4
LOS by Move:	F	D	D	D	D	D	E	D	F	F	D	D
HCM2kAvgQ:	21	15	15	5	9	2	7	6	23	15	10	12

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 State Street (NS) at Ramona Expressway (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 1.122
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 80.5
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	28	28	10	28	28	10	24	24	10	24	24
Lanes:	1	0	1	1	0	1	1	0	1	1	0	1

Volume Module:

Base Vol:	422	750	112	288	744	71	125	699	574	232	388	154
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	422	750	112	288	744	71	125	699	574	232	388	154
Added Vol:	0	0	0	0	0	0	0	92	0	0	81	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	422	750	112	288	744	71	125	791	574	232	469	154
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	444	789	118	303	783	75	132	833	604	244	494	162
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	444	789	118	303	783	75	132	833	604	244	494	162
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	444	789	118	303	783	75	132	833	604	244	494	162

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.93	0.93	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85
Lanes:	1.00	1.74	0.26	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	1805	3081	460	1805	3610	1615	1805	3610	1615	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.25	0.26	0.26	0.17	0.22	0.05	0.07	0.23	0.37	0.14	0.14	0.10
Crit Moves:	****			****			****			****		
Green/Cycle:	0.21	0.27	0.27	0.17	0.23	0.23	0.13	0.31	0.31	0.11	0.30	0.30
Volume/Cap:	1.19	0.96	0.96	0.96	0.93	0.20	0.58	0.74	1.19	1.19	0.45	0.33
Delay/Veh:	158.0	64.7	64.7	90.4	61.5	37.2	53.2	39.3	146.2	177.9	34.2	33.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	158.0	64.7	64.7	90.4	61.5	37.2	53.2	39.3	146.2	177.9	34.2	33.0
LOS by Move:	F	E	E	F	E	D	D	D	F	F	C	C
HCM2kAvgQ:	28	22	22	16	19	2	5	16	37	17	8	5

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 State Street (NS) at Ramona Expressway (EW)

Cycle (sec): 115 Critical Vol./Cap.(X): 0.699
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 42.6
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Ovl			Include		
Min. Green:	10	28	28	10	28	28	10	24	24	10	24	24
Lanes:	1	0	2	0	1	1	1	0	2	0	1	1

Volume Module:

Base Vol:	379	683	101	109	453	56	159	279	422	252	502	307
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	379	683	101	109	453	56	159	279	422	252	502	307
Added Vol:	0	0	0	0	0	0	0	69	0	0	31	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	379	683	101	109	453	56	159	348	422	252	533	307
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	399	719	106	115	477	59	167	366	444	265	561	323
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	399	719	106	115	477	59	167	366	444	265	561	323
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	399	719	106	115	477	59	167	366	444	265	561	323

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	1805	3610	1615	1805	3610	1615	1805	3610	1615	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.22	0.20	0.07	0.06	0.13	0.04	0.09	0.10	0.28	0.15	0.16	0.20
Crit Moves:	****			****			****			****		
Green/Cycle:	0.25	0.36	0.36	0.13	0.24	0.24	0.11	0.21	0.45	0.16	0.26	0.26
Volume/Cap:	0.90	0.55	0.18	0.49	0.54	0.15	0.81	0.49	0.61	0.90	0.60	0.78
Delay/Veh:	63.0	29.9	25.3	48.3	38.6	34.3	70.6	40.6	25.1	75.6	38.7	48.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	63.0	29.9	25.3	48.3	38.6	34.3	70.6	40.6	25.1	75.6	38.7	48.6
LOS by Move:	E	C	C	D	D	C	E	D	C	E	D	D
HCM2kAvgQ:	17	11	3	4	8	2	8	6	12	13	10	12

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 State Street (NS) at Ramona Expressway (EW)

Cycle (sec): 115 Critical Vol./Cap.(X): 0.963
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 54.3
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Ovl			Include		
Min. Green:	10	28	28	10	28	28	10	24	24	10	24	24
Lanes:	1	0	2	0	1	1	1	0	2	0	1	1

Volume Module:

Base Vol:	422	750	112	288	744	71	125	699	574	232	388	154
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	422	750	112	288	744	71	125	699	574	232	388	154
Added Vol:	0	0	0	0	0	0	0	92	0	0	81	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	422	750	112	288	744	71	125	791	574	232	469	154
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	444	789	118	303	783	75	132	833	604	244	494	162
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	444	789	118	303	783	75	132	833	604	244	494	162
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	444	789	118	303	783	75	132	833	604	244	494	162

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	1805	3610	1615	1805	3610	1615	1805	3610	1615	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.25	0.22	0.07	0.17	0.22	0.05	0.07	0.23	0.37	0.14	0.14	0.10
Crit Moves:	****			****			****			****		
Green/Cycle:	0.25	0.29	0.29	0.20	0.24	0.24	0.11	0.23	0.48	0.14	0.26	0.26
Volume/Cap:	0.99	0.75	0.25	0.84	0.89	0.19	0.67	0.99	0.78	0.99	0.52	0.39
Delay/Veh:	83.2	40.1	31.5	59.7	53.3	34.7	58.0	72.8	29.8	104.4	37.0	35.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	83.2	40.1	31.5	59.7	53.3	34.7	58.0	72.8	29.8	104.4	37.0	35.5
LOS by Move:	F	D	C	E	D	C	E	E	C	F	D	D
HCM2kAvgQ:	21	15	3	13	17	2	6	21	19	13	8	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.858
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 32.2
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control (Permitted/Protected), Rights (Include), Min. Green, and Lanes.

Volume Module: Table with 12 columns for traffic volumes and adjustments. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with 12 columns for saturation flow rates. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with 12 columns for capacity and delay metrics. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 1.273
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 91.8
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	28	28	28	28	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	1	0	1	1	0	1

Volume Module:

Base Vol:	138	581	125	184	712	392	195	1075	172	144	1039	126
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	138	581	125	184	712	392	195	1075	172	144	1039	126
Added Vol:	0	0	46	0	0	0	0	218	0	41	192	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	138	581	171	184	712	392	195	1293	172	185	1231	126
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	145	612	180	194	749	413	205	1361	181	195	1296	133
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	145	612	180	194	749	413	205	1361	181	195	1296	133
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	145	612	180	194	749	413	205	1361	181	195	1296	133

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.14	0.92	0.92	0.23	1.00	0.85	0.95	0.93	0.93	0.95	0.94	0.94
Lanes:	1.00	1.55	0.45	1.00	1.00	1.00	1.00	1.77	0.23	1.00	1.81	0.19
Final Sat.:	272	2694	793	437	1900	1615	1805	3129	416	1805	3229	331

Capacity Analysis Module:

Vol/Sat:	0.53	0.23	0.23	0.44	0.39	0.26	0.11	0.44	0.44	0.11	0.40	0.40
Crit Moves:	****						****			****		
Green/Cycle:	0.36	0.36	0.36	0.36	0.36	0.36	0.13	0.36	0.36	0.13	0.36	0.36
Volume/Cap:	1.49	0.63	0.63	1.23	1.10	0.71	0.89	1.21	1.21	0.84	1.12	1.12
Delay/Veh:	291.6	21.8	21.8	173.5	89.7	25.7	64.5	128	127.8	56.5	89.1	89.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	291.6	21.8	21.8	173.5	89.7	25.7	64.5	128	127.8	56.5	89.1	89.1
LOS by Move:	F	C	C	F	F	C	E	F	F	E	F	F
HCM2kAvgQ:	11	9	9	12	31	10	8	40	40	7	32	32

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.820
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 30.8
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	28	28	28	28	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	1	0	1	1	0	2

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	103	698	118	130	537	239	220	627	120	73	729	97
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	103	698	118	130	537	239	220	627	120	73	729	97
Added Vol:	0	0	35	0	0	0	0	164	0	16	70	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	103	698	153	130	537	239	220	791	120	89	799	97
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	108	735	161	137	565	252	232	833	126	94	841	102
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	108	735	161	137	565	252	232	833	126	94	841	102
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	108	735	161	137	565	252	232	833	126	94	841	102

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.17	0.92	0.92	0.21	1.00	0.85	0.95	0.93	0.93	0.95	0.95	0.85
Lanes:	1.00	1.64	0.36	1.00	1.00	1.00	1.00	1.74	0.26	1.00	2.00	1.00
Final Sat.:	323	2881	632	391	1900	1615	1805	3072	466	1805	3610	1615

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.34	0.26	0.26	0.35	0.30	0.16	0.13	0.27	0.27	0.05	0.23	0.06
Crit Moves:				****				****				
Green/Cycle:	0.41	0.41	0.41	0.41	0.41	0.41	0.15	0.34	0.34	0.12	0.31	0.31
Volume/Cap:	0.83	0.63	0.63	0.86	0.73	0.38	0.86	0.80	0.80	0.43	0.75	0.20
Delay/Veh:	56.9	22.2	22.2	59.2	26.2	19.2	60.7	30.9	30.9	38.0	30.7	23.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	56.9	22.2	22.2	59.2	26.2	19.2	60.7	30.9	30.9	38.0	30.7	23.0
LOS by Move:	E	C	C	E	C	B	E	C	C	D	C	C
HCM2kAvgQ:	5	11	11	6	14	5	9	15	15	3	13	2

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 State Street (NS) at Florida Avenue (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 0.995
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 49.4
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	28	28	28	28	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	1	0	2	1	0	2

Volume Module:

Base Vol:	138	581	125	184	712	392	195	1075	172	144	1039	126
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	138	581	125	184	712	392	195	1075	172	144	1039	126
Added Vol:	0	0	46	0	0	0	0	218	0	41	192	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	138	581	171	184	712	392	195	1293	172	185	1231	126
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	145	612	180	194	749	413	205	1361	181	195	1296	133
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	145	612	180	194	749	413	205	1361	181	195	1296	133
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	145	612	180	194	749	413	205	1361	181	195	1296	133

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.18	0.92	0.92	0.31	1.00	0.85	0.95	0.89	0.89	0.95	0.95	0.85
Lanes:	1.00	1.55	0.45	1.00	1.00	1.00	1.00	2.65	0.35	1.00	2.00	1.00
Final Sat.:	343	2694	793	585	1900	1615	1805	4496	598	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.42	0.23	0.23	0.33	0.39	0.26	0.11	0.30	0.30	0.11	0.36	0.08
Crit Moves:	****			****			****			****		
Green/Cycle:	0.43	0.43	0.43	0.43	0.43	0.43	0.11	0.35	0.35	0.12	0.36	0.36
Volume/Cap:	1.00	0.53	0.53	0.78	0.93	0.60	1.00	0.86	0.86	0.86	1.00	0.23
Delay/Veh:	107.4	26.0	26.0	44.1	49.5	28.1	114.4	41.0	41.0	79.1	62.0	26.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	107.4	26.0	26.0	44.1	49.5	28.1	114.4	41.0	41.0	79.1	62.0	26.9
LOS by Move:	F	C	C	D	D	C	F	D	D	E	E	C
HCM2kAvgQ:	9	11	11	8	30	12	12	22	22	10	32	3

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

Cycle (sec): 115 Critical Vol./Cap.(X): 1.411
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 160.5
Optimal Cycle: OPTIMIZED Level Of Service: F

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Split Phase, Permitted), Rights (Include), and Lane counts.

Volume Module table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume) across four approaches.

Saturation Flow Module table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across four approaches.

Capacity Analysis Module table with columns for Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ across four approaches.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 2.457

Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 445.1

Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	18	18	18	18	21	21	21	21	21	21
Lanes:	1	0	1	0	0	1	0	1	0	0	1	0

Volume Module:

Base Vol:	452	436	171	81	522	49	50	356	452	179	304	54
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	452	436	171	81	522	49	50	356	452	179	304	54
Added Vol:	0	0	356	92	0	0	0	0	0	314	0	81
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	452	436	527	173	522	49	50	356	452	493	304	135
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	476	459	555	182	549	52	53	375	476	519	320	142
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	476	459	555	182	549	52	53	375	476	519	320	142
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	476	459	555	182	549	52	53	375	476	519	320	142

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.93	0.93	0.93	0.95	0.99	0.99	0.86	0.86	0.85	0.44	0.44	0.44
Lanes:	1.19	0.37	0.44	1.00	0.91	0.09	0.12	0.88	1.00	0.53	0.33	0.14
Final Sat.:	2100	647	782	1805	1714	161	201	1433	1615	439	271	120

Capacity Analysis Module:

Vol/Sat:	0.23	0.71	0.71	0.10	0.32	0.32	0.26	0.26	0.29	1.18	1.18	1.18
Crit Moves:	****			****						****		
Green/Cycle:	0.28	0.28	0.28	0.15	0.15	0.15	0.47	0.47	0.47	0.47	0.47	0.47
Volume/Cap:	0.81	2.52	2.52	0.67	2.14	2.14	0.56	0.56	0.63	2.52	2.52	2.52
Delay/Veh:	42.8	732	732.4	54.7	574	574.3	23.9	23.9	25.7	723.8	724	723.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	42.8	732	732.4	54.7	574	574.3	23.9	23.9	25.7	723.8	724	723.8
LOS by Move:	D	F	F	D	F	F	C	C	C	F	F	F
HCM2kAvgQ:	16	138	138	8	62	62	11	11	14	108	108	108

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

 Cycle (sec): 90 Critical Vol./Cap.(X): 0.673
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 32.1
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ovl			Include			Ovl			Include		
Min. Green:	18	18	18	18	18	18	10	21	21	10	21	21
Lanes:	1	0	1	0	1	0	1	0	1	0	1	0

Volume Module:

Base Vol:	337	375	84	30	541	29	18	231	348	49	168	22
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	337	375	84	30	541	29	18	231	348	49	168	22
Added Vol:	0	0	267	69	0	0	0	0	0	117	0	31
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	337	375	351	99	541	29	18	231	348	166	168	53
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	355	395	369	104	569	31	19	243	366	175	177	56
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	355	395	369	104	569	31	19	243	366	175	177	56
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	355	395	369	104	569	31	19	243	366	175	177	56

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	0.85	0.95	0.94	0.94	0.95	1.00	0.85	0.92	0.96	0.96
Lanes:	1.00	1.00	1.00	1.00	1.90	0.10	1.00	1.00	1.00	2.00	0.76	0.24
Final Sat.:	1805	1900	1615	1805	3399	182	1805	1900	1615	3502	1392	439

Capacity Analysis Module:

Vol/Sat:	0.20	0.21	0.23	0.06	0.17	0.17	0.01	0.13	0.23	0.05	0.13	0.13
Crit Moves:	****			****			****			****		
Green/Cycle:	0.26	0.26	0.38	0.21	0.21	0.21	0.11	0.23	0.50	0.11	0.23	0.23
Volume/Cap:	0.74	0.79	0.61	0.27	0.79	0.79	0.09	0.55	0.46	0.45	0.54	0.54
Delay/Veh:	36.5	38.7	24.6	29.9	38.8	38.8	36.1	31.8	15.1	38.2	31.8	31.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	36.5	38.7	24.6	29.9	38.8	38.8	36.1	31.8	15.1	38.2	31.8	31.8
LOS by Move:	D	D	C	C	D	D	D	C	B	D	C	C
HCM2kAvgQ:	11	12	9	3	10	10	1	7	7	3	6	6

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

 Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

 Cycle (sec): 100 Critical Vol./Cap.(X): 0.926
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 49.5
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ovl			Include			Ovl			Include		
Min. Green:	18	18	18	18	18	18	10	21	21	10	21	21
Lanes:	1	0	1	0	1	0	1	0	1	0	1	0

Volume Module:

Base Vol:	452	436	171	81	522	49	50	356	452	179	304	54
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	452	436	171	81	522	49	50	356	452	179	304	54
Added Vol:	0	0	356	92	0	0	0	0	0	314	0	81
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	452	436	527	173	522	49	50	356	452	493	304	135
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	476	459	555	182	549	52	53	375	476	519	320	142
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	476	459	555	182	549	52	53	375	476	519	320	142
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	476	459	555	182	549	52	53	375	476	519	320	142

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	0.85	0.95	0.94	0.94	0.95	1.00	0.85	0.92	0.95	0.95
Lanes:	1.00	1.00	1.00	1.00	1.83	0.17	1.00	1.00	1.00	2.00	0.69	0.31
Final Sat.:	1805	1900	1615	1805	3257	306	1805	1900	1615	3502	1255	557

Capacity Analysis Module:

Vol/Sat:	0.26	0.24	0.34	0.10	0.17	0.17	0.03	0.20	0.29	0.15	0.25	0.25
Crit Moves:	****			****			****			****		
Green/Cycle:	0.28	0.28	0.44	0.18	0.18	0.18	0.11	0.21	0.50	0.16	0.27	0.27
Volume/Cap:	0.93	0.85	0.77	0.55	0.93	0.93	0.28	0.93	0.59	0.93	0.95	0.95
Delay/Veh:	57.6	45.7	28.7	39.2	59.5	59.5	42.0	65.6	19.1	62.9	64.8	64.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	57.6	45.7	28.7	39.2	59.5	59.5	42.0	65.6	19.1	62.9	64.8	64.8
LOS by Move:	E	D	C	D	E	E	D	E	B	E	E	E
HCM2kAvgQ:	19	16	16	6	14	14	2	15	11	12	19	19

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #6 San Jacinto Street (NS) at Esplanade Avenue (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.655
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 31.6
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	28	28	10	28	28	10	28	28	10	28	28
Lanes:	1	0	2	0	1	1	0	1	1	0	1	1

Volume Module:

Base Vol:	136	430	95	128	418	148	105	405	187	109	483	96
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	136	430	95	128	418	148	105	405	187	109	483	96
Added Vol:	0	233	0	0	101	16	35	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	136	663	95	128	519	164	140	405	187	109	483	96
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	143	698	100	135	546	173	147	426	197	115	508	101
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	143	698	100	135	546	173	147	426	197	115	508	101
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	143	698	100	135	546	173	147	426	197	115	508	101

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.92	0.92	0.95	0.95	0.85	0.95	0.93	0.93
Lanes:	1.00	2.00	1.00	1.00	1.52	0.48	1.00	2.00	1.00	1.00	1.67	0.33
Final Sat.:	1805	3610	1615	1805	2644	836	1805	3610	1615	1805	2936	584

Capacity Analysis Module:

Vol/Sat:	0.08	0.19	0.06	0.07	0.21	0.21	0.08	0.12	0.12	0.06	0.17	0.17
Crit Moves:	****			****			****			****		
Green/Cycle:	0.11	0.30	0.30	0.11	0.30	0.30	0.11	0.30	0.30	0.11	0.30	0.30
Volume/Cap:	0.73	0.64	0.20	0.69	0.68	0.68	0.75	0.39	0.40	0.58	0.57	0.57
Delay/Veh:	52.7	28.8	23.9	49.3	29.9	29.9	54.7	25.5	25.9	43.5	27.7	27.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	52.7	28.8	23.9	49.3	29.9	29.9	54.7	25.5	25.9	43.5	27.7	27.7
LOS by Move:	D	C	C	D	C	C	D	C	C	D	C	C
HCM2kAvgQ:	6	10	2	5	10	10	6	5	5	4	8	8

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #6 San Jacinto Street (NS) at Esplanade Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.900
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 53.2
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green (10, 28, 28), Lanes (1 0 2 0 1).

Volume Module: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #7 San Jacinto Street (NS) at Menlo Avenue (EW)

Cycle (sec): 70 Critical Vol./Cap.(X): 0.653
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 23.8
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	18	10	18	18	28	28	28	28	28	28
Lanes:	1	0	1	1	0	1	1	0	0	1	0	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	87	463	58	41	533	101	82	363	110	68	473	84
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	87	463	58	41	533	101	82	363	110	68	473	84
Added Vol:	0	233	0	0	101	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	87	696	58	41	634	101	82	363	110	68	473	84
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	92	733	61	43	667	106	86	382	116	72	498	88
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	92	733	61	43	667	106	86	382	116	72	498	88
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	92	733	61	43	667	106	86	382	116	72	498	88

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.94	0.94	0.95	0.93	0.93	0.27	0.97	0.97	0.27	1.00	0.85
Lanes:	1.00	1.85	0.15	1.00	1.73	0.27	1.00	0.77	0.23	1.00	1.00	1.00
Final Sat.:	1805	3292	274	1805	3049	486	517	1407	426	517	1900	1615

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.05	0.22	0.22	0.02	0.22	0.22	0.17	0.27	0.27	0.14	0.26	0.05
Crit Moves:	****			****			****					
Green/Cycle:	0.14	0.28	0.28	0.15	0.29	0.29	0.40	0.40	0.40	0.40	0.40	0.40
Volume/Cap:	0.36	0.81	0.81	0.16	0.77	0.77	0.42	0.68	0.68	0.35	0.66	0.14
Delay/Veh:	27.9	28.7	28.7	26.0	26.4	26.4	16.5	19.9	19.9	15.6	19.2	13.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	27.9	28.7	28.7	26.0	26.4	26.4	16.5	19.9	19.9	15.6	19.2	13.4
LOS by Move:	C	C	C	C	C	C	B	B	B	B	B	B
HCM2kAvgQ:	2	11	11	1	10	10	2	10	10	1	10	1

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #7 San Jacinto Street (NS) at Menlo Avenue (EW)

Cycle (sec): 105 Critical Vol./Cap.(X): 0.861
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 37.5
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, Min. Green, Lanes.

Table with columns: Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Table with columns: Saturation Flow Module, Sat/Lane, Adjustment, Lanes, Final Sat.

Table with columns: Capacity Analysis Module, Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #8 San Jacinto Street (NS) at Devonshire Avenue (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.726
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 25.9
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	18	10	18	18	28	28	28	28	28	28
Lanes:	1	0	1	1	0	1	1	0	1	1	0	1

Volume Module:

Base Vol:	134	471	90	81	554	145	100	137	97	135	415	97
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	134	471	90	81	554	145	100	137	97	135	415	97
Added Vol:	0	233	0	0	101	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	134	704	90	81	655	145	100	137	97	135	415	97
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	141	741	95	85	689	153	105	144	102	142	437	102
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	141	741	95	85	689	153	105	144	102	142	437	102
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	141	741	95	85	689	153	105	144	102	142	437	102

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.93	0.93	0.95	0.92	0.92	0.21	1.00	0.85	0.66	0.97	0.97
Lanes:	1.00	1.77	0.23	1.00	1.64	0.36	1.00	1.00	1.00	1.00	0.81	0.19
Final Sat.:	1805	3146	402	1805	2876	637	405	1900	1615	1254	1497	350

Capacity Analysis Module:

Vol/Sat:	0.08	0.24	0.24	0.05	0.24	0.24	0.26	0.08	0.06	0.11	0.29	0.29
Crit Moves:	****			****						****		
Green/Cycle:	0.13	0.29	0.29	0.16	0.32	0.32	0.39	0.39	0.39	0.39	0.39	0.39
Volume/Cap:	0.59	0.81	0.81	0.29	0.75	0.75	0.67	0.20	0.16	0.29	0.75	0.75
Delay/Veh:	34.3	29.6	29.6	28.2	25.8	25.8	29.7	15.3	15.1	16.2	24.3	24.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	34.3	29.6	29.6	28.2	25.8	25.8	29.7	15.3	15.1	16.2	24.3	24.3
LOS by Move:	C	C	C	C	C	C	C	B	B	B	C	C
HCM2kAvgQ:	4	12	12	2	11	11	3	2	2	2	12	12

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #8 San Jacinto Street (NS) at Devonshire Avenue (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.879
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 32.4
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns for volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume).

Saturation Flow Module: Table with 12 columns for saturation flow metrics (Sat/Lane, Adjustment, Lanes, Final Sat.).

Capacity Analysis Module: Table with 12 columns for capacity analysis metrics (Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ).

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.939
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 121.3
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	28	28	28	28	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	1	0	1	0	1	1	0	2

Volume Module:

Base Vol:	81	259	77	163	241	149	200	702	43	97	1114	202
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	81	259	77	163	241	149	200	702	43	97	1114	202
Added Vol:	0	35	0	0	16	85	198	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	81	294	77	163	257	234	398	702	43	97	1114	202
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	85	309	81	172	271	246	419	739	45	102	1173	213
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	85	309	81	172	271	246	419	739	45	102	1173	213
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	85	309	81	172	271	246	419	739	45	102	1173	213

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.92	0.92	0.98	0.98	0.85	0.95	0.94	0.94	0.95	0.95	0.85
Lanes:	1.00	1.58	0.42	1.00	1.00	1.00	1.00	1.88	0.12	1.00	2.00	1.00
Final Sat.:	1805	2772	726	1864	1864	1615	1805	3371	206	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.05	0.11	0.11	0.09	0.15	0.15	0.23	0.22	0.22	0.06	0.32	0.13
Crit Moves:	****			****			****			****		
Green/Cycle:	0.23	0.23	0.23	0.23	0.23	0.23	0.17	0.29	0.29	0.11	0.23	0.23
Volume/Cap:	0.20	0.48	0.48	0.39	0.62	0.65	1.39	0.74	0.74	0.54	1.39	0.56
Delay/Veh:	37.3	40.1	40.1	39.1	43.0	45.7	245.8	41.1	41.1	53.9	230	42.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	37.3	40.1	40.1	39.1	43.0	45.7	245.8	41.1	41.1	53.9	230	42.6
LOS by Move:	D	D	D	D	D	D	F	D	D	D	F	D
HCM2kAvgQ:	3	7	7	5	10	9	32	15	15	4	45	7

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.249
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 174.9
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	28	28	28	28	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	1	0	1	0	1	1	0	2

Volume Module:

Base Vol:	79	349	144	457	338	208	223	1167	108	172	1193	75
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	79	349	144	457	338	208	223	1167	108	172	1193	75
Added Vol:	0	46	0	0	41	232	264	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	79	395	144	457	379	440	487	1167	108	172	1193	75
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	83	416	152	481	399	463	513	1228	114	181	1256	79
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	83	416	152	481	399	463	513	1228	114	181	1256	79
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	83	416	152	481	399	463	513	1228	114	181	1256	79

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	0.91	0.97	0.97	0.85	0.95	0.94	0.94	0.95	0.95	0.85
Lanes:	1.00	1.47	0.53	1.09	0.91	1.00	1.00	1.83	0.17	1.00	2.00	1.00
Final Sat.:	1805	2540	926	2021	1676	1615	1805	3261	302	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.05	0.16	0.16	0.24	0.24	0.29	0.28	0.38	0.38	0.10	0.35	0.05
Crit Moves:	****			****			****			****		
Green/Cycle:	0.23	0.23	0.23	0.23	0.23	0.23	0.17	0.32	0.32	0.08	0.23	0.23
Volume/Cap:	0.20	0.70	0.70	1.02	1.02	1.23	1.70	1.19	1.19	1.19	1.49	0.21
Delay/Veh:	37.2	45.0	45.0	81.8	81.8	170.3	380.7	137	136.6	189.0	273	37.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	37.2	45.0	45.0	81.8	81.8	170.3	380.7	137	136.6	189.0	273	37.4
LOS by Move:	D	D	D	F	F	F	F	F	F	F	F	D
HCM2kAvgQ:	2	11	11	23	23	30	46	42	42	13	52	2

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
Year 2025 With Project
Morning Peak Hour - With Improvements

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

Cycle (sec): 105 Critical Vol./Cap.(X): 0.687
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 35.3
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L, T, R), Control (Protected), Rights (Include, Ovl), Min. Green, and Lanes.

Volume Module: Table with 12 columns representing different traffic movements and 12 rows of volume-related metrics such as Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module: Table with 12 columns and 4 rows showing Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with 12 columns and 10 rows showing Vol/Sat, Crit Moves, Green/Cycle, Delay/Veh, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.898
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 51.8
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	10	28	28	10	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	0	1	1	0	1	0

Volume Module:

Base Vol:	79	349	144	457	338	208	223	1167	108	172	1193	75
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	79	349	144	457	338	208	223	1167	108	172	1193	75
Added Vol:	0	46	0	0	41	232	264	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	79	395	144	457	379	440	487	1167	108	172	1193	75
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	83	416	152	481	399	463	513	1228	114	181	1256	79
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	83	416	152	481	399	463	513	1228	114	181	1256	79
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	83	416	152	481	399	463	513	1228	114	181	1256	79

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	0.91	0.92	1.00	0.85	0.92	0.94	0.94	0.95	0.90	0.90
Lanes:	1.00	1.47	0.53	2.00	1.00	1.00	2.00	1.83	0.17	1.00	2.82	0.18
Final Sat.:	1805	2540	926	3502	1900	1615	3502	3261	302	1805	4836	304

Capacity Analysis Module:

Vol/Sat:	0.05	0.16	0.16	0.14	0.21	0.29	0.15	0.38	0.38	0.10	0.26	0.26
Crit Moves:	****			****			****			****		
Green/Cycle:	0.10	0.23	0.23	0.14	0.28	0.45	0.18	0.39	0.39	0.10	0.31	0.31
Volume/Cap:	0.47	0.70	0.70	0.97	0.76	0.63	0.83	0.97	0.97	0.97	0.83	0.83
Delay/Veh:	53.0	45.0	45.0	83.9	46.2	26.9	56.5	53.5	53.5	110.5	41.7	41.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	53.0	45.0	45.0	83.9	46.2	26.9	56.5	53.5	53.5	110.5	41.7	41.7
LOS by Move:	D	D	D	F	D	C	E	D	D	F	D	D
HCM2kAvgQ:	3	11	11	13	15	13	12	31	31	10	19	19

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

Cycle (sec): 110 Critical Vol./Cap.(X): 0.863
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 44.0
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns for volume metrics. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with 12 columns for saturation flow metrics. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with 12 columns for capacity analysis metrics. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.278

Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 108.9

Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	24	24	10	24	24	10	28	28	10	28	28
Lanes:	1	0	2	0	1	1	0	1	1	0	1	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	86	406	375	182	702	23	23	344	86	560	378	157
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	86	406	375	182	702	23	23	344	86	560	378	157
Added Vol:	0	0	100	92	0	0	0	448	0	89	395	81
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	86	406	475	274	702	23	23	792	86	649	773	238
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	91	427	500	288	739	24	24	834	91	683	814	251
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	91	427	500	288	739	24	24	834	91	683	814	251
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	91	427	500	288	739	24	24	834	91	683	814	251

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.95	0.95	0.95	0.94	0.94	0.95	0.92	0.92
Lanes:	1.00	2.00	1.00	1.00	1.94	0.06	1.00	1.80	0.20	1.00	1.53	0.47
Final Sat.:	1805	3610	1615	1805	3478	114	1805	3208	348	1805	2664	820

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.05	0.12	0.31	0.16	0.21	0.21	0.01	0.26	0.26	0.38	0.31	0.31
Crit Moves:	****			****			****			****		
Green/Cycle:	0.10	0.23	0.23	0.12	0.25	0.25	0.11	0.23	0.23	0.28	0.41	0.41
Volume/Cap:	0.51	0.51	1.34	1.34	0.84	0.84	0.12	1.11	1.11	1.34	0.75	0.75
Delay/Veh:	53.7	40.8	215.7	233.2	49.9	49.9	48.4	113	113.4	208.3	32.9	32.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	53.7	40.8	215.7	233.2	49.9	49.9	48.4	113	113.4	208.3	32.9	32.9
LOS by Move:	D	D	F	F	D	D	D	F	F	F	C	C
HCM2kAvgQ:	4	8	36	22	17	17	1	28	28	48	19	19

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

 Cycle (sec): 95 Critical Vol./Cap.(X): 0.692
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 31.4
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Include		
Min. Green:	10	24	24	10	24	24	10	28	28	10	28	28
Lanes:	1	0	2	0	1	1	0	1	1	0	2	0

Volume Module:

Base Vol:	64	848	367	98	436	37	42	238	95	225	171	40
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	64	848	367	98	436	37	42	238	95	225	171	40
Added Vol:	0	0	75	69	0	0	0	336	0	37	148	31
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	64	848	442	167	436	37	42	574	95	262	319	71
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	67	893	465	176	459	39	44	604	100	276	336	75
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	67	893	465	176	459	39	44	604	100	276	336	75
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	67	893	465	176	459	39	44	604	100	276	336	75

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.92	0.94	0.94	0.95	0.93	0.93	0.92	0.92	0.92
Lanes:	1.00	2.00	1.00	2.00	1.84	0.16	1.00	1.72	0.28	2.00	1.64	0.36
Final Sat.:	1805	3610	1615	3502	3288	279	1805	3032	502	3502	2873	639

Capacity Analysis Module:

Vol/Sat:	0.04	0.25	0.29	0.05	0.14	0.14	0.02	0.20	0.20	0.08	0.12	0.12
Crit Moves:	****			****			****			****		
Green/Cycle:	0.13	0.33	0.43	0.11	0.30	0.30	0.11	0.29	0.29	0.11	0.29	0.29
Volume/Cap:	0.29	0.76	0.67	0.48	0.46	0.46	0.23	0.68	0.68	0.75	0.40	0.40
Delay/Veh:	38.3	31.5	24.0	41.0	27.0	27.0	39.6	31.3	31.3	49.5	27.0	27.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	38.3	31.5	24.0	41.0	27.0	27.0	39.6	31.3	31.3	49.5	27.0	27.0
LOS by Move:	D	C	C	D	C	C	D	C	C	D	C	C
HCM2kAvgQ:	2	14	12	3	6	6	1	11	11	6	5	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
Year 2025 With Project
Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.854
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 41.5
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, Min. Green, Lanes.

Table with columns: Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Table with columns: Saturation Flow Module, Sat/Lane, Adjustment, Lanes, Final Sat.

Table with columns: Capacity Analysis Module, Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

Average Delay (sec/veh): 21.8 Worst Case Level Of Service: F[251.6]

Approach:	North Bound			South Bound			East Bound			West Bound										
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign										
Rights:	Include			Include			Include			Include										
Lanes:	1	0	2	0	1	1	0	2	0	1	0	1	0	0	1	0	0	1	0	0

Volume Module:

Base Vol:	136	1143	8	34	614	106	71	2	113	3	5	1
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	136	1143	8	34	614	106	71	2	113	3	5	1
Added Vol:	0	72	0	0	34	3	3	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	136	1215	8	34	648	109	74	2	113	3	5	1
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	143	1279	8	36	682	115	78	2	119	3	5	1
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	143	1279	8	36	682	115	78	2	119	3	5	1

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	4.1	xxxx	xxxxxx	7.5	6.5	6.9	7.5	6.5	6.9
FollowUpTim:	2.2	xxxx	xxxxxx	2.2	xxxx	xxxxxx	3.5	4.0	3.3	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	797	xxxx	xxxxxx	1287	xxxx	xxxxxx	1682	2327	341	1979	2434	639
Potent Cap.:	834	xxxx	xxxxxx	546	xxxx	xxxxxx	63	38	661	38	32	423
Move Cap.:	834	xxxx	xxxxxx	546	xxxx	xxxxxx	44	29	661	24	25	423
Volume/Cap:	0.17	xxxx	xxxx	0.07	xxxx	xxxx	1.78	0.07	0.18	0.13	0.21	0.00

Level Of Service Module:

2Way95thQ:	0.6	xxxx	xxxxxx	0.2	xxxx	xxxxxx	xxxx	xxxx	0.7	xxxx	xxxx	xxxxxx
Control Del:	10.2	xxxx	xxxxxx	12.1	xxxx	xxxxxx	xxxxxx	xxxx	11.6	xxxxxx	xxxx	xxxxxx
LOS by Move:	B	*	*	B	*	*	*	*	B	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	43	xxxx	xxxxxx	xxxx	27	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	8.3	xxxx	xxxxxx	xxxxxx	1.1	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	608.4	xxxx	xxxxxx	xxxxxx	194	xxxxxx
Shared LOS:	*	*	*	*	*	*	F	*	*	*	F	*
ApproachDel:	xxxxxx			xxxxxx			251.6			193.7		
ApproachLOS:	*			*			F			F		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

Average Delay (sec/veh): 107.9 Worst Case Level Of Service: F[637.4]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume metrics across four approaches.

Critical Gap Module: Table with 12 columns for critical gap and follow-up time metrics.

Capacity Module: Table with 12 columns for capacity metrics such as conflict volume and potential capacity.

Level Of Service Module: Table with 12 columns for LOS metrics including delay, LOS by move, and shared queue.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 0.458
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 10.3
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	18	18	18	18	28	28	28	28	28	28
Lanes:	1	0	2	0	1	0	0	1	0	0	0	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	136	1143	8	34	614	106	71	2	113	3	5	1
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	136	1143	8	34	614	106	71	2	113	3	5	1
Added Vol:	0	72	0	0	34	3	3	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	136	1215	8	34	648	109	74	2	113	3	5	1
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	143	1279	8	36	682	115	78	2	119	3	5	1
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	143	1279	8	36	682	115	78	2	119	3	5	1
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	143	1279	8	36	682	115	78	2	119	3	5	1

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.36	0.95	0.85	0.17	0.95	0.85	0.72	0.72	0.85	0.94	0.94	0.94
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	0.97	0.03	1.00	0.33	0.56	0.11
Final Sat.:	690	3610	1615	327	3610	1615	1338	36	1615	594	990	198

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.21	0.35	0.01	0.11	0.19	0.07	0.06	0.06	0.07	0.01	0.01	0.01
Crit Moves:	****						****					
Green/Cycle:	0.70	0.70	0.70	0.70	0.70	0.70	0.23	0.23	0.23	0.23	0.23	0.23
Volume/Cap:	0.30	0.51	0.01	0.16	0.27	0.10	0.25	0.25	0.32	0.02	0.02	0.02
Delay/Veh:	7.2	8.5	5.4	6.4	6.7	5.9	37.9	37.9	38.6	35.5	35.5	35.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	7.2	8.5	5.4	6.4	6.7	5.9	37.9	37.9	38.6	35.5	35.5	35.5
LOS by Move:	A	A	A	A	A	A	D	D	D	D	D	D
HCM2kAvgQ:	2	11	0	1	5	1	2	2	4	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.735
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 14.8
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	18	18	18	18	28	28	28	28	28	28
Lanes:	1	0	2	0	1	0	0	1	0	0	0	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	109	649	1	61	1180	69	109	1	331	4	2	6
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	109	649	1	61	1180	69	109	1	331	4	2	6
Added Vol:	0	96	0	0	85	4	4	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	109	745	1	61	1265	73	113	1	331	4	2	6
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	115	784	1	64	1332	77	119	1	348	4	2	6
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	115	784	1	64	1332	77	119	1	348	4	2	6
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	115	784	1	64	1332	77	119	1	348	4	2	6

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.13	0.95	0.85	0.31	0.95	0.85	0.71	0.71	0.85	0.88	0.88	0.88
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	0.99	0.01	1.00	0.33	0.17	0.50
Final Sat.:	253	3610	1615	581	3610	1615	1332	12	1615	560	280	839

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.45	0.22	0.00	0.11	0.37	0.05	0.09	0.09	0.22	0.01	0.01	0.01
Crit Moves:	****						****					
Green/Cycle:	0.60	0.60	0.60	0.60	0.60	0.60	0.31	0.31	0.31	0.31	0.31	0.31
Volume/Cap:	0.76	0.36	0.00	0.18	0.61	0.08	0.29	0.29	0.69	0.02	0.02	0.02
Delay/Veh:	32.6	9.3	7.2	8.3	11.9	7.6	23.8	23.8	31.4	21.5	21.5	21.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	32.6	9.3	7.2	8.3	11.9	7.6	23.8	23.8	31.4	21.5	21.5	21.5
LOS by Move:	C	A	A	A	B	A	C	C	C	C	C	C
HCM2kAvgQ:	4	6	0	1	13	1	3	3	10	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #12 Mountain Avenue (NS) at Esplanade Avenue (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.900
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 31.5
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, Min. Green, Lanes.

Volume Module: Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module: Table with columns: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module: Table with columns: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #12 Mountain Avenue (NS) at Esplanade Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.086
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 66.2
 Optimal Cycle: OPTIMIZED Level Of Service: E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	24	0	0	24	24	21	0	21	0	0	0
Lanes:	1	0	1	0	0	1	0	0	1	0	0	0

-----|-----|-----|-----|

Volume Module:

Base Vol:	244	462	0	0	1003	401	249	0	360	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	244	462	0	0	1003	401	249	0	360	0	0	0
Added Vol:	0	92	0	0	81	4	4	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	244	554	0	0	1084	405	253	0	360	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	257	583	0	0	1141	426	266	0	379	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	257	583	0	0	1141	426	266	0	379	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	257	583	0	0	1141	426	266	0	379	0	0	0

-----|-----|-----|-----|

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	1.00	0.85	0.77	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	1900	1615	1461	0	1615	0	0	0

-----|-----|-----|-----|

Capacity Analysis Module:

Vol/Sat:	0.14	0.31	0.00	0.00	0.60	0.26	0.18	0.00	0.23	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.13	0.68	0.00	0.00	0.55	0.55	0.22	0.00	0.22	0.00	0.00	0.00
Volume/Cap:	1.09	0.45	0.00	0.00	1.09	0.48	0.84	0.00	1.09	0.00	0.00	0.00
Delay/Veh:	135.5	8.9	0.0	0.0	81.1	16.7	63.4	0.0	120.3	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	135.5	8.9	0.0	0.0	81.1	16.7	63.4	0.0	120.3	0.0	0.0	0.0
LOS by Move:	F	A	A	A	F	B	E	A	F	A	A	A
HCM2kAvgQ:	16	10	0	0	57	10	12	0	22	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

Average Delay (sec/veh): 36.0 Worst Case Level Of Service: F[198.1]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	0	0	0	0	0	0	1	0	0

Volume Module:

Base Vol:	194	0	24	0	0	0	0	359	109	75	548	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	194	0	24	0	0	0	0	359	109	75	548	0
Added Vol:	35	0	0	0	0	0	0	16	16	0	35	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	229	0	24	0	0	0	0	375	125	75	583	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	241	0	25	0	0	0	0	395	132	79	614	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	241	0	25	0	0	0	0	395	132	79	614	0

Critical Gap Module:

Critical Gp:	6.4	xxxx	6.2	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	4.1	xxxx	xxxxx
FollowUpTim:	3.5	xxxx	3.3	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	2.2	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	1232	xxxx	461	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	526	xxxx	xxxxx
Potent Cap.:	197	xxxx	605	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	1051	xxxx	xxxxx
Move Cap.:	186	xxxx	605	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	1051	xxxx	xxxxx
Volume/Cap:	1.30	xxxx	0.04	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.08	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	13.6	xxxx	0.1	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.2	xxxx	xxxxx
Control Del:	217.7	xxxx	11.2	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	8.7	xxxx	xxxxx
LOS by Move:	F	*	B	*	*	*	*	*	*	A	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	0.2	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	8.7	xxxx	xxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	A	*	*
ApproachDel:	198.1			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	F			*			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

Average Delay (sec/veh): 52.5 Worst Case Level Of Service: F[421.3]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume components (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume) and 4 rows for North, South, East, West bounds.

Critical Gap Module: Table with 4 columns for North, South, East, West bounds. Rows include Critical Gp and FollowUpTim.

Capacity Module: Table with 4 columns for North, South, East, West bounds. Rows include Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with 4 columns for North, South, East, West bounds. Rows include 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

Cycle (sec): 65 Critical Vol./Cap.(X): 0.651
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 11.6
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	1	0	0	0	0	0	0	0	1	0	1	0

Volume Module:

Base Vol:	194	0	24	0	0	0	0	359	109	75	548	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	194	0	24	0	0	0	0	359	109	75	548	0
Added Vol:	35	0	0	0	0	0	0	16	16	0	35	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	229	0	24	0	0	0	0	375	125	75	583	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	241	0	25	0	0	0	0	395	132	79	614	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	241	0	25	0	0	0	0	395	132	79	614	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	241	0	25	0	0	0	0	395	132	79	614	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.77	1.00	0.85	1.00	1.00	1.00	1.00	0.97	0.97	0.90	0.90	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.75	0.25	0.11	0.89	0.00
Final Sat.:	1461	0	1615	0	0	0	0	1377	459	194	1512	0

Capacity Analysis Module:

Vol/Sat:	0.16	0.00	0.02	0.00	0.00	0.00	0.00	0.29	0.29	0.41	0.41	0.00
Crit Moves:	****			****								
Green/Cycle:	0.28	0.00	0.28	0.00	0.00	0.00	0.00	0.60	0.60	0.60	0.60	0.00
Volume/Cap:	0.60	0.00	0.06	0.00	0.00	0.00	0.00	0.48	0.48	0.68	0.68	0.00
Delay/Veh:	22.8	0.0	17.3	0.0	0.0	0.0	0.0	7.6	7.6	10.6	10.6	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	22.8	0.0	17.3	0.0	0.0	0.0	0.0	7.6	7.6	10.6	10.6	0.0
LOS by Move:	C	A	B	A	A	A	A	A	A	B	B	A
HCM2kAvgQ:	5	0	0	0	0	0	0	6	6	10	10	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

Cycle (sec): 105 Critical Vol./Cap.(X): 0.852
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 17.0
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	1	0	0	0	0	0	0	0	1	0	1	0

Volume Module:

Base Vol:	138	0	56	0	0	0	0	818	242	44	465	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	138	0	56	0	0	0	0	818	242	44	465	0
Added Vol:	46	0	0	0	0	0	0	41	41	0	46	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	184	0	56	0	0	0	0	859	283	44	511	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	194	0	59	0	0	0	0	904	298	46	538	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	194	0	59	0	0	0	0	904	298	46	538	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	194	0	59	0	0	0	0	904	298	46	538	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.77	1.00	0.85	1.00	1.00	1.00	1.00	0.97	0.97	0.85	0.85	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.75	0.25	0.08	0.92	0.00
Final Sat.:	1461	0	1615	0	0	0	0	1382	455	128	1483	0

Capacity Analysis Module:

Vol/Sat:	0.13	0.00	0.04	0.00	0.00	0.00	0.00	0.65	0.65	0.36	0.36	0.00
Crit Moves:	****			****								
Green/Cycle:	0.17	0.00	0.17	0.00	0.00	0.00	0.00	0.75	0.75	0.75	0.75	0.00
Volume/Cap:	0.77	0.00	0.21	0.00	0.00	0.00	0.00	0.87	0.87	0.48	0.48	0.00
Delay/Veh:	55.4	0.0	37.8	0.0	0.0	0.0	0.0	15.5	15.5	5.4	5.4	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	55.4	0.0	37.8	0.0	0.0	0.0	0.0	15.5	15.5	5.4	5.4	0.0
LOS by Move:	E	A	D	A	A	A	A	B	B	A	A	A
HCM2kAvgQ:	8	0	2	0	0	0	0	30	30	8	8	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 3.5 Worst Case Level Of Service: F[200.9]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	0	0	0	0	0	1	0	1	0

Volume Module:

Base Vol:	25	0	17	0	0	0	0	1089	13	9	597	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	25	0	17	0	0	0	0	1089	13	9	597	0
Added Vol:	0	0	0	0	0	0	0	480	0	0	217	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	25	0	17	0	0	0	0	1569	13	9	814	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	26	0	18	0	0	0	0	1652	14	9	857	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	26	0	18	0	0	0	0	1652	14	9	857	0

Critical Gap Module:

Critical Gp:	6.4	xxxx	6.2	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	4.1	xxxx	xxxxx
FollowUpTim:	3.5	xxxx	3.3	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	2.2	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	2527	xxxx	1652	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	1665	xxxx	xxxxx
Potent Cap.:	31	xxxx	123	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	391	xxxx	xxxxx
Move Cap.:	30	xxxx	123	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	391	xxxx	xxxxx
Volume/Cap:	0.87	xxxx	0.15	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.02	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	2.9	xxxx	0.5	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.1	xxxx	xxxxx
Control Del:	310.9	xxxx	39.1	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	14.4	xxxx	xxxxx
LOS by Move:	F	*	E	*	*	*	*	*	*	B	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	200.9			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	F			*			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 7.3 Worst Case Level Of Service: F[600.0]

Approach:	North Bound			South Bound			East Bound			West Bound						
Movement:	L	T	R	L	T	R	L	T	R	L	T	R				
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled						
Rights:	Include			Include			Include			Include						
Lanes:	1	0	0	0	0	0	0	0	1	0	1	1	0	1	0	0

Volume Module:

Base Vol:	22	0	14	0	0	0	0	654	66	22	1027	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	22	0	14	0	0	0	0	654	66	22	1027	0
Added Vol:	0	0	0	0	0	0	0	641	0	0	566	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	22	0	14	0	0	0	0	1295	66	22	1593	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	23	0	15	0	0	0	0	1363	69	23	1677	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	23	0	15	0	0	0	0	1363	69	23	1677	0

Critical Gap Module:

Critical Gp:	6.4	xxxx	6.2	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	4.1	xxxx	xxxxx
FollowUpTim:	3.5	xxxx	3.3	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	2.2	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	3086	xxxx	1363	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	1433	xxxx	xxxxx
Potent Cap.:	13	xxxx	183	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	480	xxxx	xxxxx
Move Cap.:	13	xxxx	183	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	480	xxxx	xxxxx
Volume/Cap:	1.79	xxxx	0.08	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.05	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	3.7	xxxx	0.3	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.2	xxxx	xxxxx
Control Del:	965.0	xxxx	26.4	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	12.9	xxxx	xxxxx
LOS by Move:	F	*	D	*	*	*	*	*	*	B	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	600.0			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	F			*			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
Year 2025 With Project
Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: C [18.1]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns for traffic volumes and 12 rows for various volume metrics like Base Vol, Growth Adj, Initial Bse, etc.

Critical Gap Module: Table with 12 columns for gap metrics and 2 rows for Critical Gp and FollowUpTim.

Capacity Module: Table with 12 columns for capacity metrics and 4 rows for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level of Service Module: Table with 12 columns for LOS metrics and 8 rows for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 0.3 Worst Case Level Of Service: C [15.0]

Approach:	North Bound			South Bound			East Bound			West Bound						
Movement:	L	T	R	L	T	R	L	T	R	L	T	R				
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled						
Rights:	Include			Include			Include			Include						
Lanes:	1	0	0	0	0	0	0	0	2	0	1	1	0	2	0	0

Volume Module:

Base Vol:	0	0	36	0	0	0	0	654	66	22	1049	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	36	0	0	0	0	654	66	22	1049	0
Added Vol:	0	0	0	0	0	0	0	641	0	0	566	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	36	0	0	0	0	1295	66	22	1615	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	0	38	0	0	0	0	1363	69	23	1700	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	38	0	0	0	0	1363	69	23	1700	0

Critical Gap Module:

Critical Gp:	6.8	xxxx	6.9	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	4.1	xxxx	xxxxx
FollowUpTim:	3.5	xxxx	3.3	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	2.2	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	2259	xxxx	682	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	1433	xxxx	xxxxx
Potent Cap.:	36	xxxx	397	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	480	xxxx	xxxxx
Move Cap.:	34	xxxx	397	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	480	xxxx	xxxxx
Volume/Cap:	0.00	xxxx	0.10	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.05	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	0.3	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.2	xxxx	xxxxx
Control Del:	xxxxx	xxxx	15.0	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	12.9	xxxx	xxxxx
LOS by Move:	*	*	C	*	*	*	*	*	*	B	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	15.0			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	C			*			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.508
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 5.6
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	1	0	0	0	0	0	0	2	0	1	0	0

Volume Module:

Base Vol:	25	0	17	0	0	0	0	1089	13	9	597	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	25	0	17	0	0	0	0	1089	13	9	597	0
Added Vol:	0	0	0	0	0	0	0	480	0	0	217	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	25	0	17	0	0	0	0	1569	13	9	814	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	26	0	18	0	0	0	0	1652	14	9	857	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	26	0	18	0	0	0	0	1652	14	9	857	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	26	0	18	0	0	0	0	1652	14	9	857	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.82	1.00	0.85	1.00	1.00	1.00	1.00	0.95	0.85	0.12	0.95	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	2.00	1.00	1.00	2.00	0.00
Final Sat.:	1560	0	1615	0	0	0	0	3610	1615	220	3610	0

Capacity Analysis Module:

Vol/Sat:	0.02	0.00	0.01	0.00	0.00	0.00	0.00	0.46	0.01	0.04	0.24	0.00
Crit Moves:	****			****								
Green/Cycle:	0.15	0.00	0.15	0.00	0.00	0.00	0.00	0.78	0.78	0.78	0.78	0.00
Volume/Cap:	0.11	0.00	0.07	0.00	0.00	0.00	0.00	0.58	0.01	0.05	0.30	0.00
Delay/Veh:	44.3	0.0	44.0	0.0	0.0	0.0	0.0	5.5	2.8	3.1	3.8	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	44.3	0.0	44.0	0.0	0.0	0.0	0.0	5.5	2.8	3.1	3.8	0.0
LOS by Move:	D	A	D	A	A	A	A	A	A	A	A	A
HCM2kAvgQ:	1	0	1	0	0	0	0	13	0	0	5	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.513
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 5.6
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound						
Movement:	L	T	R	L	T	R	L	T	R	L	T	R				
Control:	Permitted			Permitted			Permitted			Permitted						
Rights:	Include			Include			Include			Include						
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0				
Lanes:	1	0	0	0	0	0	0	0	2	0	1	1	0	2	0	0

Volume Module:

Base Vol:	22	0	14	0	0	0	0	654	66	22	1027	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	22	0	14	0	0	0	0	654	66	22	1027	0
Added Vol:	0	0	0	0	0	0	0	641	0	0	566	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	22	0	14	0	0	0	0	1295	66	22	1593	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	23	0	15	0	0	0	0	1363	69	23	1677	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	23	0	15	0	0	0	0	1363	69	23	1677	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	23	0	15	0	0	0	0	1363	69	23	1677	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.83	1.00	0.85	1.00	1.00	1.00	1.00	0.95	0.85	0.17	0.95	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	2.00	1.00	1.00	2.00	0.00
Final Sat.:	1575	0	1615	0	0	0	0	3610	1615	325	3610	0

Capacity Analysis Module:

Vol/Sat:	0.01	0.00	0.01	0.00	0.00	0.00	0.00	0.38	0.04	0.07	0.46	0.00
Crit Moves:	****									****		
Green/Cycle:	0.15	0.00	0.15	0.00	0.00	0.00	0.00	0.78	0.78	0.78	0.78	0.00
Volume/Cap:	0.10	0.00	0.06	0.00	0.00	0.00	0.00	0.48	0.05	0.09	0.59	0.00
Delay/Veh:	44.2	0.0	43.9	0.0	0.0	0.0	0.0	4.7	3.0	3.2	5.6	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	44.2	0.0	43.9	0.0	0.0	0.0	0.0	4.7	3.0	3.2	5.6	0.0
LOS by Move:	D	A	D	A	A	A	A	A	A	A	A	A
HCM2kAvgQ:	1	0	0	0	0	0	0	9	1	0	13	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 1.9 Worst Case Level Of Service: E [44.5]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns for traffic volumes and adjustment factors across four approaches.

Critical Gap Module: Table with 12 columns for critical gap and follow-up time values.

Capacity Module: Table with 12 columns for capacity-related metrics like conflict volume and potential capacity.

Level Of Service Module: Table with 12 columns for LOS metrics including delay, queue length, and approach LOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 1.4 Worst Case Level Of Service: F[73.3]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module:

Table with 12 columns representing different traffic movements and 10 rows of volume data including Base Vol, Growth Adj, Initial Bse, etc.

Critical Gap Module:

Table with 12 columns and 2 rows showing Critical Gap and FollowUpTim values.

Capacity Module:

Table with 12 columns and 4 rows showing Capacity data including Cnflict Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module:

Table with 12 columns and 8 rows showing Level Of Service data including 2Way95thQ, Control Del, LOS by Move, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 1.3 Worst Case Level Of Service: C [24.8]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	0	1	0	0	1	0	0

Volume Module:

Base Vol:	16	627	4	7	424	13	10	0	63	17	0	9
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	16	627	4	7	424	13	10	0	63	17	0	9
Added Vol:	0	94	0	0	207	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	16	721	4	7	631	13	10	0	63	17	0	9
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	17	759	4	7	664	14	11	0	66	18	0	9
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	17	759	4	7	664	14	11	0	66	18	0	9

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	4.1	xxxx	xxxxxx	7.1	xxxx	6.2	7.1	xxxx	6.2
FollowUpTim:	2.2	xxxx	xxxxxx	2.2	xxxx	xxxxxx	3.5	xxxx	3.3	3.5	xxxx	3.3

Capacity Module:

Cnflct Vol:	678	xxxx	xxxxxx	763	xxxx	xxxxxx	1478	xxxx	332	1142	xxxx	761
Potent Cap.:	924	xxxx	xxxxxx	859	xxxx	xxxxxx	105	xxxx	714	179	xxxx	409
Move Cap.:	924	xxxx	xxxxxx	859	xxxx	xxxxxx	101	xxxx	714	159	xxxx	409
Volume/Cap:	0.02	xxxx	xxxxxx	0.01	xxxx	xxxxxx	0.10	xxxx	0.09	0.11	xxxx	0.02

Level Of Service Module:

2Way95thQ:	0.1	xxxx	xxxxxx	0.0	xxxx	xxxxxx	0.3	xxxx	0.3	0.4	xxxx	0.1
Control Del:	9.0	xxxx	xxxxxx	9.2	xxxx	xxxxxx	45.0	xxxx	10.6	30.4	xxxx	14.0
LOS by Move:	A	*	*	A	*	*	E	*	B	D	*	B
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxxx			xxxxxxx			15.3			24.8		
ApproachLOS:	*			*			C			C		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 0.9 Worst Case Level Of Service: D [30.4]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	2	0	0	0	1	0	0

Volume Module:

Base Vol:	52	454	17	9	729	15	5	0	26	6	0	4
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	52	454	17	9	729	15	5	0	26	6	0	4
Added Vol:	0	244	0	0	276	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	52	698	17	9	1005	15	5	0	26	6	0	4
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	55	735	18	9	1058	16	5	0	27	6	0	4
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	55	735	18	9	1058	16	5	0	27	6	0	4

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	4.1	xxxx	xxxxxx	7.1	xxxx	6.2	7.1	xxxx	6.2
FollowUpTim:	2.2	xxxx	xxxxxx	2.2	xxxx	xxxxxx	3.5	xxxx	3.3	3.5	xxxx	3.3

Capacity Module:

Cnflct Vol:	1074	xxxx	xxxxxx	753	xxxx	xxxxxx	1932	xxxx	529	1401	xxxx	744
Potent Cap.:	657	xxxx	xxxxxx	866	xxxx	xxxxxx	50	xxxx	554	119	xxxx	418
Move Cap.:	657	xxxx	xxxxxx	866	xxxx	xxxxxx	46	xxxx	554	105	xxxx	418
Volume/Cap:	0.08	xxxx	xxxx	0.01	xxxx	xxxx	0.11	xxxx	0.05	0.06	xxxx	0.01

Level Of Service Module:

2Way95thQ:	0.3	xxxx	xxxxxx	0.0	xxxx	xxxxxx	0.4	xxxx	0.2	0.2	xxxx	0.0
Control Del:	11.0	xxxx	xxxxxx	9.2	xxxx	xxxxxx	92.3	xxxx	11.8	41.5	xxxx	13.7
LOS by Move:	B	*	*	A	*	*	F	*	B	E	*	B
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			24.8			30.4		
ApproachLOS:	*			*			C			D		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #16 Soboba Road (NS) at Project North Access (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.332
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 16.6
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	18	0	18	0	0	0
Lanes:	2	0	1	0	0	2	0	0	1	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	238	0	0	345	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	238	0	0	345	0	0	0	0	0	0	0
Added Vol:	353	30	0	0	50	157	64	0	48	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	353	268	0	0	395	157	64	0	48	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	372	282	0	0	416	165	67	0	51	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	372	282	0	0	416	165	67	0	51	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	372	282	0	0	416	165	67	0	51	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	0.95	0.85	0.81	1.00	0.85	1.00	1.00	1.00
Lanes:	2.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	3502	1900	0	0	3610	1615	1531	0	1615	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.11	0.15	0.00	0.00	0.12	0.10	0.04	0.00	0.03	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.20	0.50	0.00	0.00	0.30	0.30	0.30	0.00	0.30	0.00	0.00	0.00
Volume/Cap:	0.53	0.30	0.00	0.00	0.38	0.34	0.15	0.00	0.10	0.00	0.00	0.00
Delay/Veh:	22.3	9.0	0.0	0.0	16.8	16.8	15.5	0.0	15.3	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	22.3	9.0	0.0	0.0	16.8	16.8	15.5	0.0	15.3	0.0	0.0	0.0
LOS by Move:	C	A	A	A	B	B	B	A	B	A	A	A
HCM2kAvgQ:	4	3	0	0	3	3	1	0	1	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #16 Soboba Road (NS) at Project North Access (EW)

Cycle (sec): 65 Critical Vol./Cap.(X): 0.602
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 18.8
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	18	0	18	0	0	0
Lanes:	2	0	1	0	0	2	0	0	1	0	0	0

Volume Module:

Base Vol:	0	437	0	0	683	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	437	0	0	683	0	0	0	0	0	0	0
Added Vol:	467	62	0	0	69	208	182	0	136	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	467	499	0	0	752	208	182	0	136	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	492	525	0	0	792	219	192	0	143	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	492	525	0	0	792	219	192	0	143	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	492	525	0	0	792	219	192	0	143	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	0.95	0.85	0.77	1.00	0.85	1.00	1.00	1.00
Lanes:	2.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	3502	1900	0	0	3610	1615	1461	0	1615	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.14	0.28	0.00	0.00	0.22	0.14	0.13	0.00	0.09	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.21	0.54	0.00	0.00	0.33	0.33	0.28	0.00	0.28	0.00	0.00	0.00
Volume/Cap:	0.67	0.51	0.00	0.00	0.67	0.41	0.47	0.00	0.32	0.00	0.00	0.00
Delay/Veh:	25.9	10.0	0.0	0.0	20.3	17.5	20.4	0.0	19.1	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	25.9	10.0	0.0	0.0	20.3	17.5	20.4	0.0	19.1	0.0	0.0	0.0
LOS by Move:	C	B	A	A	C	B	C	A	B	A	A	A
HCM2kAvgQ:	6	7	0	0	8	4	4	0	3	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Future Volume Alternative)

Intersection #17 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 1.286
 Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 74.0
 Optimal Cycle: 0 Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0	0	1! 0	0	0	1! 0	0	1	0	0	1	0

Volume Module:

Base Vol:	298	45	1	1	164	179	191	1	461	1	2	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	298	45	1	1	164	179	191	1	461	1	2	2
Added Vol:	72	-8	0	0	50	48	391	0	-109	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	370	37	1	1	214	227	582	1	352	1	2	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	389	39	1	1	225	239	613	1	371	1	2	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	389	39	1	1	225	239	613	1	371	1	2	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	389	39	1	1	225	239	613	1	371	1	2	2

Saturation Flow Module:

Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.90	0.09	0.01	0.01	0.48	0.51	0.99	0.01	1.00	0.20	0.40	0.40
Final Sat.:	474	47	1	1	273	289	476	1	568	80	159	159

Capacity Analysis Module:

Vol/Sat:	0.82	0.82	0.82	0.83	0.83	0.83	1.29	1.29	0.65	0.01	0.01	0.01
Crit Moves:	****			****			****			****		
Delay/Veh:	33.8	33.8	33.8	32.3	32.3	32.3	166.8	167	20.2	11.3	11.3	11.3
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	33.8	33.8	33.8	32.3	32.3	32.3	166.8	167	20.2	11.3	11.3	11.3
LOS by Move:	D	D	D	D	D	D	F	F	C	B	B	B
ApproachDel:	33.8			32.3			111.6			11.3		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	33.8			32.3			111.6			11.3		
LOS by Appr:	D			D			F			B		
AllWayAvgQ:	3.5	3.5	3.5	3.6	3.6	3.6	20.8	20.8	1.8	0.0	0.0	0.0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Future Volume Alternative)

Intersection #17 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 1.877
 Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 322.6
 Optimal Cycle: 0 Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0	0	1! 0	0	0	1! 0	0	1	0	0	1	0

-----|-----|-----|-----|

Volume Module:

Base Vol:	530	196	1	1	288	400	239	1	599	1	2	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	530	196	1	1	288	400	239	1	599	1	2	2
Added Vol:	157	-37	0	0	69	136	566	0	-138	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	687	159	1	1	357	536	805	1	461	1	2	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	723	167	1	1	376	564	847	1	485	1	2	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	723	167	1	1	376	564	847	1	485	1	2	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	723	167	1	1	376	564	847	1	485	1	2	2

-----|-----|-----|-----|

Saturation Flow Module:

Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.81	0.18	0.01	0.01	0.39	0.60	0.99	0.01	1.00	0.20	0.40	0.40
Final Sat.:	415	96	1	1	221	331	451	1	533	80	160	160

-----|-----|-----|-----|

Capacity Analysis Module:

Vol/Sat:	1.74	1.74	1.74	1.70	1.70	1.70	1.88	1.88	0.91	0.01	0.01	0.01
Crit Moves:	****					****	****			****		
Delay/Veh:	360.5	360	360.5	341.5	341	341.5	421.9	422	46.0	12.1	12.1	12.1
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	360.5	360	360.5	341.5	341	341.5	421.9	422	46.0	12.1	12.1	12.1
LOS by Move:	F	F	F	F	F	F	F	F	E	B	B	B
ApproachDel:	360.5			341.5			285.1			12.1		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	360.5			341.5			285.1			12.1		
LOS by Appr:	F			F			F			B		
AllWayAvgQ:	49.8	49.8	49.8	50.9	50.9	50.9	51.6	51.6	5.4	0.0	0.0	0.0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #17 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.489
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 29.0
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ovl			Ovl			Include		
Min. Green:	10	18	18	10	18	18	10	18	18	10	18	18
Lanes:	2	0	0	1	0	1	1	1	0	0	0	1

Volume Module:

Base Vol:	298	45	1	1	164	179	191	1	461	1	2	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	298	45	1	1	164	179	191	1	461	1	2	2
Added Vol:	72	-8	0	0	50	48	391	0	-109	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	370	37	1	1	214	227	582	1	352	1	2	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	389	39	1	1	225	239	613	1	371	1	2	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	389	39	1	1	225	239	613	1	371	1	2	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	389	39	1	1	225	239	613	1	371	1	2	2

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	0.95	1.00	0.75	0.95	0.95	0.85	0.94	0.94	0.94
Lanes:	2.00	0.97	0.03	1.00	1.00	2.00	1.99	0.01	1.00	0.20	0.40	0.40
Final Sat.:	3502	1843	50	1805	1900	2842	3611	6	1615	356	712	712

Capacity Analysis Module:

Vol/Sat:	0.11	0.02	0.02	0.00	0.12	0.08	0.17	0.17	0.23	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.17	0.24	0.24	0.13	0.20	0.46	0.26	0.26	0.42	0.20	0.20	0.20
Volume/Cap:	0.67	0.09	0.09	0.00	0.59	0.18	0.67	0.67	0.54	0.01	0.01	0.01
Delay/Veh:	38.0	26.9	26.9	34.0	35.2	14.7	31.9	31.9	20.4	28.9	28.9	28.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	38.0	26.9	26.9	34.0	35.2	14.7	31.9	31.9	20.4	28.9	28.9	28.9
LOS by Move:	D	C	C	C	D	B	C	C	C	C	C	C
HCM2kAvgQ:	6	1	1	0	6	2	9	9	8	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #17 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.741
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 43.9
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ovl			Ovl			Include		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	2	0	0	1	0	1	0	1	0	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	530	196	1	1	288	400	239	1	599	1	2	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	530	196	1	1	288	400	239	1	599	1	2	2
Added Vol:	157	-37	0	0	69	136	566	0	-138	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	687	159	1	1	357	536	805	1	461	1	2	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	723	167	1	1	376	564	847	1	485	1	2	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	723	167	1	1	376	564	847	1	485	1	2	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	723	167	1	1	376	564	847	1	485	1	2	2

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	0.95	1.00	0.75	0.95	0.95	0.85	0.94	0.94	0.94
Lanes:	2.00	0.99	0.01	1.00	1.00	2.00	1.99	0.01	1.00	0.20	0.40	0.40
Final Sat.:	3502	1886	12	1805	1900	2842	3613	4	1615	356	712	712

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.21	0.09	0.09	0.00	0.20	0.20	0.23	0.23	0.30	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.23	0.29	0.29	0.16	0.22	0.48	0.26	0.26	0.49	0.15	0.15	0.15
Volume/Cap:	0.89	0.30	0.30	0.00	0.89	0.41	0.89	0.89	0.61	0.02	0.02	0.02
Delay/Veh:	56.7	33.4	33.4	42.2	65.7	20.1	53.1	53.1	23.2	43.5	43.5	43.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	56.7	33.4	33.4	42.2	65.7	20.1	53.1	53.1	23.2	43.5	43.5	43.5
LOS by Move:	E	C	C	D	E	C	D	D	C	D	D	D
HCM2kAvgQ:	17	5	5	0	17	8	18	18	13	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #18 Soboba Road (NS) at Project South Access (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.453
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 8.8
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	0	0	18	18	18	0	18	0	0	0
Lanes:	1	0	1	0	0	0	1	0	0	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	343	0	0	626	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	343	0	0	626	0	0	0	0	0	0	0
Added Vol:	3	-67	0	0	-155	97	131	0	3	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	3	276	0	0	471	97	131	0	3	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	3	276	0	0	471	97	131	0	3	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	3	276	0	0	471	97	131	0	3	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	3	276	0	0	471	97	131	0	3	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.89	1.00	1.00	1.00	0.98	0.98	0.72	1.00	0.72	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	0.83	0.17	0.98	0.00	0.02	0.00	0.00	0.00
Final Sat.:	1695	1900	0	0	1539	317	1332	0	30	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.00	0.15	0.00	0.00	0.31	0.31	0.10	0.00	0.10	0.00	0.00	0.00
Crit Moves:				****			****					
Green/Cycle:	0.65	0.65	0.00	0.00	0.65	0.65	0.24	0.00	0.24	0.00	0.00	0.00
Volume/Cap:	0.00	0.22	0.00	0.00	0.47	0.47	0.41	0.00	0.41	0.00	0.00	0.00
Delay/Veh:	4.5	5.4	0.0	0.0	6.8	6.8	24.9	0.0	24.9	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	4.5	5.4	0.0	0.0	6.8	6.8	24.9	0.0	24.9	0.0	0.0	0.0
LOS by Move:	A	A	A	A	A	A	C	A	C	A	A	A
HCM2kAvgQ:	0	3	0	0	7	7	3	0	3	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #18 Soboba Road (NS) at Project South Access (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.762
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 14.1
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	0	0	18	18	18	0	18	0	0	0
Lanes:	1	0	1	0	0	0	1	0	0	0	0	0

-----|-----|-----|-----|

Volume Module:

Base Vol:	0	726	0	0	888	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	726	0	0	888	0	0	0	0	0	0	0
Added Vol:	4	-175	0	0	-197	128	295	0	4	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	4	551	0	0	691	128	295	0	4	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	4	551	0	0	691	128	295	0	4	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	4	551	0	0	691	128	295	0	4	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	4	551	0	0	691	128	295	0	4	0	0	0

-----|-----|-----|-----|

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.85	1.00	1.00	1.00	0.98	0.98	0.72	1.00	0.72	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	0.84	0.16	0.99	0.00	0.01	0.00	0.00	0.00
Final Sat.:	1609	1900	0	0	1569	291	1341	0	18	0	0	0

-----|-----|-----|-----|

Capacity Analysis Module:

Vol/Sat:	0.00	0.29	0.00	0.00	0.44	0.44	0.22	0.00	0.22	0.00	0.00	0.00
Crit Moves:				****			****					
Green/Cycle:	0.57	0.57	0.00	0.00	0.57	0.57	0.30	0.00	0.30	0.00	0.00	0.00
Volume/Cap:	0.00	0.51	0.00	0.00	0.78	0.78	0.73	0.00	0.73	0.00	0.00	0.00
Delay/Veh:	5.6	8.4	0.0	0.0	13.8	13.8	25.6	0.0	25.6	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	5.6	8.4	0.0	0.0	13.8	13.8	25.6	0.0	25.6	0.0	0.0	0.0
LOS by Move:	A	A	A	A	B	B	C	A	C	A	A	A
HCM2kAvgQ:	0	7	0	0	14	14	7	0	7	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
Year 2025 With Project
Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #19 Project Access (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 0.9 Worst Case Level Of Service: B[13.8]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume components. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with 12 columns for gap metrics. Rows include Critical Gp and FollowUpTim.

Capacity Module: Table with 12 columns for capacity metrics. Rows include Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with 12 columns for LOS metrics. Rows include 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Proposed Action "B"
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #19 Project Access (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 3.3 Worst Case Level Of Service: D[30.2]

Approach:	North Bound			South Bound			East Bound			West Bound						
Movement:	L	T	R	L	T	R	L	T	R	L	T	R				
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled						
Rights:	Include			Include			Include			Include						
Lanes:	0	0	0	0	0	0	0	0	1	1	0	0	0	2	0	0

Volume Module:

Base Vol:	0	0	0	0	0	0	0	840	0	0	932	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	0	0	0	0	840	0	0	932	0
Added Vol:	0	0	99	0	0	273	0	329	312	0	293	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	99	0	0	273	0	1169	312	0	1225	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	0	104	0	0	287	0	1231	328	0	1289	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	104	0	0	287	0	1231	328	0	1289	0

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	6.9	xxxxx	xxxx	6.9	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	xxxxx	xxxx	3.3	xxxxx	xxxx	3.3	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	xxxx	xxxx	779	xxxx	xxxx	645	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	xxxx	xxxx	343	xxxx	xxxx	420	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Move Cap.:	xxxx	xxxx	343	xxxx	xxxx	420	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	xxxx	xxxx	0.30	xxxx	xxxx	0.68	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	1.3	xxxx	xxxx	5.0	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	20.0	xxxxx	xxxx	30.2	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	*	*	C	*	*	D	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	20.0			30.2			xxxxxxx			xxxxxxx		
ApproachLOS:	C			D			*			*		

 Note: Queue reported is the number of cars per lane.

Year 2025 With Project – Alternative 1

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 Sanderson Avenue (NS) at Ramona Expressway (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.853
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 51.1
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with columns for North Bound, South Bound, East Bound, West Bound. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module table with columns for North Bound, South Bound, East Bound, West Bound. Rows include Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module table with columns for North Bound, South Bound, East Bound, West Bound. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 Sanderson Avenue (NS) at Ramona Expressway (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.846
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 52.6
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Ovl), Min. Green, Lanes.

-----|-----|-----|-----|

Volume Module:

Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

-----|-----|-----|-----|

Saturation Flow Module:

Table with columns: Sat/Lane, Adjustment, Lanes, Final Sat.

-----|-----|-----|-----|

Capacity Analysis Module:

Table with columns: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 13 columns for various volume metrics like Base Vol, Growth Adj, Initial Bse, etc.

Critical Gap Module: Table with 13 columns for critical gap and follow-up time metrics.

Capacity Module: Table with 13 columns for capacity metrics like Cnflct Vol, Potent Cap., Move Cap., etc.

Level Of Service Module: Table with 13 columns for LOS metrics like 2Way95thQ, Control Del, LOS by Move, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

Average Delay (sec/veh): 24.2 Worst Case Level Of Service: F[1090.0]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume components like Base Vol, Growth Adj, Initial Bse, etc.

Critical Gap Module: Table with 12 columns for gap metrics like Critical Gp, FollowUpTim.

Capacity Module: Table with 12 columns for capacity metrics like Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module: Table with 12 columns for LOS metrics like 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

Cycle (sec): 95 Critical Vol./Cap.(X): 0.667
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 30.7
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Ovl		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	1	0	0	1	0	0	0	0	1	0	1	0

Volume Module:

Base Vol:	1	693	15	148	252	1	1	1	1	15	1	366
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	1	693	15	148	252	1	1	1	1	15	1	366
Added Vol:	0	0	0	158	0	0	0	0	0	0	0	73
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	1	693	15	306	252	1	1	1	1	15	1	439
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	1	729	16	322	265	1	1	1	1	16	1	462
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	1	729	16	322	265	1	1	1	1	16	1	462
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	1	729	16	322	265	1	1	1	1	16	1	462

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	0.95	1.00	1.00	0.92	0.92	0.92	0.84	0.84	0.85
Lanes:	1.00	0.98	0.02	1.00	0.99	0.01	0.34	0.33	0.33	0.94	0.06	1.00
Final Sat.:	1805	1854	40	1805	1891	8	582	582	582	1487	99	1615

Capacity Analysis Module:

Vol/Sat:	0.00	0.39	0.39	0.18	0.14	0.14	0.00	0.00	0.00	0.01	0.01	0.29
Crit Moves:	****			****						****		
Green/Cycle:	0.24	0.47	0.47	0.21	0.44	0.44	0.19	0.19	0.19	0.19	0.19	0.40
Volume/Cap:	0.00	0.84	0.84	0.84	0.32	0.32	0.01	0.01	0.01	0.06	0.06	0.71
Delay/Veh:	27.1	28.9	28.9	50.4	17.6	17.6	31.3	31.3	31.3	31.6	31.6	27.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	27.1	28.9	28.9	50.4	17.6	17.6	31.3	31.3	31.3	31.6	31.6	27.4
LOS by Move:	C	C	C	D	B	B	C	C	C	C	C	C
HCM2kAvgQ:	0	21	21	12	5	5	0	0	0	0	0	12

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.698
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 28.2
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Ovl		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	1	0	0	1	0	0	0	0	1	0	1	0

Volume Module:

Base Vol:	1	466	14	380	663	1	1	1	2	21	2	241
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	1	466	14	380	663	1	1	1	2	21	2	241
Added Vol:	0	0	0	212	0	0	0	0	0	0	0	188
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	1	466	14	592	663	1	1	1	2	21	2	429
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	1	491	15	623	698	1	1	1	2	22	2	452
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	1	491	15	623	698	1	1	1	2	22	2	452
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	1	491	15	623	698	1	1	1	2	22	2	452

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	0.95	1.00	1.00	0.90	0.90	0.90	0.80	0.80	0.85
Lanes:	1.00	0.97	0.03	1.00	0.99	0.01	0.25	0.25	0.50	0.91	0.09	1.00
Final Sat.:	1805	1837	55	1805	1897	3	429	429	858	1393	133	1615

Capacity Analysis Module:

Vol/Sat:	0.00	0.27	0.27	0.35	0.37	0.37	0.00	0.00	0.00	0.02	0.02	0.28
Crit Moves:	****			****						****		
Green/Cycle:	0.14	0.33	0.33	0.42	0.61	0.61	0.15	0.15	0.15	0.15	0.15	0.57
Volume/Cap:	0.00	0.82	0.82	0.82	0.60	0.60	0.02	0.02	0.02	0.11	0.11	0.49
Delay/Veh:	44.6	45.4	45.4	37.4	15.2	15.2	43.5	43.5	43.5	44.3	44.3	15.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	44.6	45.4	45.4	37.4	15.2	15.2	43.5	43.5	43.5	44.3	44.3	15.6
LOS by Move:	D	D	D	D	B	B	D	D	D	D	D	B
HCM2kAvgQ:	0	19	19	22	16	16	0	0	0	1	1	10

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 State Street (NS) at Ramona Expressway (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.894
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 56.1
Optimal Cycle: OPTIMIZED Level Of Service: E

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns for different traffic directions. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with 12 columns for different traffic directions. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with 12 columns for different traffic directions. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 State Street (NS) at Ramona Expressway (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.122
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 80.7
Optimal Cycle: OPTIMIZED Level Of Service: F

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Approach, Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different volume metrics and 4 columns for North, South, East, and West bounds.

Saturation Flow Module:

Table with 12 columns representing saturation flow metrics and 4 columns for North, South, East, and West bounds.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis metrics and 4 columns for North, South, East, and West bounds.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 State Street (NS) at Ramona Expressway (EW)

 Cycle (sec): 115 Critical Vol./Cap.(X): 0.693
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 42.6
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Ovl			Include		
Min. Green:	10	28	28	10	28	28	10	24	24	10	24	24
Lanes:	1	0	2	0	1	1	1	0	2	0	1	1

Volume Module:

Base Vol:	379	683	101	109	453	56	159	279	422	252	502	307
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	379	683	101	109	453	56	159	279	422	252	502	307
Added Vol:	0	0	0	0	0	0	0	53	0	0	24	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	379	683	101	109	453	56	159	332	422	252	526	307
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	399	719	106	115	477	59	167	349	444	265	554	323
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	399	719	106	115	477	59	167	349	444	265	554	323
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	399	719	106	115	477	59	167	349	444	265	554	323

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	1805	3610	1615	1805	3610	1615	1805	3610	1615	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.22	0.20	0.07	0.06	0.13	0.04	0.09	0.10	0.28	0.15	0.15	0.20
Crit Moves:	****			****			****			****		
Green/Cycle:	0.25	0.36	0.36	0.13	0.24	0.24	0.11	0.21	0.45	0.16	0.26	0.26
Volume/Cap:	0.90	0.55	0.18	0.49	0.54	0.15	0.81	0.46	0.61	0.90	0.60	0.78
Delay/Veh:	63.0	29.9	25.3	48.3	38.6	34.3	70.6	40.3	25.1	75.6	38.5	48.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	63.0	29.9	25.3	48.3	38.6	34.3	70.6	40.3	25.1	75.6	38.5	48.6
LOS by Move:	E	C	C	D	D	C	E	D	C	E	D	D
HCM2kAvgQ:	17	11	3	4	8	2	8	6	12	13	10	12

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 State Street (NS) at Ramona Expressway (EW)

Cycle (sec): 115 Critical Vol./Cap.(X): 0.956
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 53.5
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Ovl			Include		
Min. Green:	10	28	28	10	28	28	10	24	24	10	24	24
Lanes:	1	0	2	0	1	1	1	0	2	0	1	1

Volume Module:

Base Vol:	422	750	112	288	744	71	125	699	574	232	388	154
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	422	750	112	288	744	71	125	699	574	232	388	154
Added Vol:	0	0	0	0	0	0	0	71	0	0	63	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	422	750	112	288	744	71	125	770	574	232	451	154
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	444	789	118	303	783	75	132	811	604	244	475	162
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	444	789	118	303	783	75	132	811	604	244	475	162
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	444	789	118	303	783	75	132	811	604	244	475	162

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	1805	3610	1615	1805	3610	1615	1805	3610	1615	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.25	0.22	0.07	0.17	0.22	0.05	0.07	0.22	0.37	0.14	0.13	0.10
Crit Moves:	****			****			****			****		
Green/Cycle:	0.25	0.29	0.29	0.20	0.24	0.24	0.11	0.23	0.48	0.14	0.26	0.26
Volume/Cap:	0.98	0.75	0.25	0.83	0.89	0.19	0.68	0.98	0.78	0.98	0.51	0.39
Delay/Veh:	80.1	39.8	31.3	59.0	53.3	34.7	58.5	70.8	30.0	101.1	36.8	35.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	80.1	39.8	31.3	59.0	53.3	34.7	58.5	70.8	30.0	101.1	36.8	35.7
LOS by Move:	F	D	C	E	D	C	E	E	C	F	D	D
HCM2kAvgQ:	21	15	3	13	17	2	6	20	19	13	8	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.846
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 31.5
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns representing different traffic movements and 11 rows of volume-related metrics like Base Vol, Growth Adj, etc.

Saturation Flow Module: Table with 12 columns and 5 rows showing saturation flow rates and adjustments.

Capacity Analysis Module: Table with 12 columns and 11 rows showing capacity analysis metrics like Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.120
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 76.7
Optimal Cycle: OPTIMIZED Level Of Service: E

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns for traffic volumes and adjustments. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with 12 columns for saturation flow factors. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with 12 columns for capacity analysis metrics. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.808
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 30.4
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	28	28	28	28	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	1	0	1	1	0	1

Volume Module:

Base Vol:	103	698	118	130	537	239	220	627	120	73	729	97
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	103	698	118	130	537	239	220	627	120	73	729	97
Added Vol:	0	0	26	0	0	0	0	123	0	12	52	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	103	698	144	130	537	239	220	750	120	85	781	97
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	108	735	152	137	565	252	232	789	126	89	822	102
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	108	735	152	137	565	252	232	789	126	89	822	102
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	108	735	152	137	565	252	232	789	126	89	822	102

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.17	0.93	0.93	0.21	1.00	0.85	0.95	0.93	0.93	0.95	0.95	0.85
Lanes:	1.00	1.66	0.34	1.00	1.00	1.00	1.00	1.72	0.28	1.00	2.00	1.00
Final Sat.:	319	2915	601	397	1900	1615	1805	3047	487	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.34	0.25	0.25	0.34	0.30	0.16	0.13	0.26	0.26	0.05	0.23	0.06
Crit Moves:				****				****				
Green/Cycle:	0.40	0.40	0.40	0.40	0.40	0.40	0.15	0.34	0.34	0.12	0.31	0.31
Volume/Cap:	0.84	0.62	0.62	0.85	0.73	0.38	0.85	0.76	0.76	0.41	0.73	0.20
Delay/Veh:	60.1	22.2	22.2	57.1	26.4	19.3	59.0	29.3	29.3	37.8	30.2	23.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	60.1	22.2	22.2	57.1	26.4	19.3	59.0	29.3	29.3	37.8	30.2	23.0
LOS by Move:	E	C	C	E	C	B	E	C	C	D	C	C
HCM2kAvgQ:	5	11	11	6	14	5	9	14	14	3	12	2

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.980
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 47.2
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	28	28	28	28	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	1	0	2	1	0	2

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	138	581	125	184	712	392	195	1075	172	144	1039	126
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	138	581	125	184	712	392	195	1075	172	144	1039	126
Added Vol:	0	0	35	0	0	0	0	165	0	31	145	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	138	581	160	184	712	392	195	1240	172	175	1184	126
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	145	612	168	194	749	413	205	1305	181	184	1246	133
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	145	612	168	194	749	413	205	1305	181	184	1246	133
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	145	612	168	194	749	413	205	1305	181	184	1246	133

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.18	0.92	0.92	0.31	1.00	0.85	0.95	0.89	0.89	0.95	0.95	0.85
Lanes:	1.00	1.57	0.43	1.00	1.00	1.00	1.00	2.63	0.37	1.00	2.00	1.00
Final Sat.:	343	2740	755	594	1900	1615	1805	4473	620	1805	3610	1615

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.42	0.22	0.22	0.33	0.39	0.26	0.11	0.29	0.29	0.10	0.35	0.08
Crit Moves:	****			****			****			****		
Green/Cycle:	0.43	0.43	0.43	0.43	0.43	0.43	0.12	0.35	0.35	0.12	0.35	0.35
Volume/Cap:	0.98	0.52	0.52	0.76	0.91	0.59	0.98	0.84	0.84	0.84	0.98	0.23
Delay/Veh:	101.3	25.3	25.3	40.8	46.5	27.4	109.2	40.0	40.0	75.8	59.0	27.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	101.3	25.3	25.3	40.8	46.5	27.4	109.2	40.0	40.0	75.8	59.0	27.6
LOS by Move:	F	C	C	D	D	C	F	D	D	E	E	C
HCM2kAvgQ:	9	11	11	8	30	12	12	21	21	9	30	3

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.317
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 135.6
Optimal Cycle: OPTIMIZED Level Of Service: F

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns representing different traffic movements. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with 12 columns. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with 12 columns. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 2.252
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 383.3
Optimal Cycle: OPTIMIZED Level Of Service: F

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Split Phase, Permitted), Rights (Include), and values for Min. Green and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume, and values for 12 movements.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, Final Sat., and values for 12 movements.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ, and values for 12 movements.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

 Cycle (sec): 90 Critical Vol./Cap.(X): 0.663
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 31.9
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ovl			Include			Ovl			Include		
Min. Green:	18	18	18	18	18	18	10	21	21	10	21	21
Lanes:	1	0	1	0	1	0	1	0	1	0	1	0

Volume Module:

Base Vol:	337	375	84	30	541	29	18	231	348	49	168	22
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	337	375	84	30	541	29	18	231	348	49	168	22
Added Vol:	0	0	202	53	0	0	0	0	0	89	0	24
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	337	375	286	83	541	29	18	231	348	138	168	46
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	355	395	301	87	569	31	19	243	366	145	177	48
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	355	395	301	87	569	31	19	243	366	145	177	48
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	355	395	301	87	569	31	19	243	366	145	177	48

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	0.85	0.95	0.94	0.94	0.95	1.00	0.85	0.92	0.97	0.97
Lanes:	1.00	1.00	1.00	1.00	1.90	0.10	1.00	1.00	1.00	2.00	0.79	0.21
Final Sat.:	1805	1900	1615	1805	3399	182	1805	1900	1615	3502	1444	395

Capacity Analysis Module:

Vol/Sat:	0.20	0.21	0.19	0.05	0.17	0.17	0.01	0.13	0.23	0.04	0.12	0.12
Crit Moves:	****			****			****			****		
Green/Cycle:	0.26	0.26	0.38	0.21	0.21	0.21	0.11	0.23	0.50	0.11	0.23	0.23
Volume/Cap:	0.74	0.79	0.50	0.23	0.79	0.79	0.09	0.55	0.46	0.37	0.52	0.52
Delay/Veh:	36.5	38.7	22.2	29.6	38.8	38.8	36.1	31.8	15.1	37.7	31.3	31.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	36.5	38.7	22.2	29.6	38.8	38.8	36.1	31.8	15.1	37.7	31.3	31.3
LOS by Move:	D	D	C	C	D	D	D	C	B	D	C	C
HCM2kAvgQ:	11	12	7	2	10	10	1	7	7	2	6	6

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

 Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

 Cycle (sec): 105 Critical Vol./Cap.(X): 0.832
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 46.3
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ovl			Include			Ovl			Include		
Min. Green:	18	18	18	18	18	18	10	21	21	10	21	21
Lanes:	1	0	1	0	1	0	1	0	1	0	1	0

Volume Module:

Base Vol:	452	436	171	81	522	49	50	356	452	179	304	54
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	452	436	171	81	522	49	50	356	452	179	304	54
Added Vol:	0	0	271	71	0	0	0	0	0	238	0	63
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	452	436	442	152	522	49	50	356	452	417	304	117
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	476	459	465	160	549	52	53	375	476	439	320	123
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	476	459	465	160	549	52	53	375	476	439	320	123
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	476	459	465	160	549	52	53	375	476	439	320	123

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	0.85	0.95	0.94	0.94	0.95	1.00	0.85	0.92	0.96	0.96
Lanes:	1.00	1.00	1.00	1.00	1.83	0.17	1.00	1.00	1.00	2.00	0.72	0.28
Final Sat.:	1805	1900	1615	1805	3257	306	1805	1900	1615	3502	1314	506

Capacity Analysis Module:

Vol/Sat:	0.26	0.24	0.29	0.09	0.17	0.17	0.03	0.20	0.29	0.13	0.24	0.24
Crit Moves:	****			****			****			****		
Green/Cycle:	0.29	0.29	0.43	0.19	0.19	0.19	0.10	0.23	0.52	0.14	0.27	0.27
Volume/Cap:	0.90	0.82	0.66	0.47	0.90	0.90	0.31	0.88	0.57	0.89	0.90	0.90
Delay/Veh:	53.6	44.2	26.0	39.0	56.6	56.6	45.3	57.3	18.2	61.9	55.9	55.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	53.6	44.2	26.0	39.0	56.6	56.6	45.3	57.3	18.2	61.9	55.9	55.9
LOS by Move:	D	D	C	D	E	E	D	E	B	E	E	E
HCM2kAvgQ:	18	16	13	5	13	13	2	15	11	10	17	17

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #6 San Jacinto Street (NS) at Esplanade Avenue (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.638
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 31.1
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns representing different traffic volumes and adjustment factors like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 12 columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #6 San Jacinto Street (NS) at Esplanade Avenue (EW)

Cycle (sec): 115 Critical Vol./Cap.(X): 0.873
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 50.0
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns and 15 rows including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module table with 12 columns and 4 rows including Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns and 10 rows including Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #7 San Jacinto Street (NS) at Menlo Avenue (EW)

Cycle (sec): 70 Critical Vol./Cap.(X): 0.644
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 22.8
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, Min. Green, Lanes.

Table with columns: Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Table with columns: Sat/Lane, Adjustment, Lanes, Final Sat., Saturation Flow Module.

Table with columns: Capacity Analysis Module, Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #7 San Jacinto Street (NS) at Menlo Avenue (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.841
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 35.5
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 sub-columns for movements (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different volume and adjustment factors. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module:

Table with 12 columns representing saturation flow factors. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis factors. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #8 San Jacinto Street (NS) at Devonshire Avenue (EW)

Cycle (sec): 70 Critical Vol./Cap.(X): 0.727
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 24.9
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, Min. Green, Lanes.

Volume Module: Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module: Table with columns: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module: Table with columns: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #8 San Jacinto Street (NS) at Devonshire Avenue (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.848
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 30.6
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns for volume metrics. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with 12 columns for saturation flow metrics. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with 12 columns for capacity analysis metrics. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.900
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 107.4
Optimal Cycle: OPTIMIZED Level Of Service: F

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different volume metrics and 12 rows of data including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module:

Table with 12 columns representing saturation flow metrics and 4 rows of data including Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis metrics and 10 rows of data including Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.160
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 159.2
Optimal Cycle: OPTIMIZED Level Of Service: F

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

-----|-----|-----|-----|

Volume Module:

Table with 12 columns representing different traffic metrics and 12 rows of data including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

-----|-----|-----|-----|

Saturation Flow Module:

Table with 12 columns representing saturation flow metrics and 4 rows of data including Sat/Lane, Adjustment, Lanes, and Final Sat.

-----|-----|-----|-----|

Capacity Analysis Module:

Table with 12 columns representing capacity analysis metrics and 10 rows of data including Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

 Cycle (sec): 105 Critical Vol./Cap.(X): 0.667
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 34.4
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	10	28	28	10	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	1	0	1	1	0	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	81	259	77	163	241	149	200	702	43	97	1114	202
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	81	259	77	163	241	149	200	702	43	97	1114	202
Added Vol:	0	26	0	0	12	64	149	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	81	285	77	163	253	213	349	702	43	97	1114	202
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	85	300	81	172	266	224	367	739	45	102	1173	213
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	85	300	81	172	266	224	367	739	45	102	1173	213
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	85	300	81	172	266	224	367	739	45	102	1173	213

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.92	0.92	0.92	1.00	0.85	0.92	0.94	0.94	0.95	0.89	0.89
Lanes:	1.00	1.57	0.43	2.00	1.00	1.00	2.00	1.88	0.12	1.00	2.54	0.46
Final Sat.:	1805	2751	743	3502	1900	1615	3502	3371	206	1805	4290	778

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.05	0.11	0.11	0.05	0.14	0.14	0.10	0.22	0.22	0.06	0.27	0.27
Crit Moves:	****			****			****			****		
Green/Cycle:	0.10	0.27	0.27	0.10	0.27	0.40	0.13	0.36	0.36	0.13	0.35	0.35
Volume/Cap:	0.50	0.41	0.41	0.51	0.53	0.35	0.78	0.61	0.61	0.44	0.78	0.78
Delay/Veh:	47.4	32.0	32.0	46.6	33.9	22.2	52.0	28.6	28.6	43.7	32.7	32.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	47.4	32.0	32.0	46.6	33.9	22.2	52.0	28.6	28.6	43.7	32.7	32.7
LOS by Move:	D	C	C	D	C	C	D	C	C	D	C	C
HCM2kAvgQ:	3	5	5	3	8	5	8	11	11	4	16	16

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.894
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 51.0
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound											
Movement:	L	T	R	L	T	R	L	T	R	L	T	R									
Control:	Protected			Protected			Protected			Protected											
Rights:	Include			Ovl			Include			Include											
Min. Green:	10	28	28	10	28	28	10	28	28	10	28	28									
Lanes:	1	0	1	1	0	1	2	0	1	0	1	2	0	1	1	0	1	0	2	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	79	349	144	457	338	208	223	1167	108	172	1193	75
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	79	349	144	457	338	208	223	1167	108	172	1193	75
Added Vol:	0	35	0	0	31	176	200	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	79	384	144	457	369	384	423	1167	108	172	1193	75
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	83	404	152	481	388	404	445	1228	114	181	1256	79
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	83	404	152	481	388	404	445	1228	114	181	1256	79
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	83	404	152	481	388	404	445	1228	114	181	1256	79

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	0.91	0.92	1.00	0.85	0.92	0.94	0.94	0.95	0.90	0.90
Lanes:	1.00	1.45	0.55	2.00	1.00	1.00	2.00	1.83	0.17	1.00	2.82	0.18
Final Sat.:	1805	2518	944	3502	1900	1615	3502	3261	302	1805	4836	304

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.05	0.16	0.16	0.14	0.20	0.25	0.13	0.38	0.38	0.10	0.26	0.26
Crit Moves:	****			****			****			****		
Green/Cycle:	0.10	0.23	0.23	0.14	0.28	0.44	0.16	0.39	0.39	0.10	0.33	0.33
Volume/Cap:	0.47	0.69	0.69	0.97	0.74	0.57	0.79	0.97	0.97	0.97	0.79	0.79
Delay/Veh:	53.0	44.5	44.5	83.9	45.1	26.4	55.5	53.5	53.5	110.5	38.9	38.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	53.0	44.5	44.5	83.9	45.1	26.4	55.5	53.5	53.5	110.5	38.9	38.9
LOS by Move:	D	D	D	F	D	C	E	D	D	F	D	D
HCM2kAvgQ:	3	11	11	13	14	11	10	31	31	10	18	18

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

Cycle (sec): 110 Critical Vol./Cap.(X): 0.808
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 41.5
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns representing different volume metrics and 12 rows of data.

Saturation Flow Module: Table with 12 columns representing saturation flow metrics and 4 rows of data.

Capacity Analysis Module: Table with 12 columns representing capacity analysis metrics and 10 rows of data.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.200

Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 94.0

Optimal Cycle: OPTIMIZED Level Of Service: F

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green, Lanes.

Table with columns: Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Table with columns: Saturation Flow Module, Sat/Lane, Adjustment, Lanes, Final Sat.

Table with columns: Capacity Analysis Module, Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

 Cycle (sec): 95 Critical Vol./Cap.(X): 0.707
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 32.1
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Include		
Min. Green:	10	24	24	10	24	24	10	28	28	10	28	28
Lanes:	1	0	2	0	1	1	0	1	1	0	2	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	64	848	367	98	436	37	42	238	95	225	171	40
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	64	848	367	98	436	37	42	238	95	225	171	40
Added Vol:	0	0	59	53	0	0	0	255	0	30	113	24
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	64	848	426	151	436	37	42	493	95	255	284	64
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	67	893	448	159	459	39	44	519	100	268	299	67
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	67	893	448	159	459	39	44	519	100	268	299	67
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	67	893	448	159	459	39	44	519	100	268	299	67

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.94	0.94	0.95	0.93	0.93	0.92	0.92	0.92
Lanes:	1.00	2.00	1.00	1.00	1.84	0.16	1.00	1.68	0.32	2.00	1.63	0.37
Final Sat.:	1805	3610	1615	1805	3288	279	1805	2954	569	3502	2864	645

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.04	0.25	0.28	0.09	0.14	0.14	0.02	0.18	0.18	0.08	0.10	0.10
Crit Moves:	****			****			****			****		
Green/Cycle:	0.13	0.32	0.42	0.11	0.30	0.30	0.11	0.29	0.29	0.11	0.29	0.29
Volume/Cap:	0.29	0.78	0.66	0.78	0.46	0.46	0.23	0.60	0.60	0.73	0.35	0.35
Delay/Veh:	38.3	32.7	24.2	57.9	27.0	27.0	39.6	29.6	29.6	48.3	26.6	26.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	38.3	32.7	24.2	57.9	27.0	27.0	39.6	29.6	29.6	48.3	26.6	26.6
LOS by Move:	D	C	C	E	C	C	D	C	C	D	C	C
HCM2kAvgQ:	2	14	11	7	6	6	1	9	9	6	5	5

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

 Cycle (sec): 110 Critical Vol./Cap.(X): 0.801
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 43.8
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Include		
Min. Green:	10	24	24	10	24	24	10	28	28	10	28	28
Lanes:	1	0	2	0	1	1	0	1	1	0	2	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	86	406	375	182	702	23	23	344	86	560	378	157
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	86	406	375	182	702	23	23	344	86	560	378	157
Added Vol:	0	0	79	71	0	0	0	341	0	71	301	63
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	86	406	454	253	702	23	23	685	86	631	679	220
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	91	427	478	266	739	24	24	721	91	664	715	232
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	91	427	478	266	739	24	24	721	91	664	715	232
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	91	427	478	266	739	24	24	721	91	664	715	232

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.95	0.95	0.95	0.93	0.93	0.92	0.91	0.91
Lanes:	1.00	2.00	1.00	1.00	1.94	0.06	1.00	1.78	0.22	2.00	1.51	0.49
Final Sat.:	1805	3610	1615	1805	3478	114	1805	3153	396	3502	2626	851

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.05	0.12	0.30	0.15	0.21	0.21	0.01	0.23	0.23	0.19	0.27	0.27
Crit Moves:	****			****			****			****		
Green/Cycle:	0.11	0.22	0.43	0.17	0.27	0.27	0.12	0.26	0.26	0.21	0.35	0.35
Volume/Cap:	0.44	0.54	0.69	0.89	0.78	0.78	0.11	0.89	0.89	0.89	0.77	0.77
Delay/Veh:	47.1	38.9	28.1	71.0	41.3	41.3	43.6	50.1	50.1	54.7	34.8	34.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	47.1	38.9	28.1	71.0	41.3	41.3	43.6	50.1	50.1	54.7	34.8	34.8
LOS by Move:	D	D	C	E	D	D	D	D	D	D	C	C
HCM2kAvgQ:	3	7	14	12	14	14	1	17	17	15	16	16

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

Average Delay (sec/veh): 20.8 Worst Case Level Of Service: F[238.0]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns representing different traffic movements and 10 rows of volume data including Base Vol, Growth Adj, Initial Bse, etc.

Critical Gap Module: Table with 12 columns and 2 rows showing Critical Gp and FollowUpTim values.

Capacity Module: Table with 12 columns and 4 rows showing Capacity-related metrics like Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with 12 columns and 10 rows showing LOS-related metrics like 2Way95thQ, Control Del, LOS by Move, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

Average Delay (sec/veh): 101.6 Worst Case Level Of Service: F[592.6]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume and growth factors across four directions.

Critical Gap Module: Table with 12 columns for critical gap and follow-up time.

Capacity Module: Table with 12 columns for capacity and volume/capacity ratios.

Level Of Service Module: Table with 12 columns for LOS, delay, and queue length.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 0.453
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 10.3
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	18	18	18	18	28	28	28	28	28	28
Lanes:	1	0	2	0	1	0	0	1	0	0	0	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	136	1143	8	34	614	106	71	2	113	3	5	1
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	136	1143	8	34	614	106	71	2	113	3	5	1
Added Vol:	0	56	0	0	27	3	3	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	136	1199	8	34	641	109	74	2	113	3	5	1
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	143	1262	8	36	675	115	78	2	119	3	5	1
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	143	1262	8	36	675	115	78	2	119	3	5	1
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	143	1262	8	36	675	115	78	2	119	3	5	1

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.37	0.95	0.85	0.18	0.95	0.85	0.72	0.72	0.85	0.94	0.94	0.94
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	0.97	0.03	1.00	0.33	0.56	0.11
Final Sat.:	695	3610	1615	334	3610	1615	1338	36	1615	594	990	198

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.21	0.35	0.01	0.11	0.19	0.07	0.06	0.06	0.07	0.01	0.01	0.01
Crit Moves:	****			****			****			****		
Green/Cycle:	0.70	0.70	0.70	0.70	0.70	0.70	0.23	0.23	0.23	0.23	0.23	0.23
Volume/Cap:	0.29	0.50	0.01	0.15	0.27	0.10	0.25	0.25	0.32	0.02	0.02	0.02
Delay/Veh:	7.1	8.5	5.4	6.4	6.7	5.9	37.9	37.9	38.6	35.5	35.5	35.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	7.1	8.5	5.4	6.4	6.7	5.9	37.9	37.9	38.6	35.5	35.5	35.5
LOS by Move:	A	A	A	A	A	A	D	D	D	D	D	D
HCM2kAvgQ:	2	11	0	1	5	1	2	2	4	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.717
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 14.6
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	18	18	18	18	28	28	28	28	28	28
Lanes:	1	0	2	0	1	0	0	1	0	0	0	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	109	649	1	61	1180	69	109	1	331	4	2	6
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	109	649	1	61	1180	69	109	1	331	4	2	6
Added Vol:	0	75	0	0	67	4	4	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	109	724	1	61	1247	73	113	1	331	4	2	6
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	115	762	1	64	1313	77	119	1	348	4	2	6
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	115	762	1	64	1313	77	119	1	348	4	2	6
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	115	762	1	64	1313	77	119	1	348	4	2	6

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.14	0.95	0.85	0.32	0.95	0.85	0.71	0.71	0.85	0.88	0.88	0.88
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	0.99	0.01	1.00	0.33	0.17	0.50
Final Sat.:	262	3610	1615	599	3610	1615	1332	12	1615	560	280	839

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.44	0.21	0.00	0.11	0.36	0.05	0.09	0.09	0.22	0.01	0.01	0.01
Crit Moves:	****									****		
Green/Cycle:	0.60	0.60	0.60	0.60	0.60	0.60	0.31	0.31	0.31	0.31	0.31	0.31
Volume/Cap:	0.73	0.35	0.00	0.18	0.61	0.08	0.29	0.29	0.69	0.02	0.02	0.02
Delay/Veh:	28.7	9.2	7.2	8.3	11.8	7.6	23.8	23.8	31.4	21.5	21.5	21.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	28.7	9.2	7.2	8.3	11.8	7.6	23.8	23.8	31.4	21.5	21.5	21.5
LOS by Move:	C	A	A	A	B	A	C	C	C	C	C	C
HCM2kAvgQ:	4	6	0	1	12	1	3	3	10	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #12 Mountain Avenue (NS) at Esplanade Avenue (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.890
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 30.1
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Permitted), Rights (Include), Min. Green, Lanes.

Table with columns: Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Table with columns: Sat/Lane, Adjustment, Lanes, Final Sat., Saturation Flow Module.

Table with columns: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ, Capacity Analysis Module.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #12 Mountain Avenue (NS) at Esplanade Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.075
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 64.2
Optimal Cycle: OPTIMIZED Level Of Service: E

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, Min. Green, Lanes.

Table with columns: Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Table with columns: Saturation Flow Module, Sat/Lane, Adjustment, Lanes, Final Sat.

Table with columns: Capacity Analysis Module, Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #12 Mountain Avenue (NS) at Esplanade Avenue (EW)

Cycle (sec): 80 Critical Vol./Cap.(X): 0.879
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 26.3
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	24	0	0	24	24	21	0	21	0	0	0
Lanes:	1	0	1	0	0	2	0	0	1	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	232	890	0	0	464	139	309	0	122	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	232	890	0	0	464	139	309	0	122	0	0	0
Added Vol:	0	53	0	0	24	3	3	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	232	943	0	0	488	142	312	0	122	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	244	993	0	0	514	149	328	0	128	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	244	993	0	0	514	149	328	0	128	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	244	993	0	0	514	149	328	0	128	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	0.95	0.85	0.77	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	3610	1615	1461	0	1615	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.14	0.52	0.00	0.00	0.14	0.09	0.22	0.00	0.08	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.29	0.59	0.00	0.00	0.30	0.30	0.26	0.00	0.26	0.00	0.00	0.00
Volume/Cap:	0.47	0.89	0.00	0.00	0.47	0.31	0.86	0.00	0.30	0.00	0.00	0.00
Delay/Veh:	24.2	23.3	0.0	0.0	23.2	22.0	45.1	0.0	24.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	24.2	23.3	0.0	0.0	23.2	22.0	45.1	0.0	24.0	0.0	0.0	0.0
LOS by Move:	C	C	A	A	C	C	D	A	C	A	A	A
HCM2kAvgQ:	5	25	0	0	6	3	11	0	3	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #12 Mountain Avenue (NS) at Esplanade Avenue (EW)

 Cycle (sec): 75 Critical Vol./Cap.(X): 0.819
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 25.4
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	24	0	0	24	24	21	0	21	0	0	0
Lanes:	1	0	1	0	0	2	0	0	1	0	0	0

Volume Module:

Base Vol:	244	462	0	0	1003	401	249	0	360	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	244	462	0	0	1003	401	249	0	360	0	0	0
Added Vol:	0	71	0	0	63	4	4	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	244	533	0	0	1066	405	253	0	360	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	257	561	0	0	1122	426	266	0	379	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	257	561	0	0	1122	426	266	0	379	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	257	561	0	0	1122	426	266	0	379	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	0.95	0.85	0.77	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	3610	1615	1461	0	1615	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.14	0.30	0.00	0.00	0.31	0.26	0.18	0.00	0.23	0.00	0.00	0.00
Crit Moves:	****			****					****			
Green/Cycle:	0.17	0.55	0.00	0.00	0.38	0.38	0.29	0.00	0.29	0.00	0.00	0.00
Volume/Cap:	0.82	0.53	0.00	0.00	0.82	0.70	0.64	0.00	0.82	0.00	0.00	0.00
Delay/Veh:	45.3	11.1	0.0	0.0	25.0	23.1	26.6	0.0	35.9	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	45.3	11.1	0.0	0.0	25.0	23.1	26.6	0.0	35.9	0.0	0.0	0.0
LOS by Move:	D	B	A	A	C	C	C	A	D	A	A	A
HCM2kAvgQ:	8	9	0	0	15	10	6	0	11	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

Average Delay (sec/veh): 30.4 Worst Case Level Of Service: F[169.8]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 13 columns for traffic volumes and 13 rows for various volume metrics like Base Vol, Growth Adj, etc.

Critical Gap Module: Table with 13 columns for gap metrics and 2 rows for Critical Gp and FollowUpTim.

Capacity Module: Table with 13 columns for capacity metrics and 4 rows for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level of Service Module: Table with 13 columns for LOS metrics and 10 rows for various LOS-related metrics.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

Average Delay (sec/veh): 42.7 Worst Case Level Of Service: F[351.5]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 13 columns for volume components (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume) and 4 columns for North, South, East, West bounds.

Critical Gap Module: Table with 13 columns for gap metrics (Critical Gp, FollowUpTim) and 4 columns for North, South, East, West bounds.

Capacity Module: Table with 13 columns for capacity metrics (Cnflct Vol, Potent Cap., Move Cap., Volume/Cap) and 4 columns for North, South, East, West bounds.

Level Of Service Module: Table with 13 columns for LOS metrics (2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS) and 4 columns for North, South, East, West bounds.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

Cycle (sec): 70 Critical Vol./Cap.(X): 0.632
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 11.3
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	1	0	0	0	0	0	0	0	0	1	0	0

Volume Module:

Base Vol:	194	0	24	0	0	0	0	359	109	75	548	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	194	0	24	0	0	0	0	359	109	75	548	0
Added Vol:	26	0	0	0	0	0	0	12	12	0	26	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	220	0	24	0	0	0	0	371	121	75	574	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	232	0	25	0	0	0	0	391	127	79	604	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	232	0	25	0	0	0	0	391	127	79	604	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	232	0	25	0	0	0	0	391	127	79	604	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.77	1.00	0.85	1.00	1.00	1.00	1.00	0.97	0.97	0.90	0.90	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.75	0.25	0.12	0.88	0.00
Final Sat.:	1461	0	1615	0	0	0	0	1385	452	197	1507	0

Capacity Analysis Module:

Vol/Sat:	0.16	0.00	0.02	0.00	0.00	0.00	0.00	0.28	0.28	0.40	0.40	0.00
Crit Moves:	****			****								
Green/Cycle:	0.26	0.00	0.26	0.00	0.00	0.00	0.00	0.63	0.63	0.63	0.63	0.00
Volume/Cap:	0.62	0.00	0.06	0.00	0.00	0.00	0.00	0.45	0.45	0.64	0.64	0.00
Delay/Veh:	26.0	0.0	19.7	0.0	0.0	0.0	0.0	7.0	7.0	9.4	9.4	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	26.0	0.0	19.7	0.0	0.0	0.0	0.0	7.0	7.0	9.4	9.4	0.0
LOS by Move:	C	A	B	A	A	A	A	A	A	A	A	A
HCM2kAvgQ:	5	0	0	0	0	0	0	6	6	10	10	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

Cycle (sec): 110 Critical Vol./Cap.(X): 0.828
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 15.8
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	1	0	0	0	0	0	0	0	0	1	0	0

Volume Module:

Base Vol:	138	0	56	0	0	0	0	818	242	44	465	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	138	0	56	0	0	0	0	818	242	44	465	0
Added Vol:	35	0	0	0	0	0	0	31	31	0	35	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	173	0	56	0	0	0	0	849	273	44	500	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	182	0	59	0	0	0	0	894	287	46	526	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	182	0	59	0	0	0	0	894	287	46	526	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	182	0	59	0	0	0	0	894	287	46	526	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.77	1.00	0.85	1.00	1.00	1.00	1.00	0.97	0.97	0.85	0.85	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.76	0.24	0.08	0.92	0.00
Final Sat.:	1461	0	1615	0	0	0	0	1390	447	130	1479	0

Capacity Analysis Module:

Vol/Sat:	0.12	0.00	0.04	0.00	0.00	0.00	0.00	0.64	0.64	0.36	0.36	0.00
Crit Moves:	****			****								
Green/Cycle:	0.16	0.00	0.16	0.00	0.00	0.00	0.00	0.76	0.76	0.76	0.76	0.00
Volume/Cap:	0.76	0.00	0.22	0.00	0.00	0.00	0.00	0.84	0.84	0.47	0.47	0.00
Delay/Veh:	57.3	0.0	40.4	0.0	0.0	0.0	0.0	13.4	13.4	5.1	5.1	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	57.3	0.0	40.4	0.0	0.0	0.0	0.0	13.4	13.4	5.1	5.1	0.0
LOS by Move:	E	A	D	A	A	A	A	B	B	A	A	A
HCM2kAvgQ:	8	0	2	0	0	0	0	28	28	7	7	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 2.6 Worst Case Level Of Service: F[136.0]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume) across 4 approaches.

Critical Gap Module: Table with 4 columns for critical gap metrics (Critical Gp, FollowUpTim) across 4 approaches.

Capacity Module: Table with 4 columns for capacity metrics (Cnflct Vol, Potent Cap., Move Cap., Volume/Cap) across 4 approaches.

Level Of Service Module: Table with 4 columns for LOS metrics (2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS) across 4 approaches.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 4.4 Worst Case Level Of Service: F[324.7]

Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: C [16.8]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns for traffic volumes and 12 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Critical Gap Module: Table with 12 columns for gap metrics and 2 rows for Critical Gp and FollowUpTim.

Capacity Module: Table with 12 columns for capacity metrics and 4 rows for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with 12 columns for LOS metrics and 8 rows for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 0.3 Worst Case Level Of Service: B [13.8]

Approach:	North Bound			South Bound			East Bound			West Bound						
Movement:	L	T	R	L	T	R	L	T	R	L	T	R				
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled						
Rights:	Include			Include			Include			Include						
Lanes:	1	0	0	0	0	0	0	0	2	0	1	1	0	2	0	0

Volume Module:

Base Vol:	0	0	36	0	0	0	0	654	66	22	1049	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	36	0	0	0	0	654	66	22	1049	0
Added Vol:	0	0	0	0	0	0	0	491	0	0	434	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	36	0	0	0	0	1145	66	22	1483	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	0	38	0	0	0	0	1205	69	23	1561	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	38	0	0	0	0	1205	69	23	1561	0

Critical Gap Module:

Critical Gp:	6.8	xxxx	6.9	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	4.1	xxxx	xxxxx
FollowUpTim:	3.5	xxxx	3.3	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	2.2	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	2032	xxxx	603	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	1275	xxxx	xxxxx
Potent Cap.:	51	xxxx	447	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	552	xxxx	xxxxx
Move Cap.:	49	xxxx	447	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	552	xxxx	xxxxx
Volume/Cap:	0.00	xxxx	0.08	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.04	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	0.3	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.1	xxxx	xxxxx
Control Del:	xxxxx	xxxx	13.8	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	11.8	xxxx	xxxxx
LOS by Move:	*	*	B	*	*	*	*	*	*	B	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	13.8			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	B			*			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.473
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 5.3
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	1	0	0	0	0	0	0	2	0	1	0	0

-----|-----|-----|-----|

Volume Module:

Base Vol:	25	0	17	0	0	0	0	1089	13	9	597	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	25	0	17	0	0	0	0	1089	13	9	597	0
Added Vol:	0	0	0	0	0	0	0	366	0	0	168	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	25	0	17	0	0	0	0	1455	13	9	765	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	26	0	18	0	0	0	0	1532	14	9	805	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	26	0	18	0	0	0	0	1532	14	9	805	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	26	0	18	0	0	0	0	1532	14	9	805	0

-----|-----|-----|-----|

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.82	1.00	0.85	1.00	1.00	1.00	1.00	0.95	0.85	0.14	0.95	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	2.00	1.00	1.00	2.00	0.00
Final Sat.:	1560	0	1615	0	0	0	0	3610	1615	260	3610	0

-----|-----|-----|-----|

Capacity Analysis Module:

Vol/Sat:	0.02	0.00	0.01	0.00	0.00	0.00	0.00	0.42	0.01	0.04	0.22	0.00
Crit Moves:	****			****								
Green/Cycle:	0.15	0.00	0.15	0.00	0.00	0.00	0.00	0.78	0.78	0.78	0.78	0.00
Volume/Cap:	0.11	0.00	0.07	0.00	0.00	0.00	0.00	0.54	0.01	0.05	0.28	0.00
Delay/Veh:	44.3	0.0	44.0	0.0	0.0	0.0	0.0	5.1	2.8	3.0	3.7	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	44.3	0.0	44.0	0.0	0.0	0.0	0.0	5.1	2.8	3.0	3.7	0.0
LOS by Move:	D	A	D	A	A	A	A	A	A	A	A	A
HCM2kAvgQ:	1	0	1	0	0	0	0	11	0	0	4	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.472
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 5.2
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound						
Movement:	L	T	R	L	T	R	L	T	R	L	T	R				
Control:	Permitted			Permitted			Permitted			Permitted						
Rights:	Include			Include			Include			Include						
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0				
Lanes:	1	0	0	0	0	0	0	0	2	0	1	1	0	2	0	0

Volume Module:

Base Vol:	22	0	14	0	0	0	0	654	66	22	1027	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	22	0	14	0	0	0	0	654	66	22	1027	0
Added Vol:	0	0	0	0	0	0	0	491	0	0	434	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	22	0	14	0	0	0	0	1145	66	22	1461	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	23	0	15	0	0	0	0	1205	69	23	1538	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	23	0	15	0	0	0	0	1205	69	23	1538	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	23	0	15	0	0	0	0	1205	69	23	1538	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.83	1.00	0.85	1.00	1.00	1.00	1.00	0.95	0.85	0.21	0.95	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	2.00	1.00	1.00	2.00	0.00
Final Sat.:	1575	0	1615	0	0	0	0	3610	1615	395	3610	0

Capacity Analysis Module:

Vol/Sat:	0.01	0.00	0.01	0.00	0.00	0.00	0.00	0.33	0.04	0.06	0.43	0.00
Crit Moves:	****									****		
Green/Cycle:	0.15	0.00	0.15	0.00	0.00	0.00	0.00	0.78	0.78	0.78	0.78	0.00
Volume/Cap:	0.10	0.00	0.06	0.00	0.00	0.00	0.00	0.43	0.05	0.07	0.54	0.00
Delay/Veh:	44.2	0.0	43.9	0.0	0.0	0.0	0.0	4.3	3.0	3.1	5.1	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	44.2	0.0	43.9	0.0	0.0	0.0	0.0	4.3	3.0	3.1	5.1	0.0
LOS by Move:	D	A	D	A	A	A	A	A	A	A	A	A
HCM2kAvgQ:	1	0	0	0	0	0	0	8	1	0	11	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 1.8 Worst Case Level Of Service: E [39.0]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume metrics across four directions.

Critical Gap Module: Table with 12 columns for critical gap and follow-up time metrics.

Capacity Module: Table with 12 columns for capacity metrics such as Cnflct Vol, Potent Cap., and Move Cap.

Level Of Service Module: Table with 12 columns for LOS metrics including 2Way95thQ, Control Del, and Shared Del.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 1.3 Worst Case Level Of Service: F [58.4]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume metrics across four approaches.

Critical Gap Module: Table with 12 columns for critical gap and follow-up time metrics.

Capacity Module: Table with 12 columns for capacity metrics such as Cnflct Vol, Potent Cap., and Move Cap.

Level Of Service Module: Table with 12 columns for LOS metrics including 2Way95thQ, Control Del, and Shared LOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 1.3 Worst Case Level Of Service: C [23.3]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	2	0	0	0	1	0	0

Volume Module:

Base Vol:	16	627	4	7	424	13	10	0	63	17	0	9
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	16	627	4	7	424	13	10	0	63	17	0	9
Added Vol:	0	73	0	0	158	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	16	700	4	7	582	13	10	0	63	17	0	9
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	17	737	4	7	613	14	11	0	66	18	0	9
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	17	737	4	7	613	14	11	0	66	18	0	9

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	4.1	xxxx	xxxxxx	7.1	xxxx	6.2	7.1	xxxx	6.2
FollowUpTim:	2.2	xxxx	xxxxxx	2.2	xxxx	xxxxxx	3.5	xxxx	3.3	3.5	xxxx	3.3

Capacity Module:

Cnflct Vol:	626	xxxx	xxxxxx	741	xxxx	xxxxxx	1405	xxxx	306	1094	xxxx	739
Potent Cap.:	965	xxxx	xxxxxx	875	xxxx	xxxxxx	118	xxxx	738	193	xxxx	421
Move Cap.:	965	xxxx	xxxxxx	875	xxxx	xxxxxx	113	xxxx	738	172	xxxx	421
Volume/Cap:	0.02	xxxx	xxxx	0.01	xxxx	xxxx	0.09	xxxx	0.09	0.10	xxxx	0.02

Level Of Service Module:

2Way95thQ:	0.1	xxxx	xxxxxx	0.0	xxxx	xxxxxx	0.3	xxxx	0.3	0.3	xxxx	0.1
Control Del:	8.8	xxxx	xxxxxx	9.1	xxxx	xxxxxx	40.0	xxxx	10.4	28.3	xxxx	13.8
LOS by Move:	A	*	*	A	*	*	E	*	B	D	*	B
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxxx			xxxxxxx			14.4			23.3		
ApproachLOS:	*			*			B			C		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 0.9 Worst Case Level Of Service: D [26.8]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	0	1	0	0	1	0	0

Volume Module:

Base Vol:	52	454	17	9	729	15	5	0	26	6	0	4
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	52	454	17	9	729	15	5	0	26	6	0	4
Added Vol:	0	188	0	0	212	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	52	642	17	9	941	15	5	0	26	6	0	4
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	55	676	18	9	991	16	5	0	27	6	0	4
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	55	676	18	9	991	16	5	0	27	6	0	4

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	4.1	xxxx	xxxxxx	7.1	xxxx	6.2	7.1	xxxx	6.2
FollowUpTim:	2.2	xxxx	xxxxxx	2.2	xxxx	xxxxxx	3.5	xxxx	3.3	3.5	xxxx	3.3

Capacity Module:

Cnflct Vol:	1006	xxxx	xxxxxx	694	xxxx	xxxxxx	1806	xxxx	495	1308	xxxx	685
Potent Cap.:	696	xxxx	xxxxxx	911	xxxx	xxxxxx	62	xxxx	578	138	xxxx	452
Move Cap.:	696	xxxx	xxxxxx	911	xxxx	xxxxxx	57	xxxx	578	122	xxxx	452
Volume/Cap:	0.08	xxxx	xxxxxx	0.01	xxxx	xxxxxx	0.09	xxxx	0.05	0.05	xxxx	0.01

Level Of Service Module:

2Way95thQ:	0.3	xxxx	xxxxxx	0.0	xxxx	xxxxxx	0.3	xxxx	0.1	0.2	xxxx	0.0
Control Del:	10.6	xxxx	xxxxxx	9.0	xxxx	xxxxxx	74.1	xxxx	11.5	36.0	xxxx	13.0
LOS by Move:	B	*	*	A	*	*	F	*	B	E	*	B
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx	xxxxxx		21.6		26.8						
ApproachLOS:	*	*		C		D						

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #16 Soboba Road (NS) at Project North Entrance (NS)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.336
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 15.8
 Optimal Cycle: OPTIMIZED Level of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	18	0	18	0	0	0
Lanes:	1	0	1	0	0	2	0	0	1	0	0	0

Volume Module:

Base Vol:	0	238	0	0	345	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	238	0	0	345	0	0	0	0	0	0	0
Added Vol:	187	23	0	0	34	124	50	0	75	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	187	261	0	0	379	124	50	0	75	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	197	275	0	0	399	131	53	0	79	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	197	275	0	0	399	131	53	0	79	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	197	275	0	0	399	131	53	0	79	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	0.95	0.85	0.82	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	3610	1615	1560	0	1615	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.11	0.14	0.00	0.00	0.11	0.08	0.03	0.00	0.05	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.20	0.50	0.00	0.00	0.30	0.30	0.30	0.00	0.30	0.00	0.00	0.00
Volume/Cap:	0.55	0.29	0.00	0.00	0.37	0.27	0.11	0.00	0.16	0.00	0.00	0.00
Delay/Veh:	23.3	8.9	0.0	0.0	16.7	16.3	15.3	0.0	15.6	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	23.3	8.9	0.0	0.0	16.7	16.3	15.3	0.0	15.6	0.0	0.0	0.0
LOS by Move:	C	A	A	A	B	B	B	A	B	A	A	A
HCM2kAvgQ:	4	3	0	0	3	2	1	0	1	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #16 Soboba Road (NS) at Project North Entrance (NS)

Cycle (sec): 65 Critical Vol./Cap.(X): 0.610
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 18.5
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	18	0	18	0	0	0
Lanes:	1	0	1	0	0	2	0	0	1	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	437	0	0	683	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	437	0	0	683	0	0	0	0	0	0	0
Added Vol:	247	44	0	0	47	165	144	0	216	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	247	481	0	0	730	165	144	0	216	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	260	506	0	0	768	174	152	0	227	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	260	506	0	0	768	174	152	0	227	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	260	506	0	0	768	174	152	0	227	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	0.95	0.85	0.77	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	3610	1615	1461	0	1615	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.14	0.27	0.00	0.00	0.21	0.11	0.10	0.00	0.14	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.22	0.54	0.00	0.00	0.32	0.32	0.28	0.00	0.28	0.00	0.00	0.00
Volume/Cap:	0.66	0.49	0.00	0.00	0.66	0.33	0.37	0.00	0.51	0.00	0.00	0.00
Delay/Veh:	27.5	9.8	0.0	0.0	20.5	17.2	19.5	0.0	20.7	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	27.5	9.8	0.0	0.0	20.5	17.2	19.5	0.0	20.7	0.0	0.0	0.0
LOS by Move:	C	A	A	A	C	B	B	A	C	A	A	A
HCM2kAvgQ:	6	7	0	0	8	3	3	0	4	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #17 Soboba Road (NS) at Project South Entrance (EW)

Cycle (sec): 65 Critical Vol./Cap.(X): 0.398
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 16.4
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	18	0	18	0	0	0
Lanes:	1	0	1	0	0	2	0	0	1	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	238	0	0	345	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	238	0	0	345	0	0	0	0	0	0	0
Added Vol:	249	185	0	0	47	62	25	0	100	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	249	423	0	0	392	62	25	0	100	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	262	445	0	0	413	65	26	0	105	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	262	445	0	0	413	65	26	0	105	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	262	445	0	0	413	65	26	0	105	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	0.95	0.85	0.86	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	3610	1615	1626	0	1615	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.15	0.23	0.00	0.00	0.11	0.04	0.02	0.00	0.07	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.26	0.54	0.00	0.00	0.28	0.28	0.28	0.00	0.28	0.00	0.00	0.00
Volume/Cap:	0.56	0.44	0.00	0.00	0.41	0.15	0.06	0.00	0.24	0.00	0.00	0.00
Delay/Veh:	22.2	9.3	0.0	0.0	19.5	17.9	17.3	0.0	18.5	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	22.2	9.3	0.0	0.0	19.5	17.9	17.3	0.0	18.5	0.0	0.0	0.0
LOS by Move:	C	A	A	A	B	B	B	A	B	A	A	A
HCM2kAvgQ:	5	6	0	0	4	1	0	0	2	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

 Intersection #17 Soboba Road (NS) at Project South Entrance (EW)

Cycle (sec): 70 Critical Vol./Cap.(X): 0.763
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 22.4
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	18	0	18	0	0	0
Lanes:	1	0	1	0	0	2	0	0	1	0	0	0

Volume Module:

Base Vol:	0	437	0	0	683	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	437	0	0	683	0	0	0	0	0	0	0
Added Vol:	330	219	0	0	181	82	72	0	288	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	330	656	0	0	864	82	72	0	288	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	347	691	0	0	909	86	76	0	303	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	347	691	0	0	909	86	76	0	303	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	347	691	0	0	909	86	76	0	303	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	0.95	0.85	0.79	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	3610	1615	1501	0	1615	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.19	0.36	0.00	0.00	0.25	0.05	0.05	0.00	0.19	0.00	0.00	0.00
Crit Moves:	****			****					****			
Green/Cycle:	0.25	0.57	0.00	0.00	0.32	0.32	0.26	0.00	0.26	0.00	0.00	0.00
Volume/Cap:	0.78	0.64	0.00	0.00	0.78	0.16	0.20	0.00	0.73	0.00	0.00	0.00
Delay/Veh:	33.0	11.4	0.0	0.0	24.8	17.0	20.6	0.0	30.3	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	33.0	11.4	0.0	0.0	24.8	17.0	20.6	0.0	30.3	0.0	0.0	0.0
LOS by Move:	C	B	A	A	C	B	C	A	C	A	A	A
HCM2kAvgQ:	9	11	0	0	11	1	1	0	8	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Morning Peak Hour

Level of Service Computation Report

2000 HCM 4-Way Stop Method (Future Volume Alternative)

Intersection #18 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 1.365
Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 86.4
Optimal Cycle: 0 Level of Service: F

Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module table with columns for Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with columns for Vol/Sat, Crit Moves, Delay/Veh, Delay Adj, AdjDel/Veh, LOS by Move, ApproachDel, Delay Adj, ApprAdjDel, LOS by Appr, and AllWayAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Future Volume Alternative)

Intersection #18 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 2.175
Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 362.4
Optimal Cycle: 0 Level Of Service: F

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns representing different traffic movements and 12 rows of volume-related metrics.

Saturation Flow Module table with 12 columns and 3 rows of saturation flow data.

Capacity Analysis Module table with 12 columns and 12 rows of capacity and delay analysis data.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #18 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 105 Critical Vol./Cap.(X): 0.507
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 30.3
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ovl			Ovl			Include		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	1	0	0	1	0	0	1	1	0	0	0	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	298	45	1	2	164	179	191	2	461	1	2	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	298	45	1	2	164	179	191	2	461	1	2	2
Added Vol:	-8	-2	0	0	-29	176	435	0	-70	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	290	43	1	2	135	355	626	2	391	1	2	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	305	45	1	2	142	374	659	2	412	1	2	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	305	45	1	2	142	374	659	2	412	1	2	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	305	45	1	2	142	374	659	2	412	1	2	2

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	0.95	1.00	0.75	0.95	0.95	0.85	0.94	0.94	0.94
Lanes:	1.00	0.98	0.02	1.00	1.00	2.00	1.99	0.01	1.00	0.20	0.40	0.40
Final Sat.:	1805	1851	43	1805	1900	2842	3610	12	1615	356	712	712

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.17	0.02	0.02	0.00	0.07	0.13	0.18	0.18	0.25	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.24	0.27	0.27	0.15	0.17	0.43	0.26	0.26	0.50	0.17	0.17	0.17
Volume/Cap:	0.70	0.09	0.09	0.01	0.44	0.30	0.70	0.70	0.50	0.02	0.02	0.02
Delay/Veh:	41.1	29.1	29.1	38.2	39.9	19.5	37.3	37.3	17.8	36.2	36.2	36.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	41.1	29.1	29.1	38.2	39.9	19.5	37.3	37.3	17.8	36.2	36.2	36.2
LOS by Move:	D	C	C	D	D	B	D	D	B	D	D	D
HCM2kAvgQ:	10	1	1	0	4	4	11	11	9	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 1
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #18 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.749
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 44.0
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ovl			Ovl			Include		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	1	0	0	1	0	0	1	1	0	0	0	1

-----|-----|-----|-----|

Volume Module:

Base Vol:	530	196	1	2	288	400	239	2	599	1	2	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	530	196	1	2	288	400	239	2	599	1	2	2
Added Vol:	-71	-29	0	0	-35	505	577	0	-86	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	459	167	1	2	253	905	816	2	513	1	2	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	483	176	1	2	266	953	859	2	540	1	2	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	483	176	1	2	266	953	859	2	540	1	2	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	483	176	1	2	266	953	859	2	540	1	2	2

-----|-----|-----|-----|

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	0.95	1.00	0.75	0.95	0.95	0.85	0.94	0.94	0.94
Lanes:	1.00	0.99	0.01	1.00	1.00	2.00	1.99	0.01	1.00	0.20	0.40	0.40
Final Sat.:	1805	1887	11	1805	1900	2842	3609	9	1615	356	712	712

-----|-----|-----|-----|

Capacity Analysis Module:

Vol/Sat:	0.27	0.09	0.09	0.00	0.14	0.34	0.24	0.24	0.33	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.30	0.29	0.29	0.16	0.16	0.42	0.26	0.26	0.56	0.15	0.15	0.15
Volume/Cap:	0.90	0.32	0.32	0.01	0.90	0.80	0.90	0.90	0.60	0.02	0.02	0.02
Delay/Veh:	58.8	33.6	33.6	42.2	78.3	34.3	54.1	54.1	18.4	43.5	43.5	43.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	58.8	33.6	33.6	42.2	78.3	34.3	54.1	54.1	18.4	43.5	43.5	43.5
LOS by Move:	E	C	C	D	E	C	D	D	B	D	D	D
HCM2kAvgQ:	21	5	5	0	13	19	19	19	13	0	0	0

Note: Queue reported is the number of cars per lane.

Year 2025 With Project – Alternative 2

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 Sanderson Avenue (NS) at Ramona Expressway (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.846
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 50.5
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Ovl), Min. Green, Lanes.

Volume Module:

Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module:

Table with columns: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with columns: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 Sanderson Avenue (NS) at Ramona Expressway (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.839
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 52.0
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different traffic metrics and 12 rows for various volume and adjustment factors.

Saturation Flow Module:

Table with 12 columns for saturation flow metrics and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis metrics and 10 rows for Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume metrics like Base Vol, Growth Adj, Initial Bse, etc.

Critical Gap Module: Table with 12 columns for gap metrics like Critical Gp, FollowUpTim.

Capacity Module: Table with 12 columns for capacity metrics like Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module: Table with 12 columns for LOS metrics like 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

Average Delay (sec/veh): 10.8 Worst Case Level Of Service: F[186.8]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume components. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with 12 columns for gap metrics. Rows include Critical Gp and FollowUpTim.

Capacity Module: Table with 12 columns for capacity metrics. Rows include Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with 12 columns for LOS metrics. Rows include 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

Cycle (sec): 85 Critical Vol./Cap.(X): 0.916
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 42.9
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	1	0	0	1	0	0	0	0	1	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	1	693	15	148	252	1	1	1	1	15	1	366
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	1	693	15	148	252	1	1	1	1	15	1	366
Added Vol:	0	0	0	72	0	0	0	0	0	0	0	40
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	1	693	15	220	252	1	1	1	1	15	1	406
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	1	729	16	232	265	1	1	1	1	16	1	427
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	1	729	16	232	265	1	1	1	1	16	1	427
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	1	729	16	232	265	1	1	1	1	16	1	427

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	0.95	1.00	1.00	0.93	0.93	0.93	0.86	0.86	0.85
Lanes:	1.00	0.98	0.02	1.00	0.99	0.01	0.34	0.33	0.33	0.94	0.06	1.00
Final Sat.:	1805	1854	40	1805	1891	8	587	587	587	1528	102	1615

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.00	0.39	0.39	0.13	0.14	0.14	0.00	0.00	0.00	0.01	0.01	0.26
Crit Moves:	****			****						****		
Green/Cycle:	0.20	0.43	0.43	0.14	0.37	0.37	0.29	0.29	0.29	0.29	0.29	0.29
Volume/Cap:	0.00	0.92	0.92	0.92	0.38	0.38	0.01	0.01	0.01	0.04	0.04	0.92
Delay/Veh:	27.0	37.7	37.7	70.9	20.2	20.2	21.5	21.5	21.5	21.7	21.7	51.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	27.0	37.7	37.7	70.9	20.2	20.2	21.5	21.5	21.5	21.7	21.7	51.9
LOS by Move:	C	D	D	E	C	C	C	C	C	C	C	D
HCM2kAvgQ:	0	23	23	10	5	5	0	0	0	0	0	15

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

 Cycle (sec): 80 Critical Vol./Cap.(X): 0.853
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 34.4
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	1	0	0	1	0	0	0	0	1	0	1	0

Volume Module:

Base Vol:	1	466	14	380	663	1	1	1	2	21	2	241
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	1	466	14	380	663	1	1	1	2	21	2	241
Added Vol:	0	0	0	54	0	0	0	0	0	0	0	73
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	1	466	14	434	663	1	1	1	2	21	2	314
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	1	491	15	457	698	1	1	1	2	22	2	331
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	1	491	15	457	698	1	1	1	2	22	2	331
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	1	491	15	457	698	1	1	1	2	22	2	331

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	0.95	1.00	1.00	0.91	0.91	0.91	0.83	0.83	0.85
Lanes:	1.00	0.97	0.03	1.00	0.99	0.01	0.25	0.25	0.50	0.91	0.09	1.00
Final Sat.:	1805	1837	55	1805	1897	3	431	431	862	1445	138	1615

Capacity Analysis Module:

Vol/Sat:	0.00	0.27	0.27	0.25	0.37	0.37	0.00	0.00	0.00	0.02	0.02	0.20
Crit Moves:	****			****						****		
Green/Cycle:	0.15	0.31	0.31	0.30	0.46	0.46	0.24	0.24	0.24	0.24	0.24	0.24
Volume/Cap:	0.00	0.85	0.85	0.85	0.81	0.81	0.01	0.01	0.01	0.06	0.06	0.85
Delay/Veh:	28.6	37.2	37.2	39.0	24.5	24.5	23.2	23.2	23.2	23.5	23.5	45.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	28.6	37.2	37.2	39.0	24.5	24.5	23.2	23.2	23.2	23.5	23.5	45.5
LOS by Move:	C	D	D	D	C	C	C	C	C	C	C	D
HCM2kAvgQ:	0	15	15	14	17	17	0	0	0	0	0	11

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 State Street (NS) at Ramona Expressway (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.894
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 56.3
Optimal Cycle: OPTIMIZED Level Of Service: E

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Approach, Movement, Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns and 15 rows including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module table with 12 columns and 5 rows including Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns and 10 rows including Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 State Street (NS) at Ramona Expressway (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.122
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 81.4
Optimal Cycle: OPTIMIZED Level Of Service: F

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns and 15 rows including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module table with 12 columns and 5 rows including Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns and 10 rows including Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 State Street (NS) at Ramona Expressway (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 0.679
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 42.6
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Ovl			Include		
Min. Green:	10	28	28	10	28	28	10	24	24	10	24	24
Lanes:	1	0	2	0	1	1	1	0	2	0	1	1

Volume Module:

Base Vol:	379	683	101	109	453	56	159	279	422	252	502	307
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	379	683	101	109	453	56	159	279	422	252	502	307
Added Vol:	0	0	0	0	0	0	0	24	0	0	13	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	379	683	101	109	453	56	159	303	422	252	515	307
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	399	719	106	115	477	59	167	319	444	265	542	323
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	399	719	106	115	477	59	167	319	444	265	542	323
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	399	719	106	115	477	59	167	319	444	265	542	323

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	1805	3610	1615	1805	3610	1615	1805	3610	1615	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.22	0.20	0.07	0.06	0.13	0.04	0.09	0.09	0.28	0.15	0.15	0.20
Crit Moves:	****			****			****			****		
Green/Cycle:	0.26	0.36	0.36	0.13	0.23	0.23	0.12	0.20	0.46	0.17	0.25	0.25
Volume/Cap:	0.85	0.55	0.18	0.49	0.57	0.16	0.78	0.44	0.60	0.85	0.59	0.78
Delay/Veh:	55.8	30.8	26.2	50.1	41.5	36.8	68.7	42.6	25.5	67.3	40.2	51.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	55.8	30.8	26.2	50.1	41.5	36.8	68.7	42.6	25.5	67.3	40.2	51.2
LOS by Move:	E	C	C	D	D	D	E	D	C	E	D	D
HCM2kAvgQ:	17	11	3	4	9	2	8	6	12	12	10	13

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 State Street (NS) at Ramona Expressway (EW)

Cycle (sec): 110 Critical Vol./Cap.(X): 0.945
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 51.4
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Ovl			Include		
Min. Green:	10	28	28	10	28	28	10	24	24	10	24	24
Lanes:	1	0	2	0	1	1	1	0	2	0	1	1

Volume Module:

Base Vol:	422	750	112	288	744	71	125	699	574	232	388	154
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	422	750	112	288	744	71	125	699	574	232	388	154
Added Vol:	0	0	0	0	0	0	0	18	0	0	24	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	422	750	112	288	744	71	125	717	574	232	412	154
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	444	789	118	303	783	75	132	755	604	244	434	162
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	444	789	118	303	783	75	132	755	604	244	434	162
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	444	789	118	303	783	75	132	755	604	244	434	162

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	1805	3610	1615	1805	3610	1615	1805	3610	1615	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.25	0.22	0.07	0.17	0.22	0.05	0.07	0.21	0.37	0.14	0.12	0.10
Crit Moves:	****			****			****			****		
Green/Cycle:	0.25	0.30	0.30	0.20	0.25	0.25	0.10	0.22	0.46	0.14	0.25	0.25
Volume/Cap:	1.00	0.72	0.24	0.84	0.85	0.18	0.70	0.96	0.81	1.00	0.48	0.40
Delay/Veh:	83.8	36.8	29.2	58.8	46.8	32.3	58.8	64.9	31.6	104.8	35.6	35.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	83.8	36.8	29.2	58.8	46.8	32.3	58.8	64.9	31.6	104.8	35.6	35.1
LOS by Move:	F	D	C	E	D	C	E	E	C	F	D	D
HCM2kAvgQ:	21	14	3	12	16	2	6	18	19	13	7	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.834
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 30.7
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Permitted, Protected), Rights (Include), Min. Green, Lanes.

Table with columns: Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Table with columns: Saturation Flow Module, Sat/Lane, Adjustment, Lanes, Final Sat.

Table with columns: Capacity Analysis Module, Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

 Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.996
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 56.2
 Optimal Cycle: OPTIMIZED Level Of Service: E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	28	28	28	28	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	1	0	1	1	0	1

Volume Module:

Base Vol:	138	581	125	184	712	392	195	1075	172	144	1039	126
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	138	581	125	184	712	392	195	1075	172	144	1039	126
Added Vol:	0	0	9	0	0	0	0	33	0	12	49	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	138	581	134	184	712	392	195	1108	172	156	1088	126
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	145	612	141	194	749	413	205	1166	181	164	1145	133
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	145	612	141	194	749	413	205	1166	181	164	1145	133
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	145	612	141	194	749	413	205	1166	181	164	1145	133

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.18	0.92	0.92	0.32	1.00	0.85	0.95	0.93	0.93	0.95	0.93	0.93
Lanes:	1.00	1.63	0.37	1.00	1.00	1.00	1.00	1.73	0.27	1.00	1.79	0.21
Final Sat.:	343	2851	658	615	1900	1615	1805	3062	475	1805	3184	369

Capacity Analysis Module:

Vol/Sat:	0.42	0.21	0.21	0.31	0.39	0.26	0.11	0.38	0.38	0.09	0.36	0.36
Crit Moves:	****						****			****		
Green/Cycle:	0.42	0.42	0.42	0.42	0.42	0.42	0.11	0.38	0.38	0.09	0.36	0.36
Volume/Cap:	1.00	0.50	0.50	0.74	0.93	0.60	1.00	0.99	0.99	0.99	1.00	1.00
Delay/Veh:	107.8	25.6	25.6	39.8	49.6	28.2	114.7	59.5	59.5	122.2	62.4	62.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	107.8	25.6	25.6	39.8	49.6	28.2	114.7	59.5	59.5	122.2	62.4	62.4
LOS by Move:	F	C	C	D	D	C	F	E	E	F	E	E
HCM2kAvgQ:	9	11	11	8	30	12	12	33	33	10	31	31

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.796
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 29.7
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	28	28	28	28	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	1	0	1	1	0	1

Volume Module:

Base Vol:	103	698	118	130	537	239	220	627	120	73	729	97
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	103	698	118	130	537	239	220	627	120	73	729	97
Added Vol:	0	0	12	0	0	0	0	51	0	7	25	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	103	698	130	130	537	239	220	678	120	80	754	97
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	108	735	137	137	565	252	232	714	126	84	794	102
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	108	735	137	137	565	252	232	714	126	84	794	102
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	108	735	137	137	565	252	232	714	126	84	794	102

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.17	0.93	0.93	0.21	1.00	0.85	0.95	0.93	0.93	0.95	0.95	0.85
Lanes:	1.00	1.69	0.31	1.00	1.00	1.00	1.00	1.70	0.30	1.00	2.00	1.00
Final Sat.:	317	2970	553	407	1900	1615	1805	3000	531	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.34	0.25	0.25	0.34	0.30	0.16	0.13	0.24	0.24	0.05	0.22	0.06
Crit Moves:	****						****			****		
Green/Cycle:	0.40	0.40	0.40	0.40	0.40	0.40	0.15	0.34	0.34	0.12	0.31	0.31
Volume/Cap:	0.85	0.61	0.61	0.83	0.74	0.39	0.85	0.70	0.70	0.38	0.71	0.20
Delay/Veh:	62.0	22.0	22.0	53.1	26.5	19.3	58.1	27.5	27.5	37.5	29.5	23.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	62.0	22.0	22.0	53.1	26.5	19.3	58.1	27.5	27.5	37.5	29.5	23.0
LOS by Move:	E	C	C	D	C	B	E	C	C	D	C	C
HCM2kAvgQ:	5	11	11	6	15	5	9	12	12	3	11	2

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.949
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 43.5
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	28	28	28	28	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	0	2	1	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	138	581	125	184	712	392	195	1075	172	144	1039	126
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	138	581	125	184	712	392	195	1075	172	144	1039	126
Added Vol:	0	0	9	0	0	0	0	33	0	12	49	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	138	581	134	184	712	392	195	1108	172	156	1088	126
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	145	612	141	194	749	413	205	1166	181	164	1145	133
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	145	612	141	194	749	413	205	1166	181	164	1145	133
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	145	612	141	194	749	413	205	1166	181	164	1145	133

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.18	0.92	0.92	0.32	1.00	0.85	0.95	0.89	0.89	0.95	0.95	0.85
Lanes:	1.00	1.63	0.37	1.00	1.00	1.00	1.00	2.60	0.40	1.00	2.00	1.00
Final Sat.:	343	2851	658	615	1900	1615	1805	4400	683	1805	3610	1615

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.42	0.21	0.21	0.31	0.39	0.26	0.11	0.27	0.27	0.09	0.32	0.08
Crit Moves:	****			****			****			****		
Green/Cycle:	0.45	0.45	0.45	0.45	0.45	0.45	0.12	0.34	0.34	0.12	0.33	0.33
Volume/Cap:	0.95	0.48	0.48	0.71	0.88	0.57	0.95	0.78	0.78	0.78	0.95	0.25
Delay/Veh:	89.4	23.7	23.7	35.0	41.4	25.9	99.3	38.2	38.2	69.0	54.3	29.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	89.4	23.7	23.7	35.0	41.4	25.9	99.3	38.2	38.2	69.0	54.3	29.2
LOS by Move:	F	C	C	D	D	C	F	D	D	E	D	C
HCM2kAvgQ:	8	10	10	7	28	12	11	18	18	8	26	4

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

Cycle (sec): 105 Critical Vol./Cap.(X): 1.167

Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 93.7

Optimal Cycle: OPTIMIZED Level Of Service: F

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Split Phase, Permitted), Rights (Include), Min. Green, Lanes.

Volume Module:

Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module:

Table with columns: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with columns: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.817

Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 253.9

Optimal Cycle: OPTIMIZED Level Of Service: F

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 sub-columns for movements (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different volume and adjustment factors. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module:

Table with 12 columns representing saturation flow factors. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis factors. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.679
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 32.7
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Ovl			Include		
Min. Green:	18	18	18	18	18	18	10	21	21	10	21	21
Lanes:	1	0	1	0	1	0	1	0	1	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	337	375	84	30	541	29	18	231	348	49	168	22
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	337	375	84	30	541	29	18	231	348	49	168	22
Added Vol:	0	0	87	24	0	0	0	0	0	45	0	13
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	337	375	171	54	541	29	18	231	348	94	168	35
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	355	395	180	57	569	31	19	243	366	99	177	37
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	355	395	180	57	569	31	19	243	366	99	177	37
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	355	395	180	57	569	31	19	243	366	99	177	37

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	0.85	0.95	0.94	0.94	0.95	1.00	0.85	0.95	0.97	0.97
Lanes:	1.00	1.00	1.00	1.00	1.90	0.10	1.00	1.00	1.00	1.00	0.83	0.17
Final Sat.:	1805	1900	1615	1805	3399	182	1805	1900	1615	1805	1532	319

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.20	0.21	0.11	0.03	0.17	0.17	0.01	0.13	0.23	0.05	0.12	0.12
Crit Moves:	****			****			****			****		
Green/Cycle:	0.26	0.26	0.26	0.21	0.21	0.21	0.11	0.23	0.50	0.11	0.23	0.23
Volume/Cap:	0.74	0.79	0.42	0.15	0.79	0.79	0.09	0.55	0.46	0.49	0.49	0.49
Delay/Veh:	36.5	38.7	28.1	28.9	38.8	38.8	36.1	31.8	15.1	39.5	30.8	30.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	36.5	38.7	28.1	28.9	38.8	38.8	36.1	31.8	15.1	39.5	30.8	30.8
LOS by Move:	D	D	C	C	D	D	D	C	B	D	C	C
HCM2kAvgQ:	11	12	4	1	10	10	1	7	7	3	6	6

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

 Cycle (sec): 100 Critical Vol./Cap.(X): 0.933
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 50.4
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Ovl			Include		
Min. Green:	18	18	18	18	18	18	10	21	21	10	21	21
Lanes:	1	0	1	0	1	1	0	1	0	1	0	0

Volume Module:

Base Vol:	452	436	171	81	522	49	50	356	452	179	304	54
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	452	436	171	81	522	49	50	356	452	179	304	54
Added Vol:	0	0	60	18	0	0	0	0	0	85	0	24
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	452	436	231	99	522	49	50	356	452	264	304	78
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	476	459	243	104	549	52	53	375	476	278	320	82
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	476	459	243	104	549	52	53	375	476	278	320	82
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	476	459	243	104	549	52	53	375	476	278	320	82

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	0.85	0.95	0.94	0.94	0.95	1.00	0.85	0.95	0.97	0.97
Lanes:	1.00	1.00	1.00	1.00	1.83	0.17	1.00	1.00	1.00	1.00	0.80	0.20
Final Sat.:	1805	1900	1615	1805	3257	306	1805	1900	1615	1805	1465	376

Capacity Analysis Module:

Vol/Sat:	0.26	0.24	0.15	0.06	0.17	0.17	0.03	0.20	0.29	0.15	0.22	0.22
Crit Moves:	****			****			****			****		
Green/Cycle:	0.28	0.28	0.28	0.18	0.18	0.18	0.12	0.21	0.49	0.17	0.26	0.26
Volume/Cap:	0.93	0.85	0.53	0.32	0.93	0.93	0.25	0.93	0.60	0.93	0.85	0.85
Delay/Veh:	59.2	46.6	31.5	36.2	60.9	60.9	40.6	67.3	19.4	76.0	48.3	48.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	59.2	46.6	31.5	36.2	60.9	60.9	40.6	67.3	19.4	76.0	48.3	48.3
LOS by Move:	E	D	C	D	E	E	D	E	B	E	D	D
HCM2kAvgQ:	19	16	7	3	14	14	2	16	11	12	14	14

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #6 San Jacinto Street (NS) at Esplanade Avenue (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.612
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 30.5
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Approach, Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different volume metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 12 columns representing saturation flow metrics like Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis metrics like Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #6 San Jacinto Street (NS) at Esplanade Avenue (EW)

Cycle (sec): 105 Critical Vol./Cap.(X): 0.770
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 44.3
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), and Lane counts (1, 0, 2, 0, 1).

Volume Module:

Table with 12 columns for various volume metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module:

Table with 12 columns for saturation flow metrics: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis metrics: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #7 San Jacinto Street (NS) at Menlo Avenue (EW)

Cycle (sec): 65 Critical Vol./Cap.(X): 0.635
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 21.7
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for different traffic movements and 12 rows for various volume and adjustment factors.

Saturation Flow Module table with 12 columns for different traffic movements and 4 rows for saturation flow parameters.

Capacity Analysis Module table with 12 columns for different traffic movements and 12 rows for capacity and delay analysis metrics.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #7 San Jacinto Street (NS) at Menlo Avenue (EW)

Cycle (sec): 85 Critical Vol./Cap.(X): 0.800
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 31.7
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Permitted), Rights (Include), and Lane counts.

Volume Module: Table showing traffic volume metrics such as Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table showing saturation flow metrics like Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table showing capacity analysis metrics such as Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #8 San Jacinto Street (NS) at Devonshire Avenue (EW)

Cycle (sec): 70 Critical Vol./Cap.(X): 0.714
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 23.4
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different traffic volumes and adjustment factors like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 12 columns for saturation flow metrics like Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis metrics like Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #8 San Jacinto Street (NS) at Devonshire Avenue (EW)

Cycle (sec): 80 Critical Vol./Cap.(X): 0.774
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 27.2
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Permitted), Rights (Include), and various timing parameters like Min. Green and Lanes.

Volume Module table showing Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume across movements.

Saturation Flow Module table showing Sat/Lane, Adjustment, Lanes, and Final Sat. values for each movement.

Capacity Analysis Module table showing Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ values.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.834
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 83.8
Optimal Cycle: OPTIMIZED Level Of Service: F

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 sub-columns for Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different volume and adjustment factors. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module:

Table with 12 columns representing saturation flow factors. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis factors. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.022
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 120.4
Optimal Cycle: OPTIMIZED Level Of Service: F

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns representing different traffic volumes and adjustments like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module: Table with 12 columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with 12 columns for Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

Cycle (sec): 110 Critical Vol./Cap.(X): 0.689
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 34.5
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	10	28	28	10	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	0	1	1	0	1	0

Volume Module:

Base Vol:	81	259	77	163	241	149	200	702	43	97	1114	202
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	81	259	77	163	241	149	200	702	43	97	1114	202
Added Vol:	0	12	0	0	7	31	63	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	81	271	77	163	248	180	263	702	43	97	1114	202
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	85	285	81	172	261	189	277	739	45	102	1173	213
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	85	285	81	172	261	189	277	739	45	102	1173	213
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	85	285	81	172	261	189	277	739	45	102	1173	213

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.92	0.92	0.92	1.00	0.85	0.92	0.94	0.94	0.95	0.95	0.85
Lanes:	1.00	1.56	0.44	2.00	1.00	1.00	2.00	1.88	0.12	1.00	2.00	1.00
Final Sat.:	1805	2718	772	3502	1900	1615	3502	3371	206	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.05	0.10	0.10	0.05	0.14	0.12	0.08	0.22	0.22	0.06	0.32	0.13
Crit Moves:	****			****			****			****		
Green/Cycle:	0.09	0.25	0.25	0.09	0.25	0.35	0.10	0.38	0.38	0.13	0.41	0.41
Volume/Cap:	0.52	0.41	0.41	0.54	0.54	0.33	0.79	0.58	0.58	0.42	0.79	0.32
Delay/Veh:	50.7	34.5	34.5	49.6	36.7	26.3	60.2	28.2	28.2	44.9	31.4	22.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	50.7	34.5	34.5	49.6	36.7	26.3	60.2	28.2	28.2	44.9	31.4	22.4
LOS by Move:	D	C	C	D	D	C	E	C	C	D	C	C
HCM2kAvgQ:	3	6	6	4	8	5	7	11	11	4	20	5

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 0.885
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 52.3
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	10	28	28	10	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	1	0	1	1	0	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	79	349	144	457	338	208	223	1167	108	172	1193	75
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	79	349	144	457	338	208	223	1167	108	172	1193	75
Added Vol:	0	9	0	0	12	61	42	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	79	358	144	457	350	269	265	1167	108	172	1193	75
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	83	377	152	481	368	283	279	1228	114	181	1256	79
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	83	377	152	481	368	283	279	1228	114	181	1256	79
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	83	377	152	481	368	283	279	1228	114	181	1256	79

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	0.91	0.92	1.00	0.85	0.92	0.94	0.94	0.95	0.95	0.85
Lanes:	1.00	1.43	0.57	2.00	1.00	1.00	2.00	1.83	0.17	1.00	2.00	1.00
Final Sat.:	1805	2464	991	3502	1900	1615	3502	3261	302	1805	3610	1615

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.05	0.15	0.15	0.14	0.19	0.18	0.08	0.38	0.38	0.10	0.35	0.05
Crit Moves:	****			****			****			****		
Green/Cycle:	0.10	0.23	0.23	0.14	0.28	0.37	0.10	0.39	0.39	0.10	0.40	0.40
Volume/Cap:	0.47	0.66	0.66	0.97	0.70	0.47	0.84	0.97	0.97	0.97	0.88	0.12
Delay/Veh:	53.0	43.6	43.6	83.9	43.2	29.3	70.3	53.5	53.5	110.5	39.9	23.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	53.0	43.6	43.6	83.9	43.2	29.3	70.3	53.5	53.5	110.5	39.9	23.0
LOS by Move:	D	D	D	F	D	C	E	D	D	F	D	C
HCM2kAvgQ:	3	10	10	13	13	8	8	31	31	10	25	2

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

Cycle (sec): 105 Critical Vol./Cap.(X): 0.715
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 38.4
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different volume metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 12 columns representing saturation flow metrics like Sat/Lane, Adjustment, Lanes, etc.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis metrics like Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.010
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 73.8
Optimal Cycle: OPTIMIZED Level Of Service: E

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns representing different traffic volumes and adjustments like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module: Table with 12 columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with 12 columns for Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

 Cycle (sec): 100 Critical Vol./Cap.(X): 0.640
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 32.6
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	24	24	10	24	24	10	28	28	10	28	28
Lanes:	1	0	2	0	1	1	0	1	1	0	2	0

Volume Module:

Base Vol:	64	848	367	98	436	37	42	238	95	225	171	40
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	64	848	367	98	436	37	42	238	95	225	171	40
Added Vol:	0	0	30	24	0	0	0	112	0	19	58	13
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	64	848	397	122	436	37	42	350	95	244	229	53
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	67	893	418	128	459	39	44	368	100	257	241	56
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	67	893	418	128	459	39	44	368	100	257	241	56
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	67	893	418	128	459	39	44	368	100	257	241	56

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.94	0.94	0.95	0.92	0.92	0.92	0.92	0.92
Lanes:	1.00	2.00	1.00	1.00	1.84	0.16	1.00	1.57	0.43	2.00	1.62	0.38
Final Sat.:	1805	3610	1615	1805	3288	279	1805	2748	746	3502	2849	659

Capacity Analysis Module:

Vol/Sat:	0.04	0.25	0.26	0.07	0.14	0.14	0.02	0.13	0.13	0.07	0.08	0.08
Crit Moves:	****			****			****			****		
Green/Cycle:	0.13	0.36	0.36	0.10	0.32	0.32	0.10	0.28	0.28	0.10	0.28	0.28
Volume/Cap:	0.28	0.69	0.72	0.71	0.43	0.43	0.24	0.48	0.48	0.72	0.30	0.30
Delay/Veh:	39.5	28.9	32.2	56.1	26.8	26.8	42.2	30.3	30.3	50.6	28.4	28.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	39.5	28.9	32.2	56.1	26.8	26.8	42.2	30.3	30.3	50.6	28.4	28.4
LOS by Move:	D	C	C	E	C	C	D	C	C	D	C	C
HCM2kAvgQ:	2	13	12	5	6	6	1	7	7	6	4	4

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

Cycle (sec): 115 Critical Vol./Cap.(X): 0.823
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 45.8
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	24	24	10	24	24	10	28	28	10	28	28
Lanes:	1	0	2	0	1	1	0	1	1	0	2	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	86	406	375	182	702	23	23	344	86	560	378	157
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	86	406	375	182	702	23	23	344	86	560	378	157
Added Vol:	0	0	26	18	0	0	0	79	0	32	110	24
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	86	406	401	200	702	23	23	423	86	592	488	181
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	91	427	422	211	739	24	24	445	91	623	514	191
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	91	427	422	211	739	24	24	445	91	623	514	191
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	91	427	422	211	739	24	24	445	91	623	514	191

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.95	0.95	0.95	0.93	0.93	0.92	0.91	0.91
Lanes:	1.00	2.00	1.00	1.00	1.94	0.06	1.00	1.66	0.34	2.00	1.46	0.54
Final Sat.:	1805	3610	1615	1805	3478	114	1805	2925	595	3502	2525	937

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.05	0.12	0.26	0.12	0.21	0.21	0.01	0.15	0.15	0.18	0.20	0.20
Crit Moves:	****			****			****			****		
Green/Cycle:	0.12	0.29	0.29	0.13	0.30	0.30	0.12	0.24	0.24	0.20	0.33	0.33
Volume/Cap:	0.41	0.41	0.90	0.90	0.71	0.71	0.12	0.63	0.63	0.90	0.63	0.63
Delay/Veh:	47.9	33.1	59.4	82.8	38.3	38.3	45.8	40.3	40.3	59.9	34.0	34.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	47.9	33.1	59.4	82.8	38.3	38.3	45.8	40.3	40.3	59.9	34.0	34.0
LOS by Move:	D	C	E	F	D	D	D	D	D	E	C	C
HCM2kAvgQ:	3	6	18	11	14	14	1	10	10	15	12	12

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

Average Delay (sec/veh): 19.3 Worst Case Level Of Service: F[216.2]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control (Uncontrolled/Stop Sign), Rights (Include), and Lanes (1-0-2-0-1).

Volume Module: Table with 12 columns for volume and adjustment factors. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with 12 columns for gap and timing. Rows include Critical Gp and FollowUpTim.

Capacity Module: Table with 12 columns for capacity and volume. Rows include Cnflict Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with 12 columns for LOS and delay. Rows include 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

Average Delay (sec/veh): 88.7 Worst Case Level Of Service: F[501.2]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume metrics across four directions.

Critical Gap Module: Table with 12 columns for gap and follow-up times.

Capacity Module: Table with 12 columns for capacity metrics.

Level Of Service Module: Table with 12 columns for LOS metrics.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.444
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 10.2
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	18	18	18	18	28	28	28	28	28	28
Lanes:	1	0	2	0	1	0	0	1	0	0	0	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	136	1143	8	34	614	106	71	2	113	3	5	1
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	136	1143	8	34	614	106	71	2	113	3	5	1
Added Vol:	0	27	0	0	16	3	3	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	136	1170	8	34	630	109	74	2	113	3	5	1
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	143	1232	8	36	663	115	78	2	119	3	5	1
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	143	1232	8	36	663	115	78	2	119	3	5	1
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	143	1232	8	36	663	115	78	2	119	3	5	1

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.37	0.95	0.85	0.18	0.95	0.85	0.72	0.72	0.85	0.94	0.94	0.94
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	0.97	0.03	1.00	0.33	0.56	0.11
Final Sat.:	705	3610	1615	348	3610	1615	1338	36	1615	594	990	198

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.20	0.34	0.01	0.10	0.18	0.07	0.06	0.06	0.07	0.01	0.01	0.01
Crit Moves:	****						****					
Green/Cycle:	0.70	0.70	0.70	0.70	0.70	0.70	0.23	0.23	0.23	0.23	0.23	0.23
Volume/Cap:	0.29	0.49	0.01	0.15	0.26	0.10	0.25	0.25	0.32	0.02	0.02	0.02
Delay/Veh:	7.1	8.3	5.4	6.3	6.7	5.9	37.9	37.9	38.6	35.5	35.5	35.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	7.1	8.3	5.4	6.3	6.7	5.9	37.9	37.9	38.6	35.5	35.5	35.5
LOS by Move:	A	A	A	A	A	A	D	D	D	D	D	D
HCM2kAvgQ:	2	11	0	1	5	1	2	2	4	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.688
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 14.4
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	18	18	18	18	28	28	28	28	28	28
Lanes:	1	0	2	0	1	0	0	1	0	0	0	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	109	649	1	61	1180	69	109	1	331	4	2	6
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	109	649	1	61	1180	69	109	1	331	4	2	6
Added Vol:	0	22	0	0	28	4	4	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	109	671	1	61	1208	73	113	1	331	4	2	6
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	115	706	1	64	1272	77	119	1	348	4	2	6
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	115	706	1	64	1272	77	119	1	348	4	2	6
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	115	706	1	64	1272	77	119	1	348	4	2	6

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.15	0.95	0.85	0.34	0.95	0.85	0.71	0.71	0.85	0.88	0.88	0.88
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	0.99	0.01	1.00	0.33	0.17	0.50
Final Sat.:	279	3610	1615	642	3610	1615	1333	12	1615	560	280	839

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.41	0.20	0.00	0.10	0.35	0.05	0.09	0.09	0.22	0.01	0.01	0.01
Crit Moves:	****			****			****			****		
Green/Cycle:	0.60	0.60	0.60	0.60	0.60	0.60	0.31	0.31	0.31	0.31	0.31	0.31
Volume/Cap:	0.69	0.33	0.00	0.17	0.59	0.08	0.28	0.28	0.69	0.02	0.02	0.02
Delay/Veh:	23.8	9.2	7.3	8.3	11.7	7.7	23.6	23.6	31.0	21.4	21.4	21.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	23.8	9.2	7.3	8.3	11.7	7.7	23.6	23.6	31.0	21.4	21.4	21.4
LOS by Move:	C	A	A	A	B	A	C	C	C	C	C	C
HCM2kAvgQ:	4	5	0	1	12	1	3	3	10	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #12 Mountain Avenue (NS) at Esplanade Avenue (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.870
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 28.1
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns representing different traffic movements and 12 rows of volume-related metrics like Base Vol, Growth Adj, etc.

Saturation Flow Module: Table with 12 columns and 4 rows showing Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with 12 columns and 10 rows showing Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #12 Mountain Avenue (NS) at Esplanade Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.051
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 60.3
Optimal Cycle: OPTIMIZED Level Of Service: E

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different traffic movements. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module:

Table with 12 columns representing different traffic movements. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns representing different traffic movements. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #12 Mountain Avenue (NS) at Esplanade Avenue (EW)

Cycle (sec): 80 Critical Vol./Cap.(X): 0.860
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 25.4
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	24	0	0	24	24	21	0	21	0	0	0
Lanes:	1	0	1	0	0	2	0	0	1	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	232	890	0	0	464	139	309	0	122	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	232	890	0	0	464	139	309	0	122	0	0	0
Added Vol:	0	24	0	0	13	3	3	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	232	914	0	0	477	142	312	0	122	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	244	962	0	0	502	149	328	0	128	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	244	962	0	0	502	149	328	0	128	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	244	962	0	0	502	149	328	0	128	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	0.95	0.85	0.77	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	3610	1615	1461	0	1615	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.14	0.51	0.00	0.00	0.14	0.09	0.22	0.00	0.08	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.29	0.59	0.00	0.00	0.30	0.30	0.26	0.00	0.26	0.00	0.00	0.00
Volume/Cap:	0.47	0.86	0.00	0.00	0.46	0.31	0.86	0.00	0.30	0.00	0.00	0.00
Delay/Veh:	24.2	20.8	0.0	0.0	23.1	22.0	45.1	0.0	24.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	24.2	20.8	0.0	0.0	23.1	22.0	45.1	0.0	24.0	0.0	0.0	0.0
LOS by Move:	C	C	A	A	C	C	D	A	C	A	A	A
HCM2kAvgQ:	5	23	0	0	6	3	11	0	3	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #12 Mountain Avenue (NS) at Esplanade Avenue (EW)

Cycle (sec): 70 Critical Vol./Cap.(X): 0.816
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 25.1
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	24	0	0	24	24	21	0	21	0	0	0
Lanes:	1	0	1	0	0	2	0	0	1	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	244	462	0	0	1003	401	249	0	360	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	244	462	0	0	1003	401	249	0	360	0	0	0
Added Vol:	0	18	0	0	24	4	4	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	244	480	0	0	1027	405	253	0	360	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	257	505	0	0	1081	426	266	0	379	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	257	505	0	0	1081	426	266	0	379	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	257	505	0	0	1081	426	266	0	379	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	0.95	0.85	0.77	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	3610	1615	1461	0	1615	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.14	0.27	0.00	0.00	0.30	0.26	0.18	0.00	0.23	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.17	0.53	0.00	0.00	0.36	0.36	0.30	0.00	0.30	0.00	0.00	0.00
Volume/Cap:	0.84	0.50	0.00	0.00	0.84	0.74	0.61	0.00	0.78	0.00	0.00	0.00
Delay/Veh:	45.8	11.0	0.0	0.0	25.5	24.5	23.4	0.0	30.5	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	45.8	11.0	0.0	0.0	25.5	24.5	23.4	0.0	30.5	0.0	0.0	0.0
LOS by Move:	D	B	A	A	C	C	C	A	C	A	A	A
HCM2kAvgQ:	8	7	0	0	14	10	6	0	10	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

Average Delay (sec/veh): 23.0 Worst Case Level Of Service: F[132.0]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Table with 13 columns representing different traffic volumes and adjustments. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Table with 13 columns for Critical Gap Module. Rows include Critical Gp and FollowUpTim.

Table with 13 columns for Capacity Module. Rows include Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Table with 13 columns for Level Of Service Module. Rows include 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Evening Peak Hour

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

Average Delay (sec/veh): 24.8 Worst Case Level Of Service: F[217.6]

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, Lanes.

Table with columns: Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume.

Table with columns: Critical Gap Module, Critical Gp, FollowUpTim.

Table with columns: Capacity Module, Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Table with columns: Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

Cycle (sec): 70 Critical Vol./Cap.(X): 0.610
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 10.9
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	1	0	0	0	0	0	0	0	1	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	194	0	24	0	0	0	0	359	109	75	548	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	194	0	24	0	0	0	0	359	109	75	548	0
Added Vol:	12	0	0	0	0	0	0	7	7	0	12	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	206	0	24	0	0	0	0	366	116	75	560	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	217	0	25	0	0	0	0	385	122	79	589	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	217	0	25	0	0	0	0	385	122	79	589	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	217	0	25	0	0	0	0	385	122	79	589	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.77	1.00	0.85	1.00	1.00	1.00	1.00	0.97	0.97	0.90	0.90	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.76	0.24	0.12	0.88	0.00
Final Sat.:	1461	0	1615	0	0	0	0	1395	442	201	1503	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.15	0.00	0.02	0.00	0.00	0.00	0.00	0.28	0.28	0.39	0.39	0.00
Crit Moves:	****									****		
Green/Cycle:	0.26	0.00	0.26	0.00	0.00	0.00	0.00	0.63	0.63	0.63	0.63	0.00
Volume/Cap:	0.58	0.00	0.06	0.00	0.00	0.00	0.00	0.44	0.44	0.62	0.62	0.00
Delay/Veh:	24.9	0.0	19.7	0.0	0.0	0.0	0.0	6.9	6.9	9.1	9.1	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	24.9	0.0	19.7	0.0	0.0	0.0	0.0	6.9	6.9	9.1	9.1	0.0
LOS by Move:	C	A	B	A	A	A	A	A	A	A	A	A
HCM2kAvgQ:	5	0	0	0	0	0	0	6	6	10	10	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

Cycle (sec): 115 Critical Vol./Cap.(X): 0.781
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 13.7
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	1	0	0	0	0	0	0	0	0	1	0	0

Volume Module:

Base Vol:	138	0	56	0	0	0	0	818	242	44	465	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	138	0	56	0	0	0	0	818	242	44	465	0
Added Vol:	9	0	0	0	0	0	0	12	12	0	9	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	147	0	56	0	0	0	0	830	254	44	474	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	155	0	59	0	0	0	0	874	267	46	499	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	155	0	59	0	0	0	0	874	267	46	499	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	155	0	59	0	0	0	0	874	267	46	499	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.77	1.00	0.85	1.00	1.00	1.00	1.00	0.97	0.97	0.85	0.85	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.77	0.23	0.08	0.92	0.00
Final Sat.:	1461	0	1615	0	0	0	0	1408	431	136	1469	0

Capacity Analysis Module:

Vol/Sat:	0.11	0.00	0.04	0.00	0.00	0.00	0.00	0.62	0.62	0.34	0.34	0.00
Crit Moves:	****			****								
Green/Cycle:	0.16	0.00	0.16	0.00	0.00	0.00	0.00	0.77	0.77	0.77	0.77	0.00
Volume/Cap:	0.68	0.00	0.23	0.00	0.00	0.00	0.00	0.80	0.80	0.44	0.44	0.00
Delay/Veh:	53.6	0.0	42.9	0.0	0.0	0.0	0.0	11.1	11.1	4.7	4.7	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	53.6	0.0	42.9	0.0	0.0	0.0	0.0	11.1	11.1	4.7	4.7	0.0
LOS by Move:	D	A	D	A	A	A	A	B	B	A	A	A
HCM2kAvgQ:	6	0	2	0	0	0	0	25	25	7	7	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 1.6 Worst Case Level Of Service: F [73.2]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume components like Base Vol, Growth Adj, Initial Bse, etc.

Critical Gap Module: Table with 12 columns for gap metrics like Critical Gp, FollowUpTim.

Capacity Module: Table with 12 columns for capacity metrics like Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module: Table with 12 columns for LOS metrics like 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 1.4 Worst Case Level Of Service: F[74.4]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Table with 12 columns representing traffic volumes. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Table with 12 columns. Rows include Critical Gap Module and FollowUpTim.

Table with 12 columns. Rows include Capacity Module metrics like Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Table with 12 columns. Rows include Level Of Service Module metrics like 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: B [14.8]

Approach:	North Bound			South Bound			East Bound			West Bound						
Movement:	L	T	R	L	T	R	L	T	R	L	T	R				
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled						
Rights:	Include			Include			Include			Include						
Lanes:	1	0	0	0	0	0	0	0	2	0	1	1	0	2	0	0

Volume Module:

Base Vol:	0	0	42	0	0	0	0	1089	13	9	622	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	42	0	0	0	0	1089	13	9	622	0
Added Vol:	0	0	0	0	0	0	0	166	0	0	91	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	42	0	0	0	0	1255	13	9	713	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	0	44	0	0	0	0	1321	14	9	751	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	44	0	0	0	0	1321	14	9	751	0

Critical Gap Module:

Critical Gp:	6.8	xxxx	6.9	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	4.1	xxxx	xxxxx
FollowUpTim:	3.5	xxxx	3.3	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	2.2	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	1715	xxxx	661	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	1335	xxxx	xxxxx
Potent Cap.:	83	xxxx	410	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	523	xxxx	xxxxx
Move Cap.:	82	xxxx	410	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	523	xxxx	xxxxx
Volume/Cap:	0.00	xxxx	0.11	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.02	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	0.4	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.1	xxxx	xxxxx
Control Del:	xxxxx	xxxx	14.8	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	12.0	xxxx	xxxxx
LOS by Move:	*	*	B	*	*	*	*	*	*	B	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	14.8			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	B			*			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 0.3 Worst Case Level Of Service: B [11.4]

Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, and Lanes.

Table with columns for Volume Module. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Table with columns for Critical Gap Module. Rows include Critical Gp and FollowUpTim.

Table with columns for Capacity Module. Rows include Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Table with columns for Level Of Service Module. Rows include 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.410
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 5.0
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	1	0	0	0	0	0	0	2	0	1	0	0

Volume Module:

Base Vol:	25	0	17	0	0	0	0	1089	13	9	597	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	25	0	17	0	0	0	0	1089	13	9	597	0
Added Vol:	0	0	0	0	0	0	0	166	0	0	91	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	25	0	17	0	0	0	0	1255	13	9	688	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	26	0	18	0	0	0	0	1321	14	9	724	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	26	0	18	0	0	0	0	1321	14	9	724	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	26	0	18	0	0	0	0	1321	14	9	724	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.82	1.00	0.85	1.00	1.00	1.00	1.00	0.95	0.85	0.18	0.95	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	2.00	1.00	1.00	2.00	0.00
Final Sat.:	1560	0	1615	0	0	0	0	3610	1615	342	3610	0

Capacity Analysis Module:

Vol/Sat:	0.02	0.00	0.01	0.00	0.00	0.00	0.00	0.37	0.01	0.03	0.20	0.00
Crit Moves:	****			****								
Green/Cycle:	0.15	0.00	0.15	0.00	0.00	0.00	0.00	0.78	0.78	0.78	0.78	0.00
Volume/Cap:	0.11	0.00	0.07	0.00	0.00	0.00	0.00	0.47	0.01	0.04	0.26	0.00
Delay/Veh:	44.3	0.0	44.0	0.0	0.0	0.0	0.0	4.6	2.8	3.0	3.6	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	44.3	0.0	44.0	0.0	0.0	0.0	0.0	4.6	2.8	3.0	3.6	0.0
LOS by Move:	D	A	D	A	A	A	A	A	A	A	A	A
HCM2kAvgQ:	1	0	1	0	0	0	0	9	0	0	4	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.388
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 4.8
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	1	0	0	0	0	0	0	2	0	1	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	22	0	14	0	0	0	0	654	66	22	1027	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	22	0	14	0	0	0	0	654	66	22	1027	0
Added Vol:	0	0	0	0	0	0	0	123	0	0	166	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	22	0	14	0	0	0	0	777	66	22	1193	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	23	0	15	0	0	0	0	818	69	23	1256	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	23	0	15	0	0	0	0	818	69	23	1256	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	23	0	15	0	0	0	0	818	69	23	1256	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.83	1.00	0.85	1.00	1.00	1.00	1.00	0.95	0.85	0.32	0.95	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	2.00	1.00	1.00	2.00	0.00
Final Sat.:	1575	0	1615	0	0	0	0	3610	1615	616	3610	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.01	0.00	0.01	0.00	0.00	0.00	0.00	0.23	0.04	0.04	0.35	0.00
Crit Moves:	****									****		
Green/Cycle:	0.15	0.00	0.15	0.00	0.00	0.00	0.00	0.78	0.78	0.78	0.78	0.00
Volume/Cap:	0.10	0.00	0.06	0.00	0.00	0.00	0.00	0.29	0.05	0.05	0.44	0.00
Delay/Veh:	44.2	0.0	43.9	0.0	0.0	0.0	0.0	3.7	3.0	3.0	4.4	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	44.2	0.0	43.9	0.0	0.0	0.0	0.0	3.7	3.0	3.0	4.4	0.0
LOS by Move:	D	A	D	A	A	A	A	A	A	A	A	A
HCM2kAvgQ:	1	0	0	0	0	0	0	4	1	0	8	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 1.7 Worst Case Level Of Service: D [31.7]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume metrics across four directions.

Critical Gap Module: Table with 12 columns for gap and follow-up times.

Capacity Module: Table with 12 columns for capacity and volume/capacity ratios.

Level Of Service Module: Table with 12 columns for LOS metrics and approach details.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 1.2 Worst Case Level Of Service: E [36.8]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume metrics across four approaches. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with 12 columns for gap metrics across four approaches. Rows include Critical Gp and FollowUpTim.

Capacity Module: Table with 12 columns for capacity metrics across four approaches. Rows include Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with 12 columns for LOS metrics across four approaches. Rows include 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 1.3 Worst Case Level Of Service: C [21.0]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	0	1	0	0	1	0	0

Volume Module:

Base Vol:	16	627	4	7	424	13	10	0	63	17	0	9
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	16	627	4	7	424	13	10	0	63	17	0	9
Added Vol:	0	40	0	0	72	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	16	667	4	7	496	13	10	0	63	17	0	9
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	17	702	4	7	522	14	11	0	66	18	0	9
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	17	702	4	7	522	14	11	0	66	18	0	9

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	4.1	xxxx	xxxxxx	7.1	xxxx	6.2	7.1	xxxx	6.2
FollowUpTim:	2.2	xxxx	xxxxxx	2.2	xxxx	xxxxxx	3.5	xxxx	3.3	3.5	xxxx	3.3

Capacity Module:

Cnflct Vol:	536	xxxx	xxxxxx	706	xxxx	xxxxxx	1279	xxxx	261	1014	xxxx	704
Potent Cap.:	1042	xxxx	xxxxxx	901	xxxx	xxxxxx	144	xxxx	782	219	xxxx	440
Move Cap.:	1042	xxxx	xxxxxx	901	xxxx	xxxxxx	138	xxxx	782	197	xxxx	440
Volume/Cap:	0.02	xxxx	xxxx	0.01	xxxx	xxxx	0.08	xxxx	0.08	0.09	xxxx	0.02

Level Of Service Module:

2Way95thQ:	0.0	xxxx	xxxxxx	0.0	xxxx	xxxxxx	0.2	xxxx	0.3	0.3	xxxx	0.1
Control Del:	8.5	xxxx	xxxxxx	9.0	xxxx	xxxxxx	33.1	xxxx	10.0	25.1	xxxx	13.4
LOS by Move:	A	*	*	A	*	*	D	*	B	D	*	B
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	13.2	xxxxxx	xxxxxx	21.0	xxxxxx	xxxxxx	
ApproachLOS:	*	*	*	*	*	B	B	B	C	C	C	

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 0.9 Worst Case Level Of Service: C [20.8]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	0	1	0	0	1	0	0

Volume Module:

Base Vol:	52	454	17	9	729	15	5	0	26	6	0	4
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	52	454	17	9	729	15	5	0	26	6	0	4
Added Vol:	0	73	0	0	54	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	52	527	17	9	783	15	5	0	26	6	0	4
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	55	555	18	9	824	16	5	0	27	6	0	4
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	55	555	18	9	824	16	5	0	27	6	0	4

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	4.1	xxxx	xxxxxx	7.1	xxxx	6.2	7.1	xxxx	6.2
FollowUpTim:	2.2	xxxx	xxxxxx	2.2	xxxx	xxxxxx	3.5	xxxx	3.3	3.5	xxxx	3.3

Capacity Module:

Cnflct Vol:	840	xxxx	xxxxxx	573	xxxx	xxxxxx	1518	xxxx	412	1104	xxxx	564
Potent Cap.:	804	xxxx	xxxxxx	1010	xxxx	xxxxxx	99	xxxx	644	190	xxxx	529
Move Cap.:	804	xxxx	xxxxxx	1010	xxxx	xxxxxx	92	xxxx	644	171	xxxx	529
Volume/Cap:	0.07	xxxx	xxxx	0.01	xxxx	xxxx	0.06	xxxx	0.04	0.04	xxxx	0.01

Level Of Service Module:

2Way95thQ:	0.2	xxxx	xxxxxx	0.0	xxxx	xxxxxx	0.2	xxxx	0.1	0.1	xxxx	0.0
Control Del:	9.8	xxxx	xxxxxx	8.6	xxxx	xxxxxx	46.5	xxxx	10.8	26.8	xxxx	11.9
LOS by Move:	A	*	*	A	*	*	E	*	B	D	*	B
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	16.6	xxxxxx	xxxxxx	20.8	xxxxxx	
ApproachLOS:	*	*	*	*	*	*	C	*	*	C	*	

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #16 Soboba Road (NS) at Project North Access (EW)

Average Delay (sec/veh): 2.0 Worst Case Level Of Service: C [17.4]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	0	1	0	0	1	0	0	0	0

Volume Module:

Base Vol:	0	238	0	0	345	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	238	0	0	345	0	0	0	0	0	0	0
Added Vol:	136	18	0	0	18	45	19	0	7	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	136	256	0	0	363	45	19	0	7	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	143	269	0	0	382	47	20	0	7	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	143	269	0	0	382	47	20	0	7	0	0	0

Critical Gap Module:

Critical Gp:	4.1	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	6.4	6.5	6.2	XXXXXX	XXXX	XXXXXX
FollowUpTim:	2.2	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	3.5	4.0	3.3	XXXXXX	XXXX	XXXXXX

Capacity Module:

Cnflict Vol:	429	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	962	962	215	XXXX	XXXX	XXXXXX
Potent Cap.:	1141	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	286	258	830	XXXX	XXXX	XXXXXX
Move Cap.:	1141	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	259	226	830	XXXX	XXXX	XXXXXX
Volume/Cap:	0.13	XXXX	XXXX	XXXX	XXXX	XXXX	0.08	0.00	0.01	XXXX	XXXX	XXXX

Level Of Service Module:

2Way95thQ:	0.4	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	XXXX	XXXX	XXXXXX
Control Del:	8.6	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
LOS by Move:	A	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	XXXX	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	XXXX	318	XXXXXX	XXXX	XXXX	XXXXXX
SharedQueue:	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	0.3	XXXXXX	XXXXXX	XXXX	XXXXXX
Shrd ConDel:	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	17.4	XXXXXX	XXXXXX	XXXX	XXXXXX
Shared LOS:	*	*	*	*	*	*	*	C	*	*	*	*
ApproachDel:	XXXXXX			XXXXXX			17.4			XXXXXX		
ApproachLOS:	*			*			C			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #16 Soboba Road (NS) at Project North Access (EW)

Average Delay (sec/veh): 2.0 Worst Case Level Of Service: D [33.5]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	0	1	0	0	1	0	0	0	0

Volume Module:

Base Vol:	0	437	0	0	683	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	437	0	0	683	0	0	0	0	0	0	0
Added Vol:	76	24	0	0	24	25	41	0	16	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	76	461	0	0	707	25	41	0	16	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	80	485	0	0	744	26	43	0	17	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	80	485	0	0	744	26	43	0	17	0	0	0

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	6.5	6.2	xxxxxx	xxxx	xxxxxx
FollowUpTim:	2.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	4.0	3.3	xxxxxx	xxxx	xxxxxx

Capacity Module:

Cnflct Vol:	771	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	1403	1403	385	xxxx	xxxx	xxxxxx
Potent Cap.:	853	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	156	141	667	xxxx	xxxx	xxxxxx
Move Cap.:	853	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	145	128	667	xxxx	xxxx	xxxxxx
Volume/Cap:	0.09	xxxx	xxxx	xxxx	xxxx	xxxx	0.30	0.00	0.03	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	0.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	9.7	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	A	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxx	185	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	1.3	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	33.5	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	D	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			33.5			xxxxxx		
ApproachLOS:	*			*			D			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Future Volume Alternative)

 Intersection #17 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.828
 Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 26.8
 Optimal Cycle: 0 Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0	0	1	0	0	1	0	1	0	0	0	1

Volume Module:

Base Vol:	298	45	1	2	164	179	191	2	461	1	2	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	298	45	1	2	164	179	191	2	461	1	2	2
Added Vol:	39	24	0	0	22	4	130	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	337	69	1	2	186	183	321	2	461	1	2	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	355	73	1	2	196	193	338	2	485	1	2	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	355	73	1	2	196	193	338	2	485	1	2	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	355	73	1	2	196	193	338	2	485	1	2	2

Saturation Flow Module:

Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.82	0.17	0.01	0.01	0.50	0.49	0.99	0.01	1.00	0.20	0.40	0.40
Final Sat.:	449	92	1	3	285	280	487	3	586	81	161	161

Capacity Analysis Module:

Vol/Sat:	0.79	0.79	0.79	0.69	0.69	0.69	0.69	0.69	0.83	0.01	0.01	0.01
Crit Moves:	****			****			****			****		
Delay/Veh:	29.1	29.1	29.1	21.2	21.2	21.2	24.7	24.7	31.1	10.8	10.8	10.8
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	29.1	29.1	29.1	21.2	21.2	21.2	24.7	24.7	31.1	10.8	10.8	10.8
LOS by Move:	D	D	D	C	C	C	C	C	D	B	B	B
ApproachDel:	29.1			21.2			28.4			10.8		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	29.1			21.2			28.4			10.8		
LOS by Appr:	D			C			D			B		
AllWayAvgQ:	2.9	2.9	2.9	1.9	1.9	1.9	2.0	2.0	3.6	0.0	0.0	0.0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Future Volume Alternative)

 Intersection #17 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 1.651
 Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 201.1
 Optimal Cycle: 0 Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0	0	1	0	0	0	0	1	0	0	1	0

Volume Module:

Base Vol:	530	196	1	2	288	400	239	2	599	1	2	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	530	196	1	2	288	400	239	2	599	1	2	2
Added Vol:	52	25	0	0	32	8	75	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	582	221	1	2	320	408	314	2	599	1	2	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	613	233	1	2	337	429	331	2	631	1	2	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	613	233	1	2	337	429	331	2	631	1	2	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	613	233	1	2	337	429	331	2	631	1	2	2

Saturation Flow Module:

Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.72	0.27	0.01	0.01	0.43	0.56	0.99	0.01	1.00	0.20	0.40	0.40
Final Sat.:	371	141	1	2	241	307	450	3	532	80	160	160

Capacity Analysis Module:

Vol/Sat:	1.65	1.65	1.65	1.40	1.40	1.40	0.74	0.74	1.19	0.01	0.01	0.01
Crit Moves:	****			****			****			****		
Delay/Veh:	319.8	320	319.8	208.8	209	208.8	29.8	29.8	124.3	12.1	12.1	12.1
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	319.8	320	319.8	208.8	209	208.8	29.8	29.8	124.3	12.1	12.1	12.1
LOS by Move:	F	F	F	F	F	F	D	D	F	B	B	B
ApproachDel:	319.8			208.8			91.6			12.1		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	319.8			208.8			91.6			12.1		
LOS by Appr:	F			F			F			B		
AllWayAvgQ:	44.1	44.1	44.1	30.4	30.4	30.4	2.4	2.4	17.0	0.0	0.0	0.0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

 Intersection #17 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 110 Critical Vol./Cap.(X): 0.574
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 32.9
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ovl			Ovl			Include		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	1	0	0	1	0	0	0	1	0	0	0	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	298	45	1	2	164	179	191	2	461	1	2	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	298	45	1	2	164	179	191	2	461	1	2	2
Added Vol:	39	24	0	0	22	4	130	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	337	69	1	2	186	183	321	2	461	1	2	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	355	73	1	2	196	193	338	2	485	1	2	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	355	73	1	2	196	193	338	2	485	1	2	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	355	73	1	2	196	193	338	2	485	1	2	2

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	0.95	1.00	0.85	0.95	0.95	0.85	0.94	0.94	0.94
Lanes:	1.00	0.99	0.01	1.00	1.00	1.00	0.99	0.01	1.00	0.20	0.40	0.40
Final Sat.:	1805	1869	27	1805	1900	1615	1799	11	1615	356	712	712

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.20	0.04	0.04	0.00	0.10	0.12	0.19	0.19	0.30	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.27	0.28	0.28	0.15	0.16	0.42	0.26	0.26	0.53	0.16	0.16	0.16
Volume/Cap:	0.73	0.14	0.14	0.01	0.63	0.28	0.73	0.73	0.57	0.02	0.02	0.02
Delay/Veh:	42.0	29.9	29.9	39.4	47.0	21.1	43.1	43.1	18.5	38.6	38.6	38.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	42.0	29.9	29.9	39.4	47.0	21.1	43.1	43.1	18.5	38.6	38.6	38.6
LOS by Move:	D	C	C	D	D	C	D	D	B	D	D	D
HCM2kAvgQ:	12	2	2	0	7	4	12	12	11	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #17 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.812
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 54.8
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ovl			Ovl			Include		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	1	0	0	1	0	0	1	0	0	1	0	0

Volume Module:

Base Vol:	530	196	1	2	288	400	239	2	599	1	2	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	530	196	1	2	288	400	239	2	599	1	2	2
Added Vol:	52	25	0	0	32	8	75	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	582	221	1	2	320	408	314	2	599	1	2	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	613	233	1	2	337	429	331	2	631	1	2	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	613	233	1	2	337	429	331	2	631	1	2	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	613	233	1	2	337	429	331	2	631	1	2	2

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	0.95	1.00	0.85	0.95	0.95	0.85	0.94	0.94	0.94
Lanes:	1.00	0.99	0.01	1.00	1.00	1.00	0.99	0.01	1.00	0.20	0.40	0.40
Final Sat.:	1805	1890	9	1805	1900	1615	1799	11	1615	356	712	712

Capacity Analysis Module:

Vol/Sat:	0.34	0.12	0.12	0.00	0.18	0.27	0.18	0.18	0.39	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.35	0.34	0.34	0.19	0.18	0.37	0.19	0.19	0.54	0.15	0.15	0.15
Volume/Cap:	0.98	0.36	0.36	0.01	0.98	0.72	0.98	0.98	0.73	0.02	0.02	0.02
Delay/Veh:	68.8	30.2	30.2	39.5	91.1	36.8	91.0	91.0	24.4	43.5	43.5	43.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	68.8	30.2	30.2	39.5	91.1	36.8	91.0	91.0	24.4	43.5	43.5	43.5
LOS by Move:	E	C	C	D	F	D	F	F	C	D	D	D
HCM2kAvgQ:	28	6	6	0	17	15	17	17	19	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #18 Soboba Road (NS) at Project South Access (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.410
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 6.4
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	0	0	18	18	18	0	18	0	0	0
Lanes:	1	0	1	0	0	0	1	0	0	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	343	0	0	626	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	343	0	0	626	0	0	0	0	0	0	0
Added Vol:	3	9	0	0	4	18	54	0	3	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	3	352	0	0	630	18	54	0	3	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	3	352	0	0	630	18	54	0	3	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	3	352	0	0	630	18	54	0	3	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	3	352	0	0	630	18	54	0	3	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.88	1.00	1.00	1.00	1.00	1.00	0.74	1.00	0.74	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	0.97	0.03	0.95	0.00	0.05	0.00	0.00	0.00
Final Sat.:	1680	1900	0	0	1840	53	1328	0	74	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.00	0.19	0.00	0.00	0.34	0.34	0.04	0.00	0.04	0.00	0.00	0.00
Crit Moves:				****			****					
Green/Cycle:	0.78	0.78	0.00	0.00	0.78	0.78	0.15	0.00	0.15	0.00	0.00	0.00
Volume/Cap:	0.00	0.24	0.00	0.00	0.44	0.44	0.27	0.00	0.27	0.00	0.00	0.00
Delay/Veh:	2.8	3.5	0.0	0.0	4.5	4.5	45.9	0.0	45.9	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	2.8	3.5	0.0	0.0	4.5	4.5	45.9	0.0	45.9	0.0	0.0	0.0
LOS by Move:	A	A	A	A	A	A	D	A	D	A	A	A
HCM2kAvgQ:	0	3	0	0	8	8	2	0	2	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #18 Soboba Road (NS) at Project South Access (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.580
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 7.5
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	0	0	18	18	18	0	18	0	0	0
Lanes:	1	0	1	0	0	0	1	0	0	0	0	0

Volume Module:

Base Vol:	0	726	0	0	888	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	726	0	0	888	0	0	0	0	0	0	0
Added Vol:	4	5	0	0	8	24	72	0	4	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	4	731	0	0	896	24	72	0	4	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	4	731	0	0	896	24	72	0	4	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	4	731	0	0	896	24	72	0	4	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	4	731	0	0	896	24	72	0	4	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.84	1.00	1.00	1.00	1.00	1.00	0.72	1.00	0.72	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	0.97	0.03	0.95	0.00	0.05	0.00	0.00	0.00
Final Sat.:	1604	1900	0	0	1843	49	1298	0	72	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.38	0.00	0.00	0.49	0.49	0.06	0.00	0.06	0.00	0.00	0.00
Crit Moves:				****			****					
Green/Cycle:	0.78	0.78	0.00	0.00	0.78	0.78	0.15	0.00	0.15	0.00	0.00	0.00
Volume/Cap:	0.00	0.49	0.00	0.00	0.62	0.62	0.37	0.00	0.37	0.00	0.00	0.00
Delay/Veh:	2.8	4.8	0.0	0.0	6.3	6.3	47.0	0.0	47.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	2.8	4.8	0.0	0.0	6.3	6.3	47.0	0.0	47.0	0.0	0.0	0.0
LOS by Move:	A	A	A	A	A	A	D	A	D	A	A	A
HCM2kAvgQ:	0	10	0	0	15	15	3	0	3	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
Year 2025 With Project
Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #19 Project Access (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 0.5 Worst Case Level Of Service: B [14.9]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control (Stop Sign, Uncontrolled), Rights (Include), and Lanes (0-1-0).

Volume Module: Table with 12 columns for volume components. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with 12 columns for gap metrics. Rows include Critical Gp and FollowUpTim.

Capacity Module: Table with 12 columns for capacity metrics. Rows include Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with 12 columns for LOS metrics. Rows include 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 2
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #19 Project Access (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 1.4 Worst Case Level Of Service: D [26.0]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Include		
Lanes:	0	0	1	0	0	1	0	0	1	0	0	1

Volume Module:

Base Vol:	0	0	0	0	0	0	0	840	0	0	932	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	0	0	0	0	840	0	0	932	0
Added Vol:	0	0	4	0	0	106	0	71	52	0	60	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	4	0	0	106	0	911	52	0	992	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	0	4	0	0	112	0	959	55	0	1044	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	4	0	0	112	0	959	55	0	1044	0

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	6.2	xxxxx	xxxx	6.2	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	xxxxx	xxxx	3.3	xxxxx	xxxx	3.3	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	xxxx	xxxx	986	xxxx	xxxx	1044	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	xxxx	xxxx	303	xxxx	xxxx	281	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Move Cap.:	xxxx	xxxx	303	xxxx	xxxx	281	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	xxxx	xxxx	0.01	xxxx	xxxx	0.40	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	0.0	xxxx	xxxx	1.8	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	17.0	xxxxx	xxxx	26.0	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	*	*	C	*	*	D	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	17.0			26.0			xxxxxx			xxxxxx		
ApproachLOS:	C			D			*			*		

 Note: Queue reported is the number of cars per lane.

Year 2025 With Project – Alternative 3

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 Sanderson Avenue (NS) at Ramona Expressway (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.845

Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 50.4

Optimal Cycle: OPTIMIZED Level Of Service: D

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Ovl), Min. Green, Lanes.

Volume Module:

Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module:

Table with columns: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with columns: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 Sanderson Avenue (NS) at Ramona Expressway (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.842
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 52.3
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different volume metrics and 4 columns for North, South, East, and West bounds.

Saturation Flow Module:

Table with 12 columns representing saturation flow metrics and 4 columns for North, South, East, and West bounds.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis metrics and 4 columns for North, South, East, and West bounds.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns for traffic volumes and adjustment factors like Base Vol, Growth Adj, Initial Bse, etc.

Critical Gap Module: Table with 12 columns for critical gap and follow-up time values.

Capacity Module: Table with 12 columns for capacity-related metrics like Cnflct Vol, Potent Cap., Move Cap., etc.

Level Of Service Module: Table with 12 columns for LOS-related metrics like 2Way95thQ, Control Del, LOS by Move, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

Average Delay (sec/veh): 15.1 Worst Case Level Of Service: F[362.8]

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, Lanes.

Volume Module:

Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume.

Critical Gap Module:

Table with columns: Critical Gp, FollowUpTim.

Capacity Module:

Table with columns: Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module:

Table with columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

 Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

Cycle (sec): 85 Critical Vol./Cap.(X): 0.899
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 40.3
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	1	0	0	1	0	0	0	0	1	0	1	0

Volume Module:

Base Vol:	1	693	15	148	252	1	1	1	1	15	1	366
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	1	693	15	148	252	1	1	1	1	15	1	366
Added Vol:	0	0	0	48	0	0	0	0	0	0	0	40
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	1	693	15	196	252	1	1	1	1	15	1	406
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	1	729	16	206	265	1	1	1	1	16	1	427
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	1	729	16	206	265	1	1	1	1	16	1	427
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	1	729	16	206	265	1	1	1	1	16	1	427

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	0.95	1.00	1.00	0.93	0.93	0.93	0.86	0.86	0.85
Lanes:	1.00	0.98	0.02	1.00	0.99	0.01	0.34	0.33	0.33	0.94	0.06	1.00
Final Sat.:	1805	1854	40	1805	1891	8	587	587	587	1530	102	1615

Capacity Analysis Module:

Vol/Sat:	0.00	0.39	0.39	0.11	0.14	0.14	0.00	0.00	0.00	0.01	0.01	0.26
Crit Moves:	****			****						****		
Green/Cycle:	0.20	0.44	0.44	0.13	0.36	0.36	0.29	0.29	0.29	0.29	0.29	0.29
Volume/Cap:	0.00	0.90	0.90	0.90	0.39	0.39	0.01	0.01	0.01	0.04	0.04	0.90
Delay/Veh:	27.1	34.9	34.9	70.3	20.4	20.4	21.2	21.2	21.2	21.4	21.4	48.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	27.1	34.9	34.9	70.3	20.4	20.4	21.2	21.2	21.2	21.4	21.4	48.5
LOS by Move:	C	C	C	E	C	C	C	C	C	C	C	D
HCM2kAvgQ:	0	22	22	9	5	5	0	0	0	0	0	15

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #2 State Street/Gilman Springs Road (NS) at Soboba Road (EW)

 Cycle (sec): 90 Critical Vol./Cap.(X): 0.920
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 43.1
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	1	0	0	1	0	0	0	0	1	0	1	0

Volume Module:

Base Vol:	1	466	14	380	663	1	1	1	2	21	2	241
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	1	466	14	380	663	1	1	1	2	21	2	241
Added Vol:	0	0	0	123	0	0	0	0	0	0	0	122
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	1	466	14	503	663	1	1	1	2	21	2	363
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	1	491	15	529	698	1	1	1	2	22	2	382
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	1	491	15	529	698	1	1	1	2	22	2	382
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	1	491	15	529	698	1	1	1	2	22	2	382

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	0.95	1.00	1.00	0.91	0.91	0.91	0.84	0.84	0.85
Lanes:	1.00	0.97	0.03	1.00	0.99	0.01	0.25	0.25	0.50	0.91	0.09	1.00
Final Sat.:	1805	1837	55	1805	1897	3	432	432	864	1449	138	1615

Capacity Analysis Module:

Vol/Sat:	0.00	0.27	0.27	0.29	0.37	0.37	0.00	0.00	0.00	0.02	0.02	0.24
Crit Moves:	****			****						****		
Green/Cycle:	0.14	0.29	0.29	0.32	0.47	0.47	0.26	0.26	0.26	0.26	0.26	0.26
Volume/Cap:	0.00	0.92	0.92	0.92	0.79	0.79	0.01	0.01	0.01	0.06	0.06	0.92
Delay/Veh:	33.2	51.7	51.7	49.6	24.8	24.8	24.9	24.9	24.9	25.3	25.3	57.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	33.2	51.7	51.7	49.6	24.8	24.8	24.9	24.9	24.9	25.3	25.3	57.9
LOS by Move:	C	D	D	D	C	C	C	C	C	C	C	E
HCM2kAvgQ:	0	18	18	18	18	18	0	0	0	1	1	14

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 State Street (NS) at Ramona Expressway (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.894
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 56.3
Optimal Cycle: OPTIMIZED Level Of Service: E

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different volume metrics and 12 rows of data.

Saturation Flow Module:

Table with 12 columns representing saturation flow metrics and 4 rows of data.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis metrics and 10 rows of data.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 State Street (NS) at Ramona Expressway (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.122
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 81.1
Optimal Cycle: OPTIMIZED Level Of Service: F

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, Min. Green, Lanes.

Table with columns: Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Table with columns: Saturation Flow Module, Sat/Lane, Adjustment, Lanes, Final Sat.

Table with columns: Capacity Analysis Module, Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 State Street (NS) at Ramona Expressway (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.676
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 42.5
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	T	R	L	T	R	L	T	R	L	T	R			
Control:	Protected			Protected			Protected			Protected					
Rights:	Include			Include			Ovl			Include					
Min. Green:	10	28	28	10	28	28	10	24	24	10	24	24			
Lanes:	1	0	2	0	1	1	0	2	0	1	1	0	2	0	1

Volume Module:

Base Vol:	379	683	101	109	453	56	159	279	422	252	502	307
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	379	683	101	109	453	56	159	279	422	252	502	307
Added Vol:	0	0	0	0	0	0	0	16	0	0	13	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	379	683	101	109	453	56	159	295	422	252	515	307
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	399	719	106	115	477	59	167	311	444	265	542	323
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	399	719	106	115	477	59	167	311	444	265	542	323
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	399	719	106	115	477	59	167	311	444	265	542	323

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	1805	3610	1615	1805	3610	1615	1805	3610	1615	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.22	0.20	0.07	0.06	0.13	0.04	0.09	0.09	0.28	0.15	0.15	0.20
Crit Moves:	****			****			****			****		
Green/Cycle:	0.26	0.36	0.36	0.13	0.23	0.23	0.12	0.20	0.46	0.17	0.25	0.25
Volume/Cap:	0.85	0.55	0.18	0.49	0.57	0.16	0.78	0.43	0.60	0.85	0.59	0.78
Delay/Veh:	55.8	30.8	26.2	50.1	41.5	36.8	68.7	42.4	25.5	67.3	40.2	51.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	55.8	30.8	26.2	50.1	41.5	36.8	68.7	42.4	25.5	67.3	40.2	51.2
LOS by Move:	E	C	C	D	D	D	E	D	C	E	D	D
HCM2kAvgQ:	17	11	3	4	9	2	8	6	12	12	10	13

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 State Street (NS) at Ramona Expressway (EW)

Cycle (sec): 115 Critical Vol./Cap.(X): 0.946
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 52.3
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	T	R	L	T	R	L	T	R	L	T	R			
Control:	Protected			Protected			Protected			Protected					
Rights:	Include			Include			Ovl			Include					
Min. Green:	10	28	28	10	28	28	10	24	24	10	24	24			
Lanes:	1	0	2	0	1	1	0	2	0	1	1	0	2	0	1

Volume Module:

Base Vol:	422	750	112	288	744	71	125	699	574	232	388	154
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	422	750	112	288	744	71	125	699	574	232	388	154
Added Vol:	0	0	0	0	0	0	0	41	0	0	41	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	422	750	112	288	744	71	125	740	574	232	429	154
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	444	789	118	303	783	75	132	779	604	244	452	162
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	444	789	118	303	783	75	132	779	604	244	452	162
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	444	789	118	303	783	75	132	779	604	244	452	162

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85	0.95	0.95	0.85
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	1805	3610	1615	1805	3610	1615	1805	3610	1615	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.25	0.22	0.07	0.17	0.22	0.05	0.07	0.22	0.37	0.14	0.13	0.10
Crit Moves:	****			****			****			****		
Green/Cycle:	0.25	0.29	0.29	0.20	0.24	0.24	0.11	0.22	0.48	0.14	0.26	0.26
Volume/Cap:	0.97	0.74	0.25	0.83	0.89	0.19	0.68	0.97	0.78	0.97	0.49	0.39
Delay/Veh:	75.9	39.5	31.1	58.1	53.3	34.7	59.1	68.1	30.3	96.6	36.8	36.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	75.9	39.5	31.1	58.1	53.3	34.7	59.1	68.1	30.3	96.6	36.8	36.0
LOS by Move:	E	D	C	E	D	C	E	E	C	F	D	D
HCM2kAvgQ:	21	15	3	13	17	2	6	19	19	13	7	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.836
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 30.7
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with 4 main columns: North Bound, South Bound, East Bound, West Bound. Sub-columns: L, T, R. Rows: Approach, Movement, Control, Rights, Min. Green, Lanes.

Volume Module: Table with 12 columns for different volume types (Base Vol, Growth Adj, etc.) and 12 rows for different metrics.

Saturation Flow Module: Table with 12 columns for different saturation flow metrics and 4 rows for different metrics (Sat/Lane, Adjustment, Lanes, Final Sat.).

Capacity Analysis Module: Table with 12 columns for different capacity analysis metrics and 12 rows for different metrics (Vol/Sat, Crit Moves, Green/Cycle, etc.).

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.058
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 65.7
Optimal Cycle: OPTIMIZED Level Of Service: E

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns for traffic volume metrics across four directions.

Saturation Flow Module: Table with 12 columns for saturation flow and adjustment factors.

Capacity Analysis Module: Table with 12 columns for capacity analysis metrics like Vol/Sat, Crit Moves, and Delay/Veh.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.799
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 29.7
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	28	28	28	28	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	1	0	1	1	0	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	103	698	118	130	537	239	220	627	120	73	729	97
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	103	698	118	130	537	239	220	627	120	73	729	97
Added Vol:	0	0	8	0	0	0	0	40	0	7	33	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	103	698	126	130	537	239	220	667	120	80	762	97
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	108	735	133	137	565	252	232	702	126	84	802	102
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	108	735	133	137	565	252	232	702	126	84	802	102
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	108	735	133	137	565	252	232	702	126	84	802	102

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.17	0.93	0.93	0.22	1.00	0.85	0.95	0.93	0.93	0.95	0.95	0.85
Lanes:	1.00	1.69	0.31	1.00	1.00	1.00	1.00	1.70	0.30	1.00	2.00	1.00
Final Sat.:	317	2988	539	410	1900	1615	1805	2989	538	1805	3610	1615

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.34	0.25	0.25	0.33	0.30	0.16	0.13	0.23	0.23	0.05	0.22	0.06
Crit Moves:	****			****			****			****		
Green/Cycle:	0.40	0.40	0.40	0.40	0.40	0.40	0.15	0.34	0.34	0.12	0.31	0.31
Volume/Cap:	0.85	0.61	0.61	0.83	0.74	0.39	0.85	0.69	0.69	0.38	0.71	0.20
Delay/Veh:	62.0	22.0	22.0	51.4	26.5	19.3	58.1	27.3	27.3	37.5	29.7	23.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	62.0	22.0	22.0	51.4	26.5	19.3	58.1	27.3	27.3	37.5	29.7	23.0
LOS by Move:	E	C	C	D	C	B	E	C	C	D	C	C
HCM2kAvgQ:	5	11	11	6	15	5	9	11	11	3	12	2

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 State Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.966
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 45.3
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	28	28	28	28	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	1	0	2	1	0	2

Volume Module:

Base Vol:	138	581	125	184	712	392	195	1075	172	144	1039	126
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	138	581	125	184	712	392	195	1075	172	144	1039	126
Added Vol:	0	0	20	0	0	0	0	102	0	20	101	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	138	581	145	184	712	392	195	1177	172	164	1140	126
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	145	612	153	194	749	413	205	1239	181	173	1200	133
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	145	612	153	194	749	413	205	1239	181	173	1200	133
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	145	612	153	194	749	413	205	1239	181	173	1200	133

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.18	0.92	0.92	0.32	1.00	0.85	0.95	0.89	0.89	0.95	0.95	0.85
Lanes:	1.00	1.60	0.40	1.00	1.00	1.00	1.00	2.62	0.38	1.00	2.00	1.00
Final Sat.:	343	2802	699	606	1900	1615	1805	4440	649	1805	3610	1615

Capacity Analysis Module:

Vol/Sat:	0.42	0.22	0.22	0.32	0.39	0.26	0.11	0.28	0.28	0.10	0.33	0.08
Crit Moves:	****						****			****		
Green/Cycle:	0.44	0.44	0.44	0.44	0.44	0.44	0.12	0.34	0.34	0.12	0.34	0.34
Volume/Cap:	0.97	0.50	0.50	0.73	0.90	0.58	0.97	0.81	0.81	0.81	0.97	0.24
Delay/Veh:	95.7	24.5	24.5	37.6	44.1	26.7	104.5	38.8	38.8	72.1	56.6	28.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	95.7	24.5	24.5	37.6	44.1	26.7	104.5	38.8	38.8	72.1	56.6	28.3
LOS by Move:	F	C	C	D	D	C	F	D	D	E	E	C
HCM2kAvgQ:	9	11	11	7	29	12	11	19	19	8	28	3

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 1.166
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 91.5
Optimal Cycle: OPTIMIZED Level Of Service: F

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 sub-columns for movements (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 13 columns for different volume metrics and 4 columns for approaches. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module table with 13 columns for saturation flow metrics and 4 columns for approaches. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 13 columns for capacity analysis metrics and 4 columns for approaches. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 2.026
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 316.5
Optimal Cycle: OPTIMIZED Level Of Service: F

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns representing different traffic movements and 11 rows of volume-related metrics like Base Vol, Growth Adj, etc.

Saturation Flow Module: Table with 12 columns and 4 rows showing saturation flow rates and adjustments.

Capacity Analysis Module: Table with 12 columns and 10 rows showing capacity analysis metrics like Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.649
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 32.6
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Ovl			Include		
Min. Green:	18	18	18	18	18	18	10	21	21	10	21	21
Lanes:	1	0	1	0	1	0	1	0	1	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	337	375	84	30	541	29	18	231	348	49	168	22
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	337	375	84	30	541	29	18	231	348	49	168	22
Added Vol:	0	0	64	16	0	0	0	0	0	53	0	13
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	337	375	148	46	541	29	18	231	348	102	168	35
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	355	395	156	48	569	31	19	243	366	107	177	37
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	355	395	156	48	569	31	19	243	366	107	177	37
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	355	395	156	48	569	31	19	243	366	107	177	37

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	0.85	0.95	0.94	0.94	0.95	1.00	0.85	0.92	0.97	0.97
Lanes:	1.00	1.00	1.00	1.00	1.90	0.10	1.00	1.00	1.00	2.00	0.83	0.17
Final Sat.:	1805	1900	1615	1805	3399	182	1805	1900	1615	3502	1532	319

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.20	0.21	0.10	0.03	0.17	0.17	0.01	0.13	0.23	0.03	0.12	0.12
Crit Moves:	****			****			****			****		
Green/Cycle:	0.26	0.26	0.26	0.21	0.21	0.21	0.11	0.23	0.50	0.11	0.23	0.23
Volume/Cap:	0.74	0.79	0.36	0.13	0.79	0.79	0.09	0.55	0.46	0.28	0.49	0.49
Delay/Veh:	36.5	38.7	27.5	28.8	38.8	38.8	36.1	31.8	15.1	37.1	30.8	30.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	36.5	38.7	27.5	28.8	38.8	38.8	36.1	31.8	15.1	37.1	30.8	30.8
LOS by Move:	D	D	C	C	D	D	D	C	B	D	C	C
HCM2kAvgQ:	11	12	4	1	10	10	1	7	7	2	6	6

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #5 San Jacinto Street (NS) at Ramona Boulevard/Main Street (EW)

 Cycle (sec): 100 Critical Vol./Cap.(X): 0.822
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 45.2
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Ovl			Include		
Min. Green:	18	18	18	18	18	18	10	21	21	10	21	21
Lanes:	1	0	1	0	1	0	1	0	1	0	1	0

Volume Module:

Base Vol:	452	436	171	81	522	49	50	356	452	179	304	54
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	452	436	171	81	522	49	50	356	452	179	304	54
Added Vol:	0	0	164	41	0	0	0	0	0	162	0	41
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	452	436	335	122	522	49	50	356	452	341	304	95
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	476	459	353	128	549	52	53	375	476	359	320	100
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	476	459	353	128	549	52	53	375	476	359	320	100
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	476	459	353	128	549	52	53	375	476	359	320	100

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	0.85	0.95	0.94	0.94	0.95	1.00	0.85	0.92	0.96	0.96
Lanes:	1.00	1.00	1.00	1.00	1.83	0.17	1.00	1.00	1.00	2.00	0.76	0.24
Final Sat.:	1805	1900	1615	1805	3257	306	1805	1900	1615	3502	1396	436

Capacity Analysis Module:

Vol/Sat:	0.26	0.24	0.22	0.07	0.17	0.17	0.03	0.20	0.29	0.10	0.23	0.23
Crit Moves:	****			****			****			****		
Green/Cycle:	0.29	0.29	0.29	0.19	0.19	0.19	0.10	0.24	0.53	0.12	0.26	0.26
Volume/Cap:	0.89	0.82	0.74	0.38	0.89	0.89	0.29	0.82	0.55	0.88	0.89	0.89
Delay/Veh:	51.1	42.1	37.9	36.1	54.0	54.0	42.6	47.6	16.1	62.2	55.0	55.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	51.1	42.1	37.9	36.1	54.0	54.0	42.6	47.6	16.1	62.2	55.0	55.0
LOS by Move:	D	D	D	D	D	D	D	D	B	E	E	E
HCM2kAvgQ:	18	15	11	4	13	13	2	13	10	9	16	16

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #6 San Jacinto Street (NS) at Esplanade Avenue (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.613
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 30.5
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green, Lanes.

Table with columns: Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Table with columns: Saturation Flow Module, Sat/Lane, Adjustment, Lanes, Final Sat.

Table with columns: Capacity Analysis Module, Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #6 San Jacinto Street (NS) at Esplanade Avenue (EW)

Cycle (sec): 110 Critical Vol./Cap.(X): 0.843
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 46.8
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green (10, 28, 28), Lanes (1 0 2 0 1).

Volume Module:
Base Vol: 229 686 158 232 632 134 181 647 225 181 535 106
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 229 686 158 232 632 134 181 647 225 181 535 106
Added Vol: 0 143 0 0 142 20 20 0 0 0 0 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 229 829 158 232 774 154 201 647 225 181 535 106
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95
PHF Volume: 241 873 166 244 815 162 212 681 237 191 563 112
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 241 873 166 244 815 162 212 681 237 191 563 112
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 241 873 166 244 815 162 212 681 237 191 563 112

Saturation Flow Module:
Sat/Lane: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900
Adjustment: 0.95 0.95 0.85 0.95 0.93 0.93 0.95 0.95 0.85 0.95 0.93 0.93
Lanes: 1.00 2.00 1.00 1.00 1.67 0.33 1.00 2.00 1.00 1.00 1.67 0.33
Final Sat.: 1805 3610 1615 1805 2936 584 1805 3610 1615 1805 2938 582

Capacity Analysis Module:
Vol/Sat: 0.13 0.24 0.10 0.14 0.28 0.28 0.12 0.19 0.15 0.11 0.19 0.19
Crit Moves: ****
Green/Cycle: 0.15 0.30 0.30 0.16 0.32 0.32 0.13 0.27 0.27 0.11 0.25 0.25
Volume/Cap: 0.88 0.79 0.34 0.84 0.88 0.88 0.88 0.69 0.54 0.93 0.75 0.75
Delay/Veh: 72.0 39.1 30.0 63.0 44.1 44.1 75.8 37.8 35.3 91.5 41.5 41.5
User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
AdjDel/Veh: 72.0 39.1 30.0 63.0 44.1 44.1 75.8 37.8 35.3 91.5 41.5 41.5
LOS by Move: E D C E D D E D D F D D
HCM2kAvgQ: 11 16 4 11 20 20 10 12 7 10 13 13

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #7 San Jacinto Street (NS) at Menlo Avenue (EW)

Cycle (sec): 65 Critical Vol./Cap.(X): 0.638
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 21.7
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for different traffic movements and 12 rows for various volume and adjustment factors.

Saturation Flow Module table with 12 columns for different traffic movements and 4 rows for saturation flow parameters.

Capacity Analysis Module table with 12 columns for different traffic movements and 12 rows for capacity and delay analysis metrics.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #7 San Jacinto Street (NS) at Menlo Avenue (EW)

Cycle (sec): 95 Critical Vol./Cap.(X): 0.817
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 33.6
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Permitted), Rights (Include), and Lane counts (1, 0, 1, 1, 0).

Volume Module table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume across 12 lanes.

Saturation Flow Module table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across 12 lanes.

Capacity Analysis Module table with columns for Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ across 12 lanes.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #8 San Jacinto Street (NS) at Devonshire Avenue (EW)

Cycle (sec): 70 Critical Vol./Cap.(X): 0.717
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 23.4
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different volume and adjustment factors like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 12 columns for saturation flow factors like Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis factors like Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #8 San Jacinto Street (NS) at Devonshire Avenue (EW)

Cycle (sec): 85 Critical Vol./Cap.(X): 0.817
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 28.7
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different volume metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 12 columns representing saturation flow metrics like Sat/Lane, Adjustment, Lanes, etc.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis metrics like Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.823
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 79.8
Optimal Cycle: OPTIMIZED Level Of Service: E

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Split Phase, Protected), Rights (Include), Min. Green, Lanes.

Volume Module:

Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module:

Table with columns: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with columns: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.082
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 138.8
Optimal Cycle: OPTIMIZED Level Of Service: F

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns representing different traffic volumes and adjustments like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module: Table with 12 columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with 12 columns for Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

 Cycle (sec): 105 Critical Vol./Cap.(X): 0.689
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 34.1
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	10	28	28	10	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	0	1	1	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	81	259	77	163	241	149	200	702	43	97	1114	202
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	81	259	77	163	241	149	200	702	43	97	1114	202
Added Vol:	0	8	0	0	7	40	48	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	81	267	77	163	248	189	248	702	43	97	1114	202
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	85	281	81	172	261	199	261	739	45	102	1173	213
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	85	281	81	172	261	199	261	739	45	102	1173	213
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	85	281	81	172	261	199	261	739	45	102	1173	213

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.92	0.92	0.92	1.00	0.85	0.92	0.94	0.94	0.95	0.95	0.85
Lanes:	1.00	1.55	0.45	2.00	1.00	1.00	2.00	1.88	0.12	1.00	2.00	1.00
Final Sat.:	1805	2707	781	3502	1900	1615	3502	3371	206	1805	3610	1615

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.05	0.10	0.10	0.05	0.14	0.12	0.07	0.22	0.22	0.06	0.32	0.13
Crit Moves:	****			****			****			****		
Green/Cycle:	0.10	0.27	0.27	0.10	0.27	0.36	0.10	0.36	0.36	0.13	0.39	0.39
Volume/Cap:	0.50	0.39	0.39	0.51	0.52	0.34	0.78	0.61	0.61	0.44	0.83	0.34
Delay/Veh:	47.4	31.8	31.8	46.6	33.6	24.7	57.9	28.6	28.6	43.7	33.3	22.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	47.4	31.8	31.8	46.6	33.6	24.7	57.9	28.6	28.6	43.7	33.3	22.8
LOS by Move:	D	C	C	D	C	C	E	C	C	D	C	C
HCM2kAvgQ:	3	5	5	3	8	5	6	11	11	4	20	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #9 San Jacinto Street (NS) at Florida Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.889
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 54.5
 Optimal Cycle: OPTIMIZED Level of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	10	28	28	10	28	28	10	28	28	10	28	28
Lanes:	1	0	1	1	0	1	1	0	1	1	0	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	79	349	144	457	338	208	223	1167	108	172	1193	75
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	79	349	144	457	338	208	223	1167	108	172	1193	75
Added Vol:	0	20	0	0	20	122	123	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	79	369	144	457	358	330	346	1167	108	172	1193	75
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	83	388	152	481	377	347	364	1228	114	181	1256	79
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	83	388	152	481	377	347	364	1228	114	181	1256	79
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	83	388	152	481	377	347	364	1228	114	181	1256	79

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	0.91	0.92	1.00	0.85	0.92	0.94	0.94	0.95	0.95	0.85
Lanes:	1.00	1.44	0.56	2.00	1.00	1.00	2.00	1.83	0.17	1.00	2.00	1.00
Final Sat.:	1805	2488	971	3502	1900	1615	3502	3261	302	1805	3610	1615

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.05	0.16	0.16	0.14	0.20	0.22	0.10	0.38	0.38	0.10	0.35	0.05
Crit Moves:	****			****			****			****		
Green/Cycle:	0.10	0.23	0.23	0.14	0.28	0.39	0.11	0.39	0.39	0.10	0.38	0.38
Volume/Cap:	0.47	0.67	0.67	0.97	0.72	0.55	0.92	0.97	0.97	0.97	0.92	0.13
Delay/Veh:	53.0	44.0	44.0	83.9	44.0	29.6	78.8	53.5	53.5	110.5	45.7	24.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	53.0	44.0	44.0	83.9	44.0	29.6	78.8	53.5	53.5	110.5	45.7	24.5
LOS by Move:	D	D	D	F	D	C	E	D	D	F	D	C
HCM2kAvgQ:	3	10	10	13	14	10	10	31	31	10	27	2

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

Cycle (sec): 105 Critical Vol./Cap.(X): 0.681
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 37.6
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns for different volume metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module: Table with 12 columns for saturation flow metrics like Sat/Lane, Adjustment, Lanes, etc.

Capacity Analysis Module: Table with 12 columns for capacity analysis metrics like Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.085
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 78.7
Optimal Cycle: OPTIMIZED Level Of Service: E

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns representing different volume and adjustment factors like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module: Table with 12 columns representing saturation flow factors like Sat/Lane, Adjustment, Lanes, Final Sat., etc.

Capacity Analysis Module: Table with 12 columns representing capacity analysis factors like Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

 Cycle (sec): 100 Critical Vol./Cap.(X): 0.607
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 32.1
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	24	24	10	24	24	10	28	28	10	28	28
Lanes:	1	0	2	0	1	1	0	1	1	0	2	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	64	848	367	98	436	37	42	238	95	225	171	40
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	64	848	367	98	436	37	42	238	95	225	171	40
Added Vol:	0	0	16	16	0	0	0	80	0	13	67	13
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	64	848	383	114	436	37	42	318	95	238	238	53
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	67	893	403	120	459	39	44	335	100	251	251	56
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	67	893	403	120	459	39	44	335	100	251	251	56
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	67	893	403	120	459	39	44	335	100	251	251	56

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.94	0.94	0.95	0.92	0.92	0.92	0.92	0.92
Lanes:	1.00	2.00	1.00	1.00	1.84	0.16	1.00	1.54	0.46	2.00	1.64	0.36
Final Sat.:	1805	3610	1615	1805	3288	279	1805	2685	802	3502	2873	640

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.04	0.25	0.25	0.07	0.14	0.14	0.02	0.12	0.12	0.07	0.09	0.09
Crit Moves:	****			****			****			****		
Green/Cycle:	0.13	0.36	0.36	0.10	0.32	0.32	0.10	0.28	0.28	0.10	0.28	0.28
Volume/Cap:	0.28	0.69	0.70	0.66	0.43	0.43	0.24	0.45	0.45	0.69	0.31	0.31
Delay/Veh:	39.5	29.1	31.4	52.4	26.9	26.9	42.1	29.9	29.9	49.0	28.4	28.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	39.5	29.1	31.4	52.4	26.9	26.9	42.1	29.9	29.9	49.0	28.4	28.4
LOS by Move:	D	C	C	D	C	C	D	C	C	D	C	C
HCM2kAvgQ:	2	13	12	5	6	6	1	6	6	5	4	4

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #10 Ramona Expressway (NS) at Main Street/Lake Park Drive (EW)

 Cycle (sec): 115 Critical Vol./Cap.(X): 0.895
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 49.5
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	24	24	10	24	24	10	28	28	10	28	28
Lanes:	1	0	2	0	1	1	0	1	1	0	2	0

Volume Module:

Base Vol:	86	406	375	182	702	23	23	344	86	560	378	157
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	86	406	375	182	702	23	23	344	86	560	378	157
Added Vol:	0	0	41	41	0	0	0	204	0	41	203	41
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	86	406	416	223	702	23	23	548	86	601	581	198
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	91	427	438	235	739	24	24	577	91	633	612	208
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	91	427	438	235	739	24	24	577	91	633	612	208
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	91	427	438	235	739	24	24	577	91	633	612	208

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	0.95	0.95	0.95	0.93	0.93	0.92	0.91	0.91
Lanes:	1.00	2.00	1.00	1.00	1.94	0.06	1.00	1.73	0.27	2.00	1.49	0.51
Final Sat.:	1805	3610	1615	1805	3478	114	1805	3058	480	3502	2590	883

Capacity Analysis Module:

Vol/Sat:	0.05	0.12	0.27	0.13	0.21	0.21	0.01	0.19	0.19	0.18	0.24	0.24
Crit Moves:	****			****			****			****		
Green/Cycle:	0.12	0.29	0.29	0.14	0.30	0.30	0.11	0.24	0.24	0.19	0.32	0.32
Volume/Cap:	0.41	0.41	0.94	0.94	0.70	0.70	0.12	0.77	0.77	0.94	0.74	0.74
Delay/Veh:	47.7	33.4	67.8	90.5	37.7	37.7	46.0	45.0	45.0	67.5	37.3	37.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	47.7	33.4	67.8	90.5	37.7	37.7	46.0	45.0	45.0	67.5	37.3	37.3
LOS by Move:	D	C	E	F	D	D	D	D	D	E	D	D
HCM2kAvgQ:	3	6	20	12	14	14	1	13	13	16	15	15

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

Average Delay (sec/veh): 17.4 Worst Case Level Of Service: F[194.0]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns representing different traffic movements and 10 rows of volume-related metrics like Base Vol, Growth Adj, etc.

Critical Gap Module: Table with 12 columns and 2 rows showing critical gap and follow-up time for various movements.

Capacity Module: Table with 12 columns and 4 rows showing capacity metrics like Cnflct Vol, Potent Cap., etc.

Level Of Service Module: Table with 12 columns and 10 rows showing LOS metrics like 2Way95thQ, Control Del, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

Average Delay (sec/veh): 87.0 Worst Case Level Of Service: F[499.3]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module table with 12 columns representing different traffic movements and 10 rows of volume-related metrics.

Critical Gap Module table with 12 columns and 2 rows of gap and follow-up time data.

Capacity Module table with 12 columns and 4 rows of capacity and volume/capacity data.

Level Of Service Module table with 12 columns and 10 rows of LOS-related metrics.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.441
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 10.2
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	18	18	18	18	28	28	28	28	28	28
Lanes:	1	0	2	0	1	0	1	0	0	0	1	0

Volume Module:

Base Vol:	136	1143	8	34	614	106	71	2	113	3	5	1
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	136	1143	8	34	614	106	71	2	113	3	5	1
Added Vol:	0	16	0	0	13	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	136	1159	8	34	627	106	71	2	113	3	5	1
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	143	1220	8	36	660	112	75	2	119	3	5	1
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	143	1220	8	36	660	112	75	2	119	3	5	1
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	143	1220	8	36	660	112	75	2	119	3	5	1

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.37	0.95	0.85	0.19	0.95	0.85	0.73	0.73	0.85	0.94	0.94	0.94
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	0.97	0.03	1.00	0.33	0.56	0.11
Final Sat.:	707	3610	1615	353	3610	1615	1340	38	1615	594	990	198

Capacity Analysis Module:

Vol/Sat:	0.20	0.34	0.01	0.10	0.18	0.07	0.06	0.06	0.07	0.01	0.01	0.01
Crit Moves:	****									****		
Green/Cycle:	0.70	0.70	0.70	0.70	0.70	0.70	0.23	0.23	0.23	0.23	0.23	0.23
Volume/Cap:	0.29	0.48	0.01	0.14	0.26	0.10	0.24	0.24	0.32	0.02	0.02	0.02
Delay/Veh:	7.1	8.3	5.4	6.3	6.7	5.8	37.7	37.7	38.6	35.5	35.5	35.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	7.1	8.3	5.4	6.3	6.7	5.8	37.7	37.7	38.6	35.5	35.5	35.5
LOS by Move:	A	A	A	A	A	A	D	D	D	D	D	D
HCM2kAvgQ:	2	11	0	1	5	1	2	2	4	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #11 Ramona Expressway (NS) at 7th Street (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.694
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 14.4
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	18	18	18	18	28	28	28	28	28	28
Lanes:	1	0	2	0	1	0	1	0	0	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	109	649	1	61	1180	69	109	1	331	4	2	6
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	109	649	1	61	1180	69	109	1	331	4	2	6
Added Vol:	0	41	0	0	41	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	109	690	1	61	1221	69	109	1	331	4	2	6
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	115	726	1	64	1285	73	115	1	348	4	2	6
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	115	726	1	64	1285	73	115	1	348	4	2	6
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	115	726	1	64	1285	73	115	1	348	4	2	6

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.15	0.95	0.85	0.33	0.95	0.85	0.71	0.71	0.85	0.88	0.88	0.88
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	0.99	0.01	1.00	0.33	0.17	0.50
Final Sat.:	276	3610	1615	627	3610	1615	1335	12	1615	560	280	839

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.42	0.20	0.00	0.10	0.36	0.04	0.09	0.09	0.22	0.01	0.01	0.01
Crit Moves:	****			****			****			****		
Green/Cycle:	0.60	0.60	0.60	0.60	0.60	0.60	0.31	0.31	0.31	0.31	0.31	0.31
Volume/Cap:	0.69	0.34	0.00	0.17	0.59	0.07	0.28	0.28	0.69	0.02	0.02	0.02
Delay/Veh:	24.4	9.1	7.2	8.2	11.6	7.6	23.7	23.7	31.4	21.5	21.5	21.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	24.4	9.1	7.2	8.2	11.6	7.6	23.7	23.7	31.4	21.5	21.5	21.5
LOS by Move:	C	A	A	A	B	A	C	C	C	C	C	C
HCM2kAvgQ:	4	5	0	1	12	1	3	3	10	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #12 Mountain Avenue (NS) at Esplanade Avenue (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.863
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 27.6
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 sub-columns for Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 13 columns for different volume types (Base Vol, Growth Adj, etc.) and 4 columns for movement directions (L, T, R).

Saturation Flow Module table with 13 columns for saturation flow values and 4 columns for movement directions.

Capacity Analysis Module table with 13 columns for capacity analysis metrics and 4 columns for movement directions.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #12 Mountain Avenue (NS) at Esplanade Avenue (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.061
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 61.9
Optimal Cycle: OPTIMIZED Level Of Service: E

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module table with 13 columns and 14 rows including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module table with 13 columns and 5 rows including Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 13 columns and 11 rows including Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #12 Mountain Avenue (NS) at Esplanade Avenue (EW)

 Cycle (sec): 75 Critical Vol./Cap.(X): 0.863
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 24.9
 Optimal Cycle: OPTIMIZED Level of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	24	0	0	24	24	21	0	21	0	0	0
Lanes:	1	0	1	0	0	2	0	0	1	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	232	890	0	0	464	139	309	0	122	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	232	890	0	0	464	139	309	0	122	0	0	0
Added Vol:	0	16	0	0	13	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	232	906	0	0	477	139	309	0	122	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	244	954	0	0	502	146	325	0	128	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	244	954	0	0	502	146	325	0	128	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	244	954	0	0	502	146	325	0	128	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	0.95	0.85	0.77	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	3610	1615	1461	0	1615	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.14	0.50	0.00	0.00	0.14	0.09	0.22	0.00	0.08	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.24	0.56	0.00	0.00	0.32	0.32	0.28	0.00	0.28	0.00	0.00	0.00
Volume/Cap:	0.56	0.90	0.00	0.00	0.43	0.28	0.80	0.00	0.28	0.00	0.00	0.00
Delay/Veh:	26.8	24.6	0.0	0.0	20.4	19.4	35.3	0.0	21.5	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	26.8	24.6	0.0	0.0	20.4	19.4	35.3	0.0	21.5	0.0	0.0	0.0
LOS by Move:	C	C	A	A	C	B	D	A	C	A	A	A
HCM2kAvgQ:	6	24	0	0	5	3	9	0	2	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #12 Mountain Avenue (NS) at Esplanade Avenue (EW)

Cycle (sec): 70 Critical Vol./Cap.(X): 0.822
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 25.2
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	10	24	0	0	24	24	21	0	21	0	0	0
Lanes:	1	0	1	0	0	2	0	0	1	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	244	462	0	0	1003	401	249	0	360	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	244	462	0	0	1003	401	249	0	360	0	0	0
Added Vol:	0	41	0	0	41	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	244	503	0	0	1044	401	249	0	360	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	257	529	0	0	1099	422	262	0	379	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	257	529	0	0	1099	422	262	0	379	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	257	529	0	0	1099	422	262	0	379	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	0.95	0.85	0.77	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	3610	1615	1461	0	1615	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.14	0.28	0.00	0.00	0.30	0.26	0.18	0.00	0.23	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.17	0.53	0.00	0.00	0.36	0.36	0.30	0.00	0.30	0.00	0.00	0.00
Volume/Cap:	0.84	0.53	0.00	0.00	0.85	0.73	0.60	0.00	0.78	0.00	0.00	0.00
Delay/Veh:	47.3	11.3	0.0	0.0	25.9	23.9	23.2	0.0	30.5	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	47.3	11.3	0.0	0.0	25.9	23.9	23.2	0.0	30.5	0.0	0.0	0.0
LOS by Move:	D	B	A	A	C	C	C	A	C	A	A	A
HCM2kAvgQ:	8	8	0	0	14	9	6	0	10	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

Average Delay (sec/veh): 21.3 Worst Case Level Of Service: F[123.6]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 13 columns for volume components like Base Vol, Growth Adj, Initial Bse, etc.

Critical Gap Module: Table with 13 columns for gap metrics like Critical Gp, FollowUpTim.

Capacity Module: Table with 13 columns for capacity metrics like Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module: Table with 13 columns for LOS metrics like 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

Average Delay (sec/veh): 31.6 Worst Case Level Of Service: F[270.1]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control (Stop Sign, Uncontrolled), Rights (Include), and Lanes (1 0 0 0 1, etc.).

Volume Module: Table with 13 columns for volume components. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume.

Critical Gap Module: Table with 13 columns for gap metrics. Rows include Critical Gp and FollowUpTim.

Capacity Module: Table with 13 columns for capacity metrics. Rows include Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with 13 columns for LOS metrics. Rows include 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

 Cycle (sec): 70 Critical Vol./Cap.(X): 0.605
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 10.8
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	1	0	0	0	0	0	0	0	1	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	194	0	24	0	0	0	0	359	109	75	548	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	194	0	24	0	0	0	0	359	109	75	548	0
Added Vol:	8	0	0	0	0	0	0	7	7	0	8	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	202	0	24	0	0	0	0	366	116	75	556	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	213	0	25	0	0	0	0	385	122	79	585	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	213	0	25	0	0	0	0	385	122	79	585	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	213	0	25	0	0	0	0	385	122	79	585	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.77	1.00	0.85	1.00	1.00	1.00	1.00	0.97	0.97	0.90	0.90	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.76	0.24	0.12	0.88	0.00
Final Sat.:	1461	0	1615	0	0	0	0	1395	442	202	1500	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.15	0.00	0.02	0.00	0.00	0.00	0.00	0.28	0.28	0.39	0.39	0.00
Crit Moves:	****									****		
Green/Cycle:	0.26	0.00	0.26	0.00	0.00	0.00	0.00	0.63	0.63	0.63	0.63	0.00
Volume/Cap:	0.57	0.00	0.06	0.00	0.00	0.00	0.00	0.44	0.44	0.62	0.62	0.00
Delay/Veh:	24.6	0.0	19.7	0.0	0.0	0.0	0.0	6.9	6.9	9.0	9.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	24.6	0.0	19.7	0.0	0.0	0.0	0.0	6.9	6.9	9.0	9.0	0.0
LOS by Move:	C	A	B	A	A	A	A	A	A	A	A	A
HCM2kAvgQ:	5	0	0	0	0	0	0	6	6	9	9	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #13 Soboba Street (NS) at Mountain Avenue (EW)

Cycle (sec): 110 Critical Vol./Cap.(X): 0.802
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 14.5
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	1	0	0	0	0	0	0	0	1	0	1	0

Volume Module:

Base Vol:	138	0	56	0	0	0	0	818	242	44	465	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	138	0	56	0	0	0	0	818	242	44	465	0
Added Vol:	20	0	0	0	0	0	0	20	20	0	20	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	158	0	56	0	0	0	0	838	262	44	485	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	166	0	59	0	0	0	0	882	276	46	511	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	166	0	59	0	0	0	0	882	276	46	511	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	166	0	59	0	0	0	0	882	276	46	511	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.77	1.00	0.85	1.00	1.00	1.00	1.00	0.97	0.97	0.85	0.85	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.76	0.24	0.08	0.92	0.00
Final Sat.:	1461	0	1615	0	0	0	0	1401	438	134	1475	0

Capacity Analysis Module:

Vol/Sat:	0.11	0.00	0.04	0.00	0.00	0.00	0.00	0.63	0.63	0.35	0.35	0.00
Crit Moves:	****			****								
Green/Cycle:	0.16	0.00	0.16	0.00	0.00	0.00	0.00	0.76	0.76	0.76	0.76	0.00
Volume/Cap:	0.70	0.00	0.22	0.00	0.00	0.00	0.00	0.82	0.82	0.45	0.45	0.00
Delay/Veh:	52.0	0.0	40.4	0.0	0.0	0.0	0.0	12.4	12.4	5.0	5.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	52.0	0.0	40.4	0.0	0.0	0.0	0.0	12.4	12.4	5.0	5.0	0.0
LOS by Move:	D	A	D	A	A	A	A	B	B	A	A	A
HCM2kAvgQ:	7	0	2	0	0	0	0	27	27	7	7	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 1.5 Worst Case Level Of Service: F[65.5]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume components like Base Vol, Growth Adj, Initial Bse, etc.

Critical Gap Module: Table with 12 columns for gap metrics like Critical Gp, FollowUpTim.

Capacity Module: Table with 12 columns for capacity metrics like Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module: Table with 12 columns for LOS metrics like 2Way95thQ, Control Del, LOS by Move, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 2.2 Worst Case Level Of Service: F[140.0]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module table with 12 columns and 10 rows including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module table with 12 columns and 2 rows including Critical Gp and FollowUpTim.

Capacity Module table with 12 columns and 4 rows including Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module table with 12 columns and 10 rows including 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: B [14.4]

Approach:	North Bound			South Bound			East Bound			West Bound						
Movement:	L	T	R	L	T	R	L	T	R	L	T	R				
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled						
Rights:	Include			Include			Include			Include						
Lanes:	1	0	0	0	0	0	0	0	2	0	1	1	0	2	0	0

Volume Module:

Base Vol:	0	0	42	0	0	0	0	1089	13	9	622	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	42	0	0	0	0	1089	13	9	622	0
Added Vol:	0	0	0	0	0	0	0	111	0	0	93	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	42	0	0	0	0	1200	13	9	715	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	0	44	0	0	0	0	1263	14	9	753	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	44	0	0	0	0	1263	14	9	753	0

Critical Gap Module:

Critical Gp:	6.8	xxxx	6.9	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	4.1	xxxx	xxxxx
FollowUpTim:	3.5	xxxx	3.3	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	2.2	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	1658	xxxx	632	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	1277	xxxx	xxxxx
Potent Cap.:	90	xxxx	428	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	551	xxxx	xxxxx
Move Cap.:	89	xxxx	428	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	551	xxxx	xxxxx
Volume/Cap:	0.00	xxxx	0.10	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.02	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	0.3	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.1	xxxx	xxxxx
Control Del:	xxxxx	xxxx	14.4	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	11.7	xxxx	xxxxx
LOS by Move:	*	*	B	*	*	*	*	*	*	B	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	14.4			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	B			*			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Average Delay (sec/veh): 0.5 Worst Case Level Of Service: C [21.4]

Approach:	North Bound			South Bound			East Bound			West Bound						
Movement:	L	T	R	L	T	R	L	T	R	L	T	R				
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled						
Rights:	Include			Include			Include			Include						
Lanes:	1	0	0	0	0	0	0	0	2	0	1	1	0	2	0	0

Volume Module:

Base Vol:	9	0	36	0	0	0	0	654	66	22	1049	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	9	0	36	0	0	0	0	654	66	22	1049	0
Added Vol:	0	0	0	0	0	0	0	286	0	0	284	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	9	0	36	0	0	0	0	940	66	22	1333	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	9	0	38	0	0	0	0	989	69	23	1403	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	9	0	38	0	0	0	0	989	69	23	1403	0

Critical Gap Module:

Critical Gp:	6.8	xxxx	6.9	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	4.1	xxxx	xxxxx
FollowUpTim:	3.5	xxxx	3.3	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	2.2	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	1737	xxxx	495	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	1059	xxxx	xxxxx
Potent Cap.:	80	xxxx	526	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	665	xxxx	xxxxx
Move Cap.:	78	xxxx	526	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	665	xxxx	xxxxx
Volume/Cap:	0.12	xxxx	0.07	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.03	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	0.4	xxxx	0.2	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.1	xxxx	xxxxx
Control Del:	57.5	xxxx	12.4	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	10.6	xxxx	xxxxx
LOS by Move:	F	*	B	*	*	*	*	*	*	B	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	21.4			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	C			*			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

 Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.393
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 5.0
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	1	0	0	0	0	0	0	0	2	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	25	0	17	0	0	0	0	1089	13	9	597	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	25	0	17	0	0	0	0	1089	13	9	597	0
Added Vol:	0	0	0	0	0	0	0	111	0	0	93	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	25	0	17	0	0	0	0	1200	13	9	690	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	26	0	18	0	0	0	0	1263	14	9	726	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	26	0	18	0	0	0	0	1263	14	9	726	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	26	0	18	0	0	0	0	1263	14	9	726	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.82	1.00	0.85	1.00	1.00	1.00	1.00	0.95	0.85	0.19	0.95	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	2.00	1.00	1.00	2.00	0.00
Final Sat.:	1560	0	1615	0	0	0	0	3610	1615	369	3610	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.02	0.00	0.01	0.00	0.00	0.00	0.00	0.35	0.01	0.03	0.20	0.00
Crit Moves:	****						****					
Green/Cycle:	0.15	0.00	0.15	0.00	0.00	0.00	0.00	0.78	0.78	0.78	0.78	0.00
Volume/Cap:	0.11	0.00	0.07	0.00	0.00	0.00	0.00	0.45	0.01	0.03	0.26	0.00
Delay/Veh:	44.3	0.0	44.0	0.0	0.0	0.0	0.0	4.4	2.8	2.9	3.6	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	44.3	0.0	44.0	0.0	0.0	0.0	0.0	4.4	2.8	2.9	3.6	0.0
LOS by Move:	D	A	D	A	A	A	A	A	A	A	A	A
HCM2kAvgQ:	1	0	1	0	0	0	0	8	0	0	4	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #14 Soboba Springs Drive (NS) at Lake Park Drive (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 0.425
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 4.9
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	1	0	0	0	0	0	0	0	2	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	22	0	14	0	0	0	0	654	66	22	1027	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	22	0	14	0	0	0	0	654	66	22	1027	0
Added Vol:	0	0	0	0	0	0	0	286	0	0	284	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	22	0	14	0	0	0	0	940	66	22	1311	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	23	0	15	0	0	0	0	989	69	23	1380	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	23	0	15	0	0	0	0	989	69	23	1380	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	23	0	15	0	0	0	0	989	69	23	1380	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.83	1.00	0.85	1.00	1.00	1.00	1.00	0.95	0.85	0.27	0.95	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	2.00	1.00	1.00	2.00	0.00
Final Sat.:	1575	0	1615	0	0	0	0	3610	1615	513	3610	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.01	0.00	0.01	0.00	0.00	0.00	0.00	0.27	0.04	0.05	0.38	0.00
Crit Moves:	****									****		
Green/Cycle:	0.15	0.00	0.15	0.00	0.00	0.00	0.00	0.78	0.78	0.78	0.78	0.00
Volume/Cap:	0.10	0.00	0.06	0.00	0.00	0.00	0.00	0.35	0.05	0.06	0.49	0.00
Delay/Veh:	44.2	0.0	43.9	0.0	0.0	0.0	0.0	4.0	3.0	3.0	4.7	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	44.2	0.0	43.9	0.0	0.0	0.0	0.0	4.0	3.0	3.0	4.7	0.0
LOS by Move:	D	A	D	A	A	A	A	A	A	A	A	A
HCM2kAvgQ:	1	0	0	0	0	0	0	6	1	0	9	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 1.7 Worst Case Level Of Service: D [30.5]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module table with 12 columns and 12 rows including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module table with 12 columns and 2 rows including Critical Gp and FollowUpTim.

Capacity Module table with 12 columns and 4 rows including Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module table with 12 columns and 10 rows including 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
Year 2025 With Project
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 1.2 Worst Case Level Of Service: E[44.5]

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, Lanes.

Volume Module: Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume.

Critical Gap Module: Table with columns: Critical Gp, FollowUpTim.

Capacity Module: Table with columns: Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module: Table with columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 1.3 Worst Case Level Of Service: C [20.7]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	0	1	0	0	1	0	0

Volume Module:

Base Vol:	16	627	4	7	424	13	10	0	63	17	0	9
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	16	627	4	7	424	13	10	0	63	17	0	9
Added Vol:	0	40	0	0	48	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	16	667	4	7	472	13	10	0	63	17	0	9
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	17	702	4	7	497	14	11	0	66	18	0	9
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	17	702	4	7	497	14	11	0	66	18	0	9

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	4.1	xxxx	xxxxxx	7.1	xxxx	6.2	7.1	xxxx	6.2
FollowUpTim:	2.2	xxxx	xxxxxx	2.2	xxxx	xxxxxx	3.5	xxxx	3.3	3.5	xxxx	3.3

Capacity Module:

Cnflct Vol:	511	xxxx	xxxxxx	706	xxxx	xxxxxx	1254	xxxx	248	1001	xxxx	704
Potent Cap.:	1065	xxxx	xxxxxx	901	xxxx	xxxxxx	150	xxxx	795	223	xxxx	440
Move Cap.:	1065	xxxx	xxxxxx	901	xxxx	xxxxxx	144	xxxx	795	201	xxxx	440
Volume/Cap:	0.02	xxxx	xxxxxx	0.01	xxxx	xxxxxx	0.07	xxxx	0.08	0.09	xxxx	0.02

Level Of Service Module:

2Way95thQ:	0.0	xxxx	xxxxxx	0.0	xxxx	xxxxxx	0.2	xxxx	0.3	0.3	xxxx	0.1
Control Del:	8.4	xxxx	xxxxxx	9.0	xxxx	xxxxxx	31.9	xxxx	9.9	24.6	xxxx	13.4
LOS by Move:	A	*	*	A	*	*	D	*	A	C	*	B
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			13.0			20.7		
ApproachLOS:	*			*			B			C		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #15 Soboba Road (NS) at Chabella Drive (EW)

Average Delay (sec/veh): 0.9 Worst Case Level Of Service: C [23.2]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	0	1	0	0	1	0	0

Volume Module:

Base Vol:	52	454	17	9	729	15	5	0	26	6	0	4
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	52	454	17	9	729	15	5	0	26	6	0	4
Added Vol:	0	122	0	0	123	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	52	576	17	9	852	15	5	0	26	6	0	4
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	55	606	18	9	897	16	5	0	27	6	0	4
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	55	606	18	9	897	16	5	0	27	6	0	4

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	4.1	xxxx	xxxxxx	7.1	xxxx	6.2	7.1	xxxx	6.2
FollowUpTim:	2.2	xxxx	xxxxxx	2.2	xxxx	xxxxxx	3.5	xxxx	3.3	3.5	xxxx	3.3

Capacity Module:

Cnflct Vol:	913	xxxx	xxxxxx	624	xxxx	xxxxxx	1643	xxxx	448	1192	xxxx	615
Potent Cap.:	755	xxxx	xxxxxx	967	xxxx	xxxxxx	81	xxxx	615	165	xxxx	495
Move Cap.:	755	xxxx	xxxxxx	967	xxxx	xxxxxx	75	xxxx	615	148	xxxx	495
Volume/Cap:	0.07	xxxx	xxxxxx	0.01	xxxx	xxxxxx	0.07	xxxx	0.04	0.04	xxxx	0.01

Level Of Service Module:

2Way95thQ:	0.2	xxxx	xxxxxx	0.0	xxxx	xxxxxx	0.2	xxxx	0.1	0.1	xxxx	0.0
Control Del:	10.1	xxxx	xxxxxx	8.8	xxxx	xxxxxx	56.6	xxxx	11.1	30.4	xxxx	12.3
LOS by Move:	B	*	*	A	*	*	F	*	B	D	*	B
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			18.5			23.2		
ApproachLOS:	*			*			C			C		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Future Volume Alternative)

 Intersection #16 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.867
 Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 27.6
 Optimal Cycle: 0 Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound			
Movement:	L	T	R	L	T	R	L	T	R	L	T	R	
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign			
Rights:	Include			Include			Include			Include			
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0	
Lanes:	0	0	1	0	0	0	0	1	0	0	1	0	0

Volume Module:

Base Vol:	298	45	1	2	164	179	191	2	461	1	2	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	298	45	1	2	164	179	191	2	461	1	2	2
Added Vol:	13	40	0	0	48	16	13	0	16	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	311	85	1	2	212	195	204	2	477	1	2	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	327	89	1	2	223	205	215	2	502	1	2	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	327	89	1	2	223	205	215	2	502	1	2	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	327	89	1	2	223	205	215	2	502	1	2	2

Saturation Flow Module:

Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.78	0.21	0.01	0.01	0.52	0.47	0.99	0.01	1.00	0.20	0.40	0.40
Final Sat.:	423	115	1	3	297	273	480	5	579	80	159	159

Capacity Analysis Module:

Vol/Sat:	0.77	0.77	0.77	0.75	0.75	0.75	0.45	0.45	0.87	0.01	0.01	0.01
Crit Moves:	****			****			****			****		
Delay/Veh:	27.7	27.7	27.7	24.5	24.5	24.5	15.8	15.8	35.5	10.8	10.8	10.8
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	27.7	27.7	27.7	24.5	24.5	24.5	15.8	15.8	35.5	10.8	10.8	10.8
LOS by Move:	D	D	D	C	C	C	C	C	E	B	B	B
ApproachDel:	27.7			24.5			29.5			10.8		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	27.7			24.5			29.5			10.8		
LOS by Appr:	D			C			D			B		
AllWayAvgQ:	2.7	2.7	2.7	2.4	2.4	2.4	0.8	0.8	4.3	0.0	0.0	0.0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Future Volume Alternative)

Intersection #16 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 1.701
 Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 233.6
 Optimal Cycle: 0 Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0	0	1! 0	0	0	1! 0	0	1	0	0	1	0

Volume Module:

Base Vol:	530	196	1	2	288	400	239	2	599	1	2	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	530	196	1	2	288	400	239	2	599	1	2	2
Added Vol:	41	61	0	0	61	41	41	0	41	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	571	257	1	2	349	441	280	2	640	1	2	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	601	271	1	2	367	464	295	2	674	1	2	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	601	271	1	2	367	464	295	2	674	1	2	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	601	271	1	2	367	464	295	2	674	1	2	2

Saturation Flow Module:

Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.68	0.31	0.01	0.01	0.44	0.55	0.99	0.01	1.00	0.20	0.40	0.40
Final Sat.:	353	159	1	1	242	306	449	3	532	80	160	160

Capacity Analysis Module:

Vol/Sat:	1.70	1.70	1.70	1.52	1.52	1.52	0.66	0.66	1.27	0.01	0.01	0.01
Crit Moves:	****			****			****			****		
Delay/Veh:	341.5	342	341.5	259.5	260	259.5	24.6	24.6	155.7	12.1	12.1	12.1
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	341.5	342	341.5	259.5	260	259.5	24.6	24.6	155.7	12.1	12.1	12.1
LOS by Move:	F	F	F	F	F	F	C	C	F	B	B	B
ApproachDel:	341.5			259.5			115.6			12.1		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	341.5			259.5			115.6			12.1		
LOS by Appr:	F			F			F			B		
AllWayAvgQ:	47.2	47.2	47.2	38.2	38.2	38.2	1.8	1.8	21.6	0.0	0.0	0.0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #16 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 115 Critical Vol./Cap.(X): 0.420
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 32.4
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ovl			Ovl			Include		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	1	0	0	1	0	0	1	1	0	0	0	1

Volume Module:

Base Vol:	298	45	1	2	164	179	191	2	461	1	2	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	298	45	1	2	164	179	191	2	461	1	2	2
Added Vol:	13	40	0	0	48	16	13	0	16	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	311	85	1	2	212	195	204	2	477	1	2	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	327	89	1	2	223	205	215	2	502	1	2	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	327	89	1	2	223	205	215	2	502	1	2	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	327	89	1	2	223	205	215	2	502	1	2	2

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	0.95	1.00	0.85	0.95	0.95	0.85	0.94	0.94	0.94
Lanes:	1.00	0.99	0.01	1.00	1.00	1.00	1.98	0.02	1.00	0.20	0.40	0.40
Final Sat.:	1805	1874	22	1805	1900	1615	3586	35	1615	356	712	712

Capacity Analysis Module:

Vol/Sat:	0.18	0.05	0.05	0.00	0.12	0.13	0.06	0.06	0.31	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.24	0.25	0.25	0.14	0.16	0.46	0.31	0.31	0.55	0.16	0.16	0.16
Volume/Cap:	0.76	0.19	0.19	0.01	0.75	0.27	0.19	0.19	0.57	0.02	0.02	0.02
Delay/Veh:	48.2	33.7	33.7	42.4	56.5	19.1	29.3	29.3	17.9	41.1	41.1	41.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	48.2	33.7	33.7	42.4	56.5	19.1	29.3	29.3	17.9	41.1	41.1	41.1
LOS by Move:	D	C	C	D	E	B	C	C	B	D	D	D
HCM2kAvgQ:	12	2	2	0	9	4	3	3	12	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #16 Soboba Road (NS) at Lake Park Drive (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.705
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 46.6
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ovl			Ovl			Include		
Min. Green:	10	18	18	10	18	18	18	18	18	18	18	18
Lanes:	1	0	0	1	0	0	1	1	0	0	0	1

Volume Module:

Base Vol:	530	196	1	2	288	400	239	2	599	1	2	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	530	196	1	2	288	400	239	2	599	1	2	2
Added Vol:	41	61	0	0	61	41	41	0	41	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	571	257	1	2	349	441	280	2	640	1	2	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	601	271	1	2	367	464	295	2	674	1	2	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	601	271	1	2	367	464	295	2	674	1	2	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	601	271	1	2	367	464	295	2	674	1	2	2

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	0.95	1.00	0.85	0.95	0.95	0.85	0.94	0.94	0.94
Lanes:	1.00	0.99	0.01	1.00	1.00	1.00	1.99	0.01	1.00	0.20	0.40	0.40
Final Sat.:	1805	1891	7	1805	1900	1615	3596	26	1615	356	712	712

Capacity Analysis Module:

Vol/Sat:	0.33	0.14	0.14	0.00	0.19	0.29	0.08	0.08	0.42	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.36	0.36	0.36	0.20	0.21	0.36	0.15	0.15	0.51	0.15	0.15	0.15
Volume/Cap:	0.93	0.39	0.39	0.01	0.93	0.80	0.55	0.55	0.82	0.02	0.02	0.02
Delay/Veh:	56.9	28.7	28.7	38.2	74.7	42.6	48.4	48.4	31.5	43.5	43.5	43.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	56.9	28.7	28.7	38.2	74.7	42.6	48.4	48.4	31.5	43.5	43.5	43.5
LOS by Move:	E	C	C	D	E	D	D	D	C	D	D	D
HCM2kAvgQ:	26	7	7	0	17	17	6	6	23	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #17 Soboba Road (NS) at Project Access (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.440
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 6.6
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	0	0	18	18	18	0	18	0	0	0
Lanes:	1	0	1	0	0	0	1	0	0	0	0	0

Volume Module:

Base Vol:	0	343	0	0	626	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	343	0	0	626	0	0	0	0	0	0	0
Added Vol:	8	0	0	0	0	64	53	0	7	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	8	343	0	0	626	64	53	0	7	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	8	343	0	0	626	64	53	0	7	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	8	343	0	0	626	64	53	0	7	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	8	343	0	0	626	64	53	0	7	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.80	1.00	1.00	1.00	0.99	0.99	0.74	1.00	0.74	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	0.91	0.09	0.88	0.00	0.12	0.00	0.00	0.00
Final Sat.:	1518	1900	0	0	1701	174	1250	0	165	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.01	0.18	0.00	0.00	0.37	0.37	0.04	0.00	0.04	0.00	0.00	0.00
Crit Moves:				****			****					
Green/Cycle:	0.78	0.78	0.00	0.00	0.78	0.78	0.15	0.00	0.15	0.00	0.00	0.00
Volume/Cap:	0.01	0.23	0.00	0.00	0.47	0.47	0.28	0.00	0.28	0.00	0.00	0.00
Delay/Veh:	2.8	3.5	0.0	0.0	4.7	4.7	46.0	0.0	46.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	2.8	3.5	0.0	0.0	4.7	4.7	46.0	0.0	46.0	0.0	0.0	0.0
LOS by Move:	A	A	A	A	A	A	D	A	D	A	A	A
HCM2kAvgQ:	0	3	0	0	9	9	2	0	2	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #17 Soboba Road (NS) at Project Access (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.659
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 9.1
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	18	0	0	18	18	18	0	18	0	0	0
Lanes:	1	0	1	0	0	0	1	0	0	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	726	0	0	888	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	726	0	0	888	0	0	0	0	0	0	0
Added Vol:	20	0	0	0	0	102	101	0	20	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	20	726	0	0	888	102	101	0	20	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	20	726	0	0	888	102	101	0	20	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	20	726	0	0	888	102	101	0	20	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	20	726	0	0	888	102	101	0	20	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.64	1.00	1.00	1.00	0.99	0.99	0.73	1.00	0.73	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	0.90	0.10	0.83	0.00	0.17	0.00	0.00	0.00
Final Sat.:	1207	1900	0	0	1680	193	1163	0	230	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.02	0.38	0.00	0.00	0.53	0.53	0.09	0.00	0.09	0.00	0.00	0.00
Crit Moves:				****			****					
Green/Cycle:	0.78	0.78	0.00	0.00	0.78	0.78	0.15	0.00	0.15	0.00	0.00	0.00
Volume/Cap:	0.02	0.49	0.00	0.00	0.67	0.67	0.58	0.00	0.58	0.00	0.00	0.00
Delay/Veh:	2.9	4.8	0.0	0.0	7.2	7.2	51.5	0.0	51.5	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	2.9	4.8	0.0	0.0	7.2	7.2	51.5	0.0	51.5	0.0	0.0	0.0
LOS by Move:	A	A	A	A	A	A	D	A	D	A	A	A
HCM2kAvgQ:	0	9	0	0	17	17	5	0	5	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #18 Project Access (NS) at Lake Park Drive (EW)

 Cycle (sec): 85 Critical Vol./Cap.(X): 0.487
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 7.7
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	0	0	1	0	0	0	0	0	1	0	1	0

Volume Module:

Base Vol:	0	0	0	0	0	0	0	654	0	0	479	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	0	0	0	0	654	0	0	479	0
Added Vol:	80	0	13	0	0	0	0	16	95	16	13	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	80	0	13	0	0	0	0	670	95	16	492	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	84	0	14	0	0	0	0	705	100	17	518	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	84	0	14	0	0	0	0	705	100	17	518	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	84	0	14	0	0	0	0	705	100	17	518	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.74	1.00	0.74	1.00	1.00	1.00	1.00	1.00	0.85	0.31	1.00	1.00
Lanes:	0.86	0.00	0.14	0.00	0.00	0.00	0.00	1.00	1.00	1.00	1.00	0.00
Final Sat.:	1211	0	197	0	0	0	0	1900	1615	581	1900	0

Capacity Analysis Module:

Vol/Sat:	0.07	0.00	0.07	0.00	0.00	0.00	0.00	0.37	0.06	0.03	0.27	0.00
Crit Moves:	****			****								
Green/Cycle:	0.21	0.00	0.21	0.00	0.00	0.00	0.00	0.69	0.69	0.69	0.69	0.00
Volume/Cap:	0.33	0.00	0.33	0.00	0.00	0.00	0.00	0.53	0.09	0.04	0.39	0.00
Delay/Veh:	29.0	0.0	29.0	0.0	0.0	0.0	0.0	6.8	4.3	4.1	5.7	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	29.0	0.0	29.0	0.0	0.0	0.0	0.0	6.8	4.3	4.1	5.7	0.0
LOS by Move:	C	A	C	A	A	A	A	A	A	A	A	A
HCM2kAvgQ:	2	0	2	0	0	0	0	9	1	0	6	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property - Alternative 3
 Year 2025 With Project
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #18 Project Access (NS) at Lake Park Drive (EW)

Cycle (sec): 70 Critical Vol./Cap.(X): 0.852
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 16.8
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	18	0	18	0	0	0	0	18	18	18	18	0
Lanes:	0	0	1	0	0	0	0	0	1	0	1	0

Volume Module:

Base Vol:	0	0	0	0	0	0	0	840	0	0	932	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	0	0	0	0	840	0	0	932	0
Added Vol:	243	0	41	0	0	0	0	41	245	41	41	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	243	0	41	0	0	0	0	881	245	41	973	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	256	0	43	0	0	0	0	927	258	43	1024	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	256	0	43	0	0	0	0	927	258	43	1024	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	256	0	43	0	0	0	0	927	258	43	1024	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.73	1.00	0.73	1.00	1.00	1.00	1.00	1.00	0.85	0.15	1.00	1.00
Lanes:	0.86	0.00	0.14	0.00	0.00	0.00	0.00	1.00	1.00	1.00	1.00	0.00
Final Sat.:	1188	0	200	0	0	0	0	1900	1615	283	1900	0

Capacity Analysis Module:

Vol/Sat:	0.22	0.00	0.22	0.00	0.00	0.00	0.00	0.49	0.16	0.15	0.54	0.00
Crit Moves:	****									****		
Green/Cycle:	0.26	0.00	0.26	0.00	0.00	0.00	0.00	0.63	0.63	0.63	0.63	0.00
Volume/Cap:	0.84	0.00	0.84	0.00	0.00	0.00	0.00	0.78	0.25	0.24	0.86	0.00
Delay/Veh:	40.4	0.0	40.4	0.0	0.0	0.0	0.0	12.7	5.9	6.4	16.8	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	40.4	0.0	40.4	0.0	0.0	0.0	0.0	12.7	5.9	6.4	16.8	0.0
LOS by Move:	D	A	D	A	A	A	A	B	A	A	B	A
HCM2kAvgQ:	9	0	9	0	0	0	0	16	3	1	21	0

Note: Queue reported is the number of cars per lane.

Freeway Analysis

Existing

Horseshoe Grande Property (Freeway Analysis)
Existing
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): 4.1 Worst Case Level Of Service: F [52.7]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns representing traffic volumes and adjustment factors for each bound.

Critical Gap Module: Table with 12 columns showing critical gap values and follow-up times.

Capacity Module: Table with 12 columns showing conflict volumes, potential capacity, and volume-to-capacity ratios.

Level Of Service Module: Table with 12 columns showing LOS values, control delay, and approach delay.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Existing
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): 2.4 Worst Case Level Of Service: E[41.3]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 13 columns representing different traffic volumes and adjustments like Base Vol, Growth Adj, Initial Bse, etc.

Critical Gap Module: Table with 13 columns showing critical gap values and follow-up times for different movements.

Capacity Module: Table with 13 columns showing capacity-related metrics like Conflict Vol, Potent Cap., Move Cap., etc.

Level of Service Module: Table with 13 columns showing level of service values, control delay, and approach delay for different movements.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Existing
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 7.5 Worst Case Level Of Service: F[57.8]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control (Stop Sign, Uncontrolled), Rights (Include, Ignore), and Lanes (0 0 0 0 0).

Volume Module: Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume. Columns correspond to the four directions and their lanes.

Critical Gap Module: Critical Gp, FollowUpTim. Columns correspond to the four directions and their lanes.

Capacity Module: Cnflct Vol, Potent Cap., Move Cap., Volume/Cap. Columns correspond to the four directions and their lanes.

Level Of Service Module: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS. Columns correspond to the four directions and their lanes.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Existing
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 5.4 Worst Case Level Of Service: E[40.0]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control, Rights, and Lanes.

Volume Module: Table with 13 columns for volume metrics. Rows include Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume.

Critical Gap Module: Table with 13 columns for gap metrics. Rows include Critical Gp and FollowUpTim.

Capacity Module: Table with 13 columns for capacity metrics. Rows include Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with 13 columns for LOS metrics. Rows include 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Existing
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Base Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.755
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 31.4
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green, Lanes.

Volume Module: Table with columns: Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module: Table with columns: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module: Table with columns: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Existing
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Base Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 110 Critical Vol./Cap.(X): 0.887
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 43.9
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, Min. Green, Lanes.

Table with columns: Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Table with columns: Saturation Flow Module, Sat/Lane, Adjustment, Lanes, Final Sat.

Table with columns: Capacity Analysis Module, Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Existing
Evening Peak Hour

Level of Service Computation Report

2000 HCM Operations Method (Base Volume Alternative)

Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.845
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 32.8
Optimal Cycle: OPTIMIZED Level of Service: C

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Protected Protected Protected Protected
Rights: Include Include Include Include
Min. Green: 0 18 18 10 18 0 0 0 0 0 0 0
Lanes: 0 0 1 1 0 1 0 2 0 0 0 1 0 1 0 0 0 0 0 0

Volume Module:
Base Vol: 0 458 321 105 692 0 88 3 558 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 458 321 105 692 0 88 3 558 0 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.95 0.95 0.95 0.91 0.91 0.91 0.93 0.93 0.93 1.00 1.00 1.00
PHF Volume: 0 482 338 116 764 0 94 3 599 0 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 0 482 338 116 764 0 94 3 599 0 0 0
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Final Volume: 0 482 338 116 764 0 94 3 599 0 0 0

Saturation Flow Module:
Sat/Lane: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900
Adjustment: 1.00 0.89 0.89 0.95 0.95 1.00 0.81 0.81 0.81 1.00 1.00 1.00
Lanes: 0.00 1.18 0.82 1.00 2.00 0.00 0.97 0.03 1.00 0.00 0.00 0.00
Final Sat.: 0 1991 1395 1805 3610 0 1490 51 1541 0 0 0

Capacity Analysis Module:
Vol/Sat: 0.00 0.24 0.24 0.06 0.21 0.00 0.06 0.06 0.39 0.00 0.00 0.00
Crit Moves: **** **** ****
Green/Cycle: 0.00 0.27 0.27 0.11 0.38 0.00 0.44 0.44 0.44 0.00 0.00 0.00
Volume/Cap: 0.00 0.89 0.89 0.58 0.55 0.00 0.14 0.14 0.89 0.00 0.00 0.00
Delay/Veh: 0.0 41.8 41.8 42.1 22.1 0.0 15.2 15.2 35.2 0.0 0.0 0.0
User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
AdjDel/Veh: 0.0 41.8 41.8 42.1 22.1 0.0 15.2 15.2 35.2 0.0 0.0 0.0
LOS by Move: A D D D C A B B D A A A
HCM2kAvgQ: 0 15 15 4 9 0 2 2 20 0 0 0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Existing
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Base Volume Alternative)

Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.789
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 27.3
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0
Lanes:	0	0	1 1 0	1	0	2 0 0	0	1	0 1 0	0	0	0 0 0

Volume Module:

Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	554	499	107	443	0	56	1	354	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.96	0.96	0.96	0.90	0.90	0.90	1.00	1.00	1.00
PHF Volume:	0	585	527	112	464	0	62	1	393	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	585	527	112	464	0	62	1	393	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	585	527	112	464	0	62	1	393	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.05	0.95	1.00	2.00	0.00	0.98	0.02	1.00	0.00	0.00	0.00
Final Sat.:	0	1764	1589	1805	3610	0	1514	27	1541	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.33	0.33	0.06	0.13	0.00	0.04	0.04	0.26	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.00	0.40	0.40	0.11	0.51	0.00	0.31	0.31	0.31	0.00	0.00	0.00
Volume/Cap:	0.00	0.82	0.82	0.56	0.25	0.00	0.13	0.13	0.82	0.00	0.00	0.00
Delay/Veh:	0.0	28.4	28.4	41.4	12.3	0.0	22.4	22.4	38.6	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	28.4	28.4	41.4	12.3	0.0	22.4	22.4	38.6	0.0	0.0	0.0
LOS by Move:	A	C	C	D	B	A	C	C	D	A	A	A
HCM2kAvgQ:	0	17	17	4	4	0	1	1	13	0	0	0

Note: Queue reported is the number of cars per lane.

Opening Year (2010) Without Project – No Action

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) Without Project - No Action
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): 5.0 Worst Case Level Of Service: F [81.9]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Ignore			Ignore			Include		
Lanes:	1	0	1	0	0	1	0	0	1	1	0	0

Volume Module:

Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	335	190	0	0	693	41	28	0	208	0	0	0
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	335	190	0	0	693	41	28	0	208	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.93	0.93	0.00	0.91	0.91	0.00	1.00	1.00	1.00
PHF Volume:	352	199	0	0	747	0	30	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	352	199	0	0	747	0	30	0	0	0	0	0

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	xxxx	6.2	xxxxxx	xxxx	xxxxxx
FollowUpTim:	2.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	xxxx	3.3	xxxxxx	xxxx	xxxxxx

Capacity Module:

Cnflict Vol:	747	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	1649	xxxx	747	xxxx	xxxx	xxxxxx
Potent Cap.:	871	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	110	xxxx	416	xxxx	xxxx	xxxxxx
Move Cap.:	871	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	75	xxxx	416	xxxx	xxxx	xxxxxx
Volume/Cap:	0.40	xxxx	xxxx	xxxx	xxxx	xxxx	0.40	xxxx	0.00	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	2.0	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	1.6	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	11.9	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	81.9	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	B	*	*	*	*	*	F	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			81.9			xxxxxx		
ApproachLOS:	*			*			F			*		

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) Without Project - No Action
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): 2.8 Worst Case Level Of Service: F [56.5]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Ignore			Ignore			Include		
Lanes:	1	0	1	0	0	1	0	0	1	1	0	0

Volume Module:

Base Vol:	200	183	0	0	782	14	12	0	233	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	222	203	0	0	869	16	13	0	259	0	0	0
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	222	203	0	0	869	16	13	0	259	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.89	0.89	0.89	0.97	0.97	0.00	0.86	0.86	0.00	1.00	1.00	1.00
PHF Volume:	250	229	0	0	895	0	15	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	250	229	0	0	895	0	15	0	0	0	0	0

Critical Gap Module:

Critical Gp:	4.1	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	6.4	XXXX	6.2	XXXXXX	XXXX	XXXXXX
FollowUpTim:	2.2	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	3.5	XXXX	3.3	XXXXXX	XXXX	XXXXXX

Capacity Module:

Cnflct Vol:	895	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	1625	XXXX	895	XXXX	XXXX	XXXXXX
Potent Cap.:	767	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	114	XXXX	342	XXXX	XXXX	XXXXXX
Move Cap.:	767	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	85	XXXX	342	XXXX	XXXX	XXXXXX
Volume/Cap:	0.33	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	0.18	XXXX	0.00	XXXX	XXXX	XXXXXX

Level Of Service Module:

2Way95thQ:	1.4	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	0.6	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
Control Del:	12.0	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	56.5	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
LOS by Move:	B	*	*	*	*	*	F	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	XXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	XXXX	XXXX	XXXXXX
SharedQueue:	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
Shrd ConDel:	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	XXXXXX			XXXXXX			56.5			XXXXXX		
ApproachLOS:	*			*			F			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) Without Project - No Action
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.755
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 16.5
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	0	24	24	0	0	0	0	0	0
Lanes:	1	0	1	0	0	1	0	0	1	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	335	190	0	0	693	41	28	0	208	0	0	0
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	335	190	0	0	693	41	28	0	208	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.93	0.93	0.00	0.91	0.91	0.00	1.00	1.00	1.00
PHF Volume:	352	199	0	0	747	0	30	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	352	199	0	0	747	0	30	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	352	199	0	0	747	0	30	0	0	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	1900	1900	1900	0	1900	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.19	0.10	0.00	0.00	0.39	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.26	0.78	0.00	0.00	0.52	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.75	0.13	0.00	0.00	0.75	0.00	0.25	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	27.4	1.7	0.0	0.0	14.7	0.0	30.0	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	27.4	1.7	0.0	0.0	14.7	0.0	30.0	0.0	0.0	0.0	0.0	0.0
LOS by Move:	C	A	A	A	B	A	C	A	A	A	A	A
HCM2kAvgQ:	8	1	0	0	13	0	1	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) Without Project - No Action
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Cycle (sec): 65 Critical Vol./Cap.(X): 0.758
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 14.4
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	0	24	24	0	0	0	0	0	0
Lanes:	1	0	1	0	0	1	1	0	0	0	0	0

Volume Module:

Base Vol:	200	183	0	0	782	14	12	0	233	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	222	203	0	0	869	16	13	0	259	0	0	0
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	222	203	0	0	869	16	13	0	259	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.89	0.89	0.89	0.97	0.97	0.00	0.86	0.86	0.00	1.00	1.00	1.00
PHF Volume:	250	229	0	0	895	0	15	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	250	229	0	0	895	0	15	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	250	229	0	0	895	0	15	0	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	1900	1900	1900	0	1900	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.14	0.12	0.00	0.00	0.47	0.00	0.01	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.18	0.80	0.00	0.00	0.62	0.00	0.01	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.76	0.15	0.00	0.00	0.76	0.00	0.14	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	34.9	1.5	0.0	0.0	11.7	0.0	32.4	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	34.9	1.5	0.0	0.0	11.7	0.0	32.4	0.0	0.0	0.0	0.0	0.0
LOS by Move:	C	A	A	A	B	A	C	A	A	A	A	A
HCM2kAvgQ:	7	1	0	0	14	0	0	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) Without Project - No Action
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 15.0 Worst Case Level Of Service: F[120.1]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Ignore		
Lanes:	0	0	0	0	0	1!0	0	0	2	0	0	2

Volume Module:

Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	154	0	23	92	819	0	0	493	781
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	154	0	23	92	819	0	0	493	781
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.87	0.87	0.87	0.94	0.94	0.94	0.94	0.94	0.00
PHF Volume:	0	0	0	177	0	27	98	871	0	0	523	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	0	177	0	27	98	871	0	0	523	0

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxx	1155	1590	262	523	xxxx	xxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	xxxx	xxxx	xxxxx	193	109	743	1054	xxxx	xxxxx	xxxx	xxxx	xxxxx
Move Cap.:	xxxx	xxxx	xxxxx	180	98	743	1054	xxxx	xxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	xxxx	xxxx	xxxx	0.99	0.00	0.04	0.09	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.3	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	8.8	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	A	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	199	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	9.1	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	120	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	F	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			120.1			xxxxxx			xxxxxx		
ApproachLOS:	*			F			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) Without Project - No Action
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 9.7 Worst Case Level Of Service: F[73.5]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Ignore		
Lanes:	0	0	0	0	0	1	0	2	0	0	2	1

Volume Module:

Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	159	0	40	24	1120	0	0	378	651
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	159	0	40	24	1120	0	0	378	651
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.83	0.83	0.83	0.96	0.96	0.96	0.95	0.95	0.00
PHF Volume:	0	0	0	192	0	48	25	1165	0	0	400	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	0	192	0	48	25	1165	0	0	400	0

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxx	1033	1616	200	400	xxxx	xxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	xxxx	xxxx	xxxxx	232	105	814	1170	xxxx	xxxxx	xxxx	xxxx	xxxxx
Move Cap.:	xxxx	xxxx	xxxxx	228	102	814	1170	xxxx	xxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	xxxx	xxxx	xxxx	0.84	0.00	0.06	0.02	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.1	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	8.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	A	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	266	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	8.0	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	73.5	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	F	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			73.5			xxxxxx			xxxxxx		
ApproachLOS:	*			F			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) Without Project - No Action
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.449
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 9.2
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	0	0	1	1	0	2	0	0	2

-----|-----|-----|-----|

Volume Module:

Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	154	0	23	92	819	0	0	493	781
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	154	0	23	92	819	0	0	493	781
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.87	0.87	0.87	0.94	0.94	0.94	0.94	0.94	0.00
PHF Volume:	0	0	0	177	0	27	98	871	0	0	523	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	177	0	27	98	871	0	0	523	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	177	0	27	98	871	0	0	523	0

-----|-----|-----|-----|

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.73	1.00	0.73	0.44	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	0.87	0.00	0.13	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	1203	0	182	836	3610	0	0	3610	1900

-----|-----|-----|-----|

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.15	0.00	0.15	0.12	0.24	0.00	0.00	0.14	0.00
Crit Moves:				****				****				
Green/Cycle:	0.00	0.00	0.00	0.33	0.00	0.33	0.54	0.54	0.00	0.00	0.54	0.00
Volume/Cap:	0.00	0.00	0.00	0.45	0.00	0.45	0.22	0.45	0.00	0.00	0.27	0.00
Delay/Veh:	0.0	0.0	0.0	16.6	0.0	16.6	7.5	8.6	0.0	0.0	7.6	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	16.6	0.0	16.6	7.5	8.6	0.0	0.0	7.6	0.0
LOS by Move:	A	A	A	B	A	B	A	A	A	A	A	A
HCM2kAvgQ:	0	0	0	3	0	3	1	5	0	0	3	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) Without Project - No Action
 Evening Peak Hour - With Improvements

Level of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.570
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 9.6
 Optimal Cycle: OPTIMIZED Level of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	0	0	1	1	0	2	0	0	2

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	159	0	40	24	1120	0	0	378	651
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	159	0	40	24	1120	0	0	378	651
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.83	0.83	0.83	0.96	0.96	0.96	0.95	0.95	0.00
PHF Volume:	0	0	0	192	0	48	25	1165	0	0	400	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	192	0	48	25	1165	0	0	400	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	192	0	48	25	1165	0	0	400	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.74	1.00	0.74	0.52	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	0.80	0.00	0.20	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	1119	0	282	979	3610	0	0	3610	1900

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.00	0.00	0.00	0.17	0.00	0.17	0.03	0.32	0.00	0.00	0.11	0.00
Crit Moves:				****				****				
Green/Cycle:	0.00	0.00	0.00	0.30	0.00	0.30	0.57	0.57	0.00	0.00	0.57	0.00
Volume/Cap:	0.00	0.00	0.00	0.57	0.00	0.57	0.05	0.57	0.00	0.00	0.20	0.00
Delay/Veh:	0.0	0.0	0.0	19.6	0.0	19.6	5.8	8.7	0.0	0.0	6.4	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	19.6	0.0	19.6	5.8	8.7	0.0	0.0	6.4	0.0
LOS by Move:	A	A	A	B	A	B	A	A	A	A	A	A
HCM2kAvgQ:	0	0	0	5	0	5	0	8	0	0	2	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) Without Project - No Action
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 95 Critical Vol./Cap.(X): 0.808
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 35.1
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

Volume Module:

Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	504	182	0	0	273	69	0	0	0	297	13	89
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	504	182	0	0	273	69	0	0	0	297	13	89
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.82	0.82	0.82	0.83	0.83	0.83	1.00	1.00	1.00	0.89	0.89	0.89
PHF Volume:	614	222	0	0	328	83	0	0	0	332	15	99
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	614	222	0	0	328	83	0	0	0	332	15	99
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	614	222	0	0	328	83	0	0	0	332	15	99

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.59	0.41	0.00	0.00	0.00	1.00	0.13	0.87
Final Sat.:	1805	3610	0	0	2792	709	0	0	0	1550	198	1352

Capacity Analysis Module:

Vol/Sat:	0.34	0.06	0.00	0.00	0.12	0.12	0.00	0.00	0.00	0.21	0.07	0.07
Crit Moves:	****			****						****		
Green/Cycle:	0.39	0.58	0.00	0.00	0.19	0.19	0.00	0.00	0.00	0.25	0.25	0.25
Volume/Cap:	0.86	0.11	0.00	0.00	0.62	0.62	0.00	0.00	0.00	0.86	0.30	0.30
Delay/Veh:	37.1	8.8	0.0	0.0	37.2	37.2	0.0	0.0	0.0	48.2	29.1	29.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	37.1	8.8	0.0	0.0	37.2	37.2	0.0	0.0	0.0	48.2	29.1	29.1
LOS by Move:	D	A	A	A	D	D	A	A	A	D	C	C
HCM2kAvgQ:	19	1	0	0	7	7	0	0	0	13	3	3

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) Without Project - No Action
 Evening Peak Hour

Level of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

 Cycle (sec): 115 Critical Vol./Cap.(X): 0.954
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 53.7
 Optimal Cycle: OPTIMIZED Level of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	378	213	0	0	350	103	0	0	0	588	41	123
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	378	213	0	0	350	103	0	0	0	588	41	123
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.93	0.93	0.93	1.00	1.00	1.00	0.84	0.84	0.84
PHF Volume:	421	238	0	0	375	110	0	0	0	697	49	146
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	421	238	0	0	375	110	0	0	0	697	49	146
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	421	238	0	0	375	110	0	0	0	697	49	146

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.55	0.45	0.00	0.00	0.00	1.00	0.25	0.75
Final Sat.:	1805	3610	0	0	2695	793	0	0	0	1554	388	1165

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.23	0.07	0.00	0.00	0.14	0.14	0.00	0.00	0.00	0.45	0.13	0.13
Crit Moves:	****			****						****		
Green/Cycle:	0.24	0.40	0.00	0.00	0.16	0.16	0.00	0.00	0.00	0.46	0.46	0.46
Volume/Cap:	0.97	0.17	0.00	0.00	0.89	0.89	0.00	0.00	0.00	0.97	0.27	0.27
Delay/Veh:	78.1	22.4	0.0	0.0	63.8	63.8	0.0	0.0	0.0	52.2	19.0	19.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	78.1	22.4	0.0	0.0	63.8	63.8	0.0	0.0	0.0	52.2	19.0	19.0
LOS by Move:	E	C	A	A	E	E	A	A	A	D	B	B
HCM2kAvgQ:	20	3	0	0	12	12	0	0	0	31	4	4

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) Without Project - No Action
 Morning Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 95 Critical Vol./Cap.(X): 0.845
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 31.0
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound										
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Protected			Protected			Protected			Protected										
Rights:	Include			Include			Include			Include										
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0								
Lanes:	0	0	1	1	0	1	0	2	0	0	0	1	0	1	0	0	0	0	0	0

Volume Module:

Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	0	600	540	116	480	0	61	1	383	0	0	0
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	600	540	116	480	0	61	1	383	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.96	0.96	0.96	0.90	0.90	0.90	1.00	1.00	1.00
PHF Volume:	0	633	571	121	502	0	67	1	425	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	633	571	121	502	0	67	1	425	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	633	571	121	502	0	67	1	425	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.05	0.95	1.00	2.00	0.00	0.98	0.02	1.00	0.00	0.00	0.00
Final Sat.:	0	1764	1589	1805	3610	0	1514	27	1541	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.36	0.36	0.07	0.14	0.00	0.04	0.04	0.28	0.00	0.00	0.00	
Crit Moves:	****			****			****			****			
Green/Cycle:	0.00	0.41	0.41	0.11	0.52	0.00	0.32	0.32	0.32	0.00	0.00	0.00	
Volume/Cap:	0.00	0.87	0.87	0.64	0.27	0.00	0.14	0.14	0.87	0.00	0.00	0.00	
Delay/Veh:	0.0	32.3	32.3	47.8	13.0	0.0	23.3	23.3	44.9	0.0	0.0	0.0	
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
AdjDel/Veh:	0.0	32.3	32.3	47.8	13.0	0.0	23.3	23.3	44.9	0.0	0.0	0.0	
LOS by Move:	A	C	C	D	B	A	A	C	C	D	A	A	A
HCM2kAvgQ:	0	20	20	5	4	0	2	2	16	0	0	0	

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) Without Project - No Action
 Evening Peak Hour

Level of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 100 Critical Vol./Cap.(X): 0.896
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 38.4
 Optimal Cycle: OPTIMIZED Level of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound										
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Protected			Protected			Protected			Protected										
Rights:	Include			Include			Include			Include										
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0								
Lanes:	0	0	1	1	0	1	0	2	0	0	0	1	0	1	0	0	0	0	0	0

Volume Module:

Base Vol:	0	458	321	105	692	0	88	3	558	0	0	0
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	0	496	348	114	749	0	95	3	604	0	0	0
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	496	348	114	749	0	95	3	604	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.91	0.91	0.91	0.93	0.93	0.93	1.00	1.00	1.00
PHF Volume:	0	522	366	125	827	0	102	3	648	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	522	366	125	827	0	102	3	648	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	522	366	125	827	0	102	3	648	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.89	0.89	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.18	0.82	1.00	2.00	0.00	0.97	0.03	1.00	0.00	0.00	0.00
Final Sat.:	0	1991	1395	1805	3610	0	1490	51	1541	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.26	0.26	0.07	0.23	0.00	0.07	0.07	0.42	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.00	0.28	0.28	0.10	0.38	0.00	0.46	0.46	0.46	0.00	0.00	0.00
Volume/Cap:	0.00	0.92	0.92	0.70	0.60	0.00	0.15	0.15	0.92	0.00	0.00	0.00
Delay/Veh:	0.0	48.7	48.7	54.7	25.3	0.0	15.9	15.9	41.4	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	48.7	48.7	54.7	25.3	0.0	15.9	15.9	41.4	0.0	0.0	0.0
LOS by Move:	A	D	D	D	C	A	B	B	D	A	A	A
HCM2kAvgQ:	0	18	18	5	11	0	2	2	24	0	0	0

Note: Queue reported is the number of cars per lane.

Opening Year (2010) With Project – Proposed Action “A”

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "A"
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): 5.0 Worst Case Level Of Service: F[87.2]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Ignore			Ignore			Include		
Lanes:	1	0	0	0	0	1	1	0	0	0	0	0

Volume Module:

Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	335	190	0	0	693	41	28	0	208	0	0	0
Added Vol:	0	29	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	335	219	0	0	693	41	28	0	208	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.93	0.93	0.00	0.91	0.91	0.00	1.00	1.00	1.00
PHF Volume:	352	230	0	0	747	0	30	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	352	230	0	0	747	0	30	0	0	0	0	0

Critical Gap Module:

Critical Gp:	4.1	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	6.4	XXXX	6.2	XXXXXX	XXXX	XXXXXX
FollowUpTim:	2.2	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	3.5	XXXX	3.3	XXXXXX	XXXX	XXXXXX

Capacity Module:

Cnflct Vol:	747	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	1680	XXXX	747	XXXX	XXXX	XXXXXX
Potent Cap.:	871	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	105	XXXX	416	XXXX	XXXX	XXXXXX
Move Cap.:	871	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	72	XXXX	416	XXXX	XXXX	XXXXXX
Volume/Cap:	0.40	XXXX	XXXX	XXXX	XXXX	XXXX	0.42	XXXX	0.00	XXXX	XXXX	XXXX

Level Of Service Module:

2Way95thQ:	2.0	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	1.7	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
Control Del:	11.9	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	87.2	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
LOS by Move:	B	*	*	*	*	*	F	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	XXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	XXXX	XXXX	XXXXXX
SharedQueue:	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
Shrd ConDel:	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	XXXXXX			XXXXXX			87.2			XXXXXX		
ApproachLOS:	*			*			F			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "A"
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): 2.7 Worst Case Level Of Service: F[65.2]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Ignore			Ignore			Include		
Lanes:	1	0	0	0	1	0	1	0	0	0	0	0

Volume Module:

Base Vol:	200	183	0	0	782	14	12	0	233	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	222	203	0	0	869	16	13	0	259	0	0	0
Added Vol:	0	80	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	222	283	0	0	869	16	13	0	259	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.89	0.89	0.89	0.97	0.97	0.00	0.86	0.86	0.00	1.00	1.00	1.00
PHF Volume:	250	319	0	0	895	0	15	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	250	319	0	0	895	0	15	0	0	0	0	0

Critical Gap Module:

Critical Gp:	4.1	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	6.4	XXXX	6.2	XXXXXX	XXXX	XXXXXX
FollowUpTim:	2.2	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	3.5	XXXX	3.3	XXXXXX	XXXX	XXXXXX

Capacity Module:

Cnflct Vol:	895	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	1715	XXXX	895	XXXX	XXXX	XXXXXX
Potent Cap.:	767	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	100	XXXX	342	XXXX	XXXX	XXXXXX
Move Cap.:	767	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	75	XXXX	342	XXXX	XXXX	XXXXXX
Volume/Cap:	0.33	XXXX	XXXX	XXXX	XXXX	XXXX	0.21	XXXX	0.00	XXXX	XXXX	XXXX

Level Of Service Module:

2Way95thQ:	1.4	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	0.7	XXXX	XXXXXX	XXXX	XXXX	XXXXXX
Control Del:	12.0	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	65.2	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
LOS by Move:	B	*	*	*	*	*	F	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	XXXX	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	XXXX	XXXX	XXXXXX
SharedQueue:	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
Shrd ConDel:	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	XXXXXX			XXXXXX			65.2			XXXXXX		
ApproachLOS:	*			*			F			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "A"
 Morning Peak Hour - With Improvements

Level of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.719
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 17.2
 Optimal Cycle: OPTIMIZED Level of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	0	24	24	0	0	0	0	0	0
Lanes:	1	0	1	0	0	1	1	0	0	0	0	0

Volume Module:

Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	335	190	0	0	693	41	28	0	208	0	0	0
Added Vol:	0	29	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	335	219	0	0	693	41	28	0	208	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.93	0.93	0.00	0.91	0.91	0.00	1.00	1.00	1.00
PHF Volume:	352	230	0	0	747	0	30	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	352	230	0	0	747	0	30	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	352	230	0	0	747	0	30	0	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	1900	1900	1900	0	1900	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.19	0.12	0.00	0.00	0.39	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.27	0.82	0.00	0.00	0.55	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.72	0.15	0.00	0.00	0.72	0.00	0.32	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	29.9	1.5	0.0	0.0	15.2	0.0	38.0	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	29.9	1.5	0.0	0.0	15.2	0.0	38.0	0.0	0.0	0.0	0.0	0.0
LOS by Move:	C	A	A	A	B	A	D	A	A	A	A	A
HCM2kAvgQ:	9	1	0	0	14	0	1	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "A"
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Cycle (sec): 110 Critical Vol./Cap.(X): 0.693
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 16.2
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	0	24	24	0	0	0	0	0	0
Lanes:	1	0	1	0	0	1	0	0	1	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	200	183	0	0	782	14	12	0	233	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	222	203	0	0	869	16	13	0	259	0	0	0
Added Vol:	0	80	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	222	283	0	0	869	16	13	0	259	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.89	0.89	0.89	0.97	0.97	0.00	0.86	0.86	0.00	1.00	1.00	1.00
PHF Volume:	250	319	0	0	895	0	15	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	250	319	0	0	895	0	15	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	250	319	0	0	895	0	15	0	0	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	1900	1900	1900	0	1900	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.14	0.17	0.00	0.00	0.47	0.00	0.01	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.20	0.88	0.00	0.00	0.68	0.00	0.01	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.69	0.19	0.00	0.00	0.69	0.00	0.24	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	46.6	1.0	0.0	0.0	12.4	0.0	55.7	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	46.6	1.0	0.0	0.0	12.4	0.0	55.7	0.0	0.0	0.0	0.0	0.0
LOS by Move:	D	A	A	A	B	A	E	A	A	A	A	A
HCM2kAvgQ:	9	2	0	0	19	0	1	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "A"
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 49.1 Worst Case Level Of Service: F[310.5]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Ignore		
Lanes:	0	0	0	0	0	1	0	2	0	0	2	1

Volume Module:

Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	154	0	23	92	819	0	0	493	781
Added Vol:	0	0	0	68	0	0	0	0	0	0	29	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	222	0	23	92	819	0	0	522	781
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.87	0.87	0.87	0.94	0.94	0.94	0.94	0.94	0.00
PHF Volume:	0	0	0	256	0	27	98	871	0	0	554	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	0	256	0	27	98	871	0	0	554	0

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflict Vol:	xxxx	xxxx	xxxxx	1185	1621	277	554	xxxx	xxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	xxxx	xxxx	xxxxx	185	104	727	1027	xxxx	xxxxx	xxxx	xxxx	xxxxx
Move Cap.:	xxxx	xxxx	xxxxx	171	94	727	1027	xxxx	xxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	xxxx	xxxx	xxxx	1.49	0.00	0.04	0.10	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.3	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	8.9	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	A	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	184	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	18.1	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	310	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	F	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			310.5			xxxxxx			xxxxxx		
ApproachLOS:	*			F			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "A"
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 54.4 Worst Case Level Of Service: F[314.5]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Ignore		
Lanes:	0	0	0	0	0	1	0	2	0	0	2	1

Volume Module:

Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	159	0	40	24	1120	0	0	378	651
Added Vol:	0	0	0	91	0	0	0	0	0	0	80	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	250	0	40	24	1120	0	0	458	651
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.83	0.83	0.83	0.96	0.96	0.96	0.95	0.95	0.00
PHF Volume:	0	0	0	301	0	48	25	1165	0	0	484	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	0	301	0	48	25	1165	0	0	484	0

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxx	1118	1700	242	484	xxxx	xxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	xxxx	xxxx	xxxxx	204	93	765	1089	xxxx	xxxxx	xxxx	xxxx	xxxxx
Move Cap.:	xxxx	xxxx	xxxxx	201	91	765	1089	xxxx	xxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	xxxx	xxxx	xxxx	1.50	0.00	0.06	0.02	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.1	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	8.4	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	A	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	223	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	21.8	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	314	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	F	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			314.5			xxxxxx			xxxxxx		
ApproachLOS:	*			F			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "A"
 Morning Peak Hour - With Improvements

Level of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.515
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 11.5
 Optimal Cycle: OPTIMIZED Level of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	0	0	1	1	0	2	0	0	2

Volume Module:

Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	154	0	23	92	819	0	0	493	781
Added Vol:	0	0	0	68	0	0	0	0	0	0	29	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	222	0	23	92	819	0	0	522	781
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.87	0.87	0.87	0.94	0.94	0.94	0.94	0.94	0.00
PHF Volume:	0	0	0	256	0	27	98	871	0	0	554	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	256	0	27	98	871	0	0	554	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	256	0	27	98	871	0	0	554	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.72	1.00	0.72	0.41	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	0.91	0.00	0.09	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	1246	0	131	773	3610	0	0	3610	1900

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.21	0.00	0.21	0.13	0.24	0.00	0.00	0.15	0.00	
Crit Moves:				****				****					
Green/Cycle:	0.00	0.00	0.00	0.40	0.00	0.40	0.47	0.47	0.00	0.00	0.47	0.00	
Volume/Cap:	0.00	0.00	0.00	0.52	0.00	0.52	0.27	0.52	0.00	0.00	0.33	0.00	
Delay/Veh:	0.0	0.0	0.0	14.5	0.0	14.5	10.1	11.5	0.0	0.0	10.1	0.0	
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
AdjDel/Veh:	0.0	0.0	0.0	14.5	0.0	14.5	10.1	11.5	0.0	0.0	10.1	0.0	
LOS by Move:	A	A	A	B	A	B	B	B	A	A	B	A	
HCM2kAvgQ:	0	0	0	5	0	5	1	6	0	0	4	0	

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "A"
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.663
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 12.8
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	0	0	1	1	0	2	0	0	2

Volume Module:

Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	159	0	40	24	1120	0	0	378	651
Added Vol:	0	0	0	91	0	0	0	0	0	0	80	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	250	0	40	24	1120	0	0	458	651
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.83	0.83	0.83	0.96	0.96	0.96	0.95	0.95	0.00
PHF Volume:	0	0	0	301	0	48	25	1165	0	0	484	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	301	0	48	25	1165	0	0	484	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
FinalVolume:	0	0	0	301	0	48	25	1165	0	0	484	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.73	1.00	0.73	0.46	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	0.86	0.00	0.14	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	1195	0	191	865	3610	0	0	3610	1900

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.25	0.00	0.25	0.03	0.32	0.00	0.00	0.13	0.00
Crit Moves:				****				****				
Green/Cycle:	0.00	0.00	0.00	0.38	0.00	0.38	0.49	0.49	0.00	0.00	0.49	0.00
Volume/Cap:	0.00	0.00	0.00	0.66	0.00	0.66	0.06	0.66	0.00	0.00	0.28	0.00
Delay/Veh:	0.0	0.0	0.0	18.6	0.0	18.6	8.2	12.6	0.0	0.0	9.2	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	18.6	0.0	18.6	8.2	12.6	0.0	0.0	9.2	0.0
LOS by Move:	A	A	A	B	A	B	A	B	A	A	A	A
HCM2kAvgQ:	0	0	0	7	0	7	0	10	0	0	3	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "A"
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 105 Critical Vol./Cap.(X): 0.847
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 40.0
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	0	0	0	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	504	182	0	0	273	69	0	0	0	297	13	89
Added Vol:	29	0	0	0	0	0	0	0	0	36	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	533	182	0	0	273	69	0	0	0	333	13	89
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.82	0.82	0.82	0.83	0.83	0.83	1.00	1.00	1.00	0.89	0.89	0.89
PHF Volume:	649	222	0	0	328	83	0	0	0	373	15	99
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	649	222	0	0	328	83	0	0	0	373	15	99
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	649	222	0	0	328	83	0	0	0	373	15	99

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.81	0.81	0.81
Lanes:	1.00	2.00	0.00	0.00	1.59	0.41	0.00	0.00	0.00	1.00	0.13	0.87
Final Sat.:	1805	3610	0	0	2792	709	0	0	0	1548	198	1350

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.36	0.06	0.00	0.00	0.12	0.12	0.00	0.00	0.00	0.24	0.07	0.07
Crit Moves:	****			****						****		
Green/Cycle:	0.41	0.58	0.00	0.00	0.17	0.17	0.00	0.00	0.00	0.27	0.27	0.27
Volume/Cap:	0.89	0.11	0.00	0.00	0.69	0.69	0.00	0.00	0.00	0.89	0.27	0.27
Delay/Veh:	41.8	10.1	0.0	0.0	44.1	44.1	0.0	0.0	0.0	52.9	30.2	30.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	41.8	10.1	0.0	0.0	44.1	44.1	0.0	0.0	0.0	52.9	30.2	30.2
LOS by Move:	D	B	A	A	D	D	A	A	A	D	C	C
HCM2kAvgQ:	23	2	0	0	8	8	0	0	0	16	3	3

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "A"
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.046
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 74.2
 Optimal Cycle: OPTIMIZED Level Of Service: E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

Volume Module:

Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	378	213	0	0	350	103	0	0	0	588	41	123
Added Vol:	80	0	0	0	0	0	0	0	0	47	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	458	213	0	0	350	103	0	0	0	635	41	123
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.93	0.93	0.93	1.00	1.00	1.00	0.84	0.84	0.84
PHF Volume:	510	238	0	0	375	110	0	0	0	753	49	146
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	510	238	0	0	375	110	0	0	0	753	49	146
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	510	238	0	0	375	110	0	0	0	753	49	146

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.55	0.45	0.00	0.00	0.00	1.00	0.25	0.75
Final Sat.:	1805	3610	0	0	2695	793	0	0	0	1554	388	1165

Capacity Analysis Module:

Vol/Sat:	0.28	0.07	0.00	0.00	0.14	0.14	0.00	0.00	0.00	0.48	0.13	0.13
Crit Moves:	****			****						****		
Green/Cycle:	0.26	0.41	0.00	0.00	0.15	0.15	0.00	0.00	0.00	0.45	0.45	0.45
Volume/Cap:	1.07	0.16	0.00	0.00	0.93	0.93	0.00	0.00	0.00	1.07	0.28	0.28
Delay/Veh:	105.6	22.1	0.0	0.0	73.2	73.2	0.0	0.0	0.0	83.8	20.6	20.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	105.6	22.1	0.0	0.0	73.2	73.2	0.0	0.0	0.0	83.8	20.6	20.6
LOS by Move:	F	C	A	A	E	E	A	A	A	F	C	C
HCM2kAvgQ:	27	3	0	0	13	13	0	0	0	40	5	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "A"
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 85 Critical Vol./Cap.(X): 0.736
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 28.4
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	504	182	0	0	273	69	0	0	0	297	13	89
Added Vol:	29	0	0	0	0	0	0	0	0	36	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	533	182	0	0	273	69	0	0	0	333	13	89
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.82	0.82	0.82	0.83	0.83	0.83	1.00	1.00	1.00	0.89	0.89	0.89
PHF Volume:	649	222	0	0	328	83	0	0	0	373	15	99
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	649	222	0	0	328	83	0	0	0	373	15	99
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	649	222	0	0	328	83	0	0	0	373	15	99

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.81	0.81	0.81
Lanes:	1.00	2.00	0.00	0.00	1.59	0.41	0.00	0.00	0.00	2.00	0.13	0.87
Final Sat.:	1805	3610	0	0	2792	709	0	0	0	3096	198	1350

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.36	0.06	0.00	0.00	0.12	0.12	0.00	0.00	0.00	0.12	0.07	0.07
Crit Moves:	****			****						****		
Green/Cycle:	0.45	0.66	0.00	0.00	0.21	0.21	0.00	0.00	0.00	0.15	0.15	0.15
Volume/Cap:	0.80	0.09	0.00	0.00	0.55	0.55	0.00	0.00	0.00	0.80	0.49	0.49
Delay/Veh:	25.8	5.2	0.0	0.0	30.8	30.8	0.0	0.0	0.0	42.3	33.5	33.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	25.8	5.2	0.0	0.0	30.8	30.8	0.0	0.0	0.0	42.3	33.5	33.5
LOS by Move:	C	A	A	A	C	C	A	A	A	D	C	C
HCM2kAvgQ:	17	1	0	0	6	6	0	0	0	7	4	4

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "A"
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.808
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 33.0
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	378	213	0	0	350	103	0	0	0	588	41	123
Added Vol:	80	0	0	0	0	0	0	0	0	47	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	458	213	0	0	350	103	0	0	0	635	41	123
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.93	0.93	0.93	1.00	1.00	1.00	0.84	0.84	0.84
PHF Volume:	510	238	0	0	375	110	0	0	0	753	49	146
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	510	238	0	0	375	110	0	0	0	753	49	146
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	510	238	0	0	375	110	0	0	0	753	49	146

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.55	0.45	0.00	0.00	0.00	2.00	0.25	0.75
Final Sat.:	1805	3610	0	0	2695	793	0	0	0	3107	388	1165

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.28	0.07	0.00	0.00	0.14	0.14	0.00	0.00	0.00	0.24	0.13	0.13
Crit Moves:	****			****			****			****		
Green/Cycle:	0.33	0.53	0.00	0.00	0.20	0.20	0.00	0.00	0.00	0.29	0.29	0.29
Volume/Cap:	0.84	0.12	0.00	0.00	0.70	0.70	0.00	0.00	0.00	0.84	0.44	0.44
Delay/Veh:	38.2	10.4	0.0	0.0	36.5	36.5	0.0	0.0	0.0	36.1	26.3	26.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	38.2	10.4	0.0	0.0	36.5	36.5	0.0	0.0	0.0	36.1	26.3	26.3
LOS by Move:	D	B	A	A	D	D	A	A	A	D	C	C
HCM2kAvgQ:	16	2	0	0	8	8	0	0	0	13	5	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "A"
 Morning Peak Hour

Level of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

Cycle (sec): 110 Critical Vol./Cap.(X): 0.896
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 38.3
 Optimal Cycle: OPTIMIZED Level of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound											
Movement:	L	T	R	L	T	R	L	T	R	L	T	R									
Control:	Protected			Protected			Protected			Protected											
Rights:	Include			Include			Include			Include											
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0									
Lanes:	0	0	1	1	0	0	1	0	2	0	0	0	1	0	1	0	0	0	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	0	600	540	116	480	0	61	1	383	0	0	0
Added Vol:	0	29	16	0	36	0	0	0	68	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	629	556	116	516	0	61	1	451	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.96	0.96	0.96	0.90	0.90	0.90	1.00	1.00	1.00
PHF Volume:	0	664	587	121	540	0	67	1	501	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	664	587	121	540	0	67	1	501	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	664	587	121	540	0	67	1	501	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.06	0.94	1.00	2.00	0.00	0.98	0.02	1.00	0.00	0.00	0.00
Final Sat.:	0	1781	1576	1805	3610	0	1512	27	1539	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.00	0.37	0.37	0.07	0.15	0.00	0.04	0.04	0.33	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.00	0.41	0.41	0.09	0.50	0.00	0.36	0.36	0.36	0.00	0.00	0.00
Volume/Cap:	0.00	0.91	0.91	0.74	0.30	0.00	0.13	0.13	0.91	0.00	0.00	0.00
Delay/Veh:	0.0	40.5	40.5	65.0	16.4	0.0	23.9	23.9	52.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	40.5	40.5	65.0	16.4	0.0	23.9	23.9	52.0	0.0	0.0	0.0
LOS by Move:	A	D	D	E	B	A	C	C	D	A	A	A
HCM2kAvgQ:	0	25	25	6	6	0	2	2	21	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "A"
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.985
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 54.9
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
	L	T	R	L	T	R	L	T	R	L	T	R
Movement:												
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0
Lanes:	0	0	1	1	0	2	0	1	0	1	0	0

Volume Module:												
Base Vol:	0	458	321	105	692	0	88	3	558	0	0	0
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	0	496	348	114	749	0	95	3	604	0	0	0
Added Vol:	0	80	42	0	47	0	0	0	91	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	576	390	114	796	0	95	3	695	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.91	0.91	0.91	0.93	0.93	0.93	1.00	1.00	1.00
PHF Volume:	0	606	410	125	879	0	102	3	746	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	606	410	125	879	0	102	3	746	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	606	410	125	879	0	102	3	746	0	0	0

Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.89	0.89	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.19	0.81	1.00	2.00	0.00	0.97	0.03	1.00	0.00	0.00	0.00
Final Sat.:	0	2024	1369	1805	3610	0	1490	51	1540	0	0	0

Capacity Analysis Module:												
Vol/Sat:	0.00	0.30	0.30	0.07	0.24	0.00	0.07	0.07	0.48	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.00	0.30	0.30	0.08	0.38	0.00	0.48	0.48	0.48	0.00	0.00	0.00
Volume/Cap:	0.00	1.00	1.00	0.83	0.64	0.00	0.14	0.14	1.00	0.00	0.00	0.00
Delay/Veh:	0.0	70.4	70.4	85.4	31.2	0.0	17.2	17.2	61.9	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	70.4	70.4	85.4	31.2	0.0	17.2	17.2	61.9	0.0	0.0	0.0
LOS by Move:	A	E	E	F	C	A	B	B	E	A	A	A
HCM2kAvgQ:	0	26	26	7	14	0	2	2	36	0	0	0

Note: Queue reported is the number of cars per lane.

Opening Year (2010) With Project – Proposed Action “B”

Horseshoe Grande Property (Freeway Analysis)
Opening Year (2010) With Project - Proposed Action "B"
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): 5.0 Worst Case Level Of Service: F[87.0]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 13 columns representing different traffic volumes and adjustment factors like Base Vol, Growth Adj, Initial Bse, etc.

Critical Gap Module: Table with 13 columns showing critical gap values and follow-up times for different movements.

Capacity Module: Table with 13 columns showing capacity-related metrics like Cnflict Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with 13 columns showing Level of Service (LOS) and control delay for different movements.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Opening Year (2010) With Project - Proposed Action "B"
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): 2.7 Worst Case Level Of Service: F[64.8]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 13 columns representing different traffic volumes and adjustment factors like Base Vol, Growth Adj, Initial Bse, etc.

Critical Gap Module: Table with 13 columns showing critical gap values and follow-up times for different movements.

Capacity Module: Table with 13 columns showing capacity-related metrics like Cnflict Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with 13 columns showing Level of Service (LOS) for different movements and approach delays.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "B"
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.719
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 17.2
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	0	24	24	0	0	0	0	0	0
Lanes:	1	0	1	0	0	1	0	0	1	0	0	0

Volume Module:

Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	335	190	0	0	693	41	28	0	208	0	0	0
Added Vol:	0	28	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	335	218	0	0	693	41	28	0	208	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.93	0.93	0.00	0.91	0.91	0.00	1.00	1.00	1.00
PHF Volume:	352	228	0	0	747	0	30	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	352	228	0	0	747	0	30	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
FinalVolume:	352	228	0	0	747	0	30	0	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	1900	1900	1900	0	1900	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.19	0.12	0.00	0.00	0.39	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.27	0.82	0.00	0.00	0.55	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.72	0.15	0.00	0.00	0.72	0.00	0.32	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	29.9	1.5	0.0	0.0	15.2	0.0	38.0	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	29.9	1.5	0.0	0.0	15.2	0.0	38.0	0.0	0.0	0.0	0.0	0.0
LOS by Move:	C	A	A	A	B	A	D	A	A	A	A	A
HCM2kAvgQ:	9	1	0	0	14	0	1	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "B"
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Cycle (sec): 110 Critical Vol./Cap.(X): 0.693
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 16.2
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	0	24	24	0	0	0	0	0	0
Lanes:	1	0	1	0	0	1	1	0	0	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	200	183	0	0	782	14	12	0	233	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	222	203	0	0	869	16	13	0	259	0	0	0
Added Vol:	0	77	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	222	280	0	0	869	16	13	0	259	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.89	0.89	0.89	0.97	0.97	0.00	0.86	0.86	0.00	1.00	1.00	1.00
PHF Volume:	250	316	0	0	895	0	15	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	250	316	0	0	895	0	15	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
FinalVolume:	250	316	0	0	895	0	15	0	0	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	1900	1900	1900	0	1900	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.14	0.17	0.00	0.00	0.47	0.00	0.01	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.20	0.88	0.00	0.00	0.68	0.00	0.01	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.69	0.19	0.00	0.00	0.69	0.00	0.24	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	46.6	1.0	0.0	0.0	12.4	0.0	55.7	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	46.6	1.0	0.0	0.0	12.4	0.0	55.7	0.0	0.0	0.0	0.0	0.0
LOS by Move:	D	A	A	A	B	A	E	A	A	A	A	A
HCM2kAvgQ:	9	2	0	0	19	0	1	0	0	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "B"
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 47.7 Worst Case Level Of Service: F[303.8]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Ignore		
Lanes:	0	0	0	0	0	1	0	2	0	0	2	1

Volume Module:

Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	154	0	23	92	819	0	0	493	781
Added Vol:	0	0	0	66	0	0	0	0	0	0	28	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	220	0	23	92	819	0	0	521	781
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.87	0.87	0.87	0.94	0.94	0.94	0.94	0.94	0.00
PHF Volume:	0	0	0	253	0	27	98	871	0	0	553	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	0	253	0	27	98	871	0	0	553	0

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	xxxxx	xxxx	xxxxx	1184	1620	276	553	xxxx	xxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	xxxx	xxxx	xxxxx	185	104	727	1027	xxxx	xxxxx	xxxx	xxxx	xxxxx
Move Cap.:	xxxx	xxxx	xxxxx	171	94	727	1027	xxxx	xxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	xxxx	xxxx	xxxx	1.48	0.00	0.04	0.10	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.3	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	8.9	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	A	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	185	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	17.8	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	304	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	F	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			303.8			xxxxxx			xxxxxx		
ApproachLOS:	*			F			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "B"
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 52.2 Worst Case Level Of Service: F[303.6]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Ignore		
Lanes:	0	0	0	0	0	1	0	2	0	0	2	1

Volume Module:

Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	159	0	40	24	1120	0	0	378	651
Added Vol:	0	0	0	88	0	0	0	0	0	0	77	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	247	0	40	24	1120	0	0	455	651
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.83	0.83	0.83	0.96	0.96	0.96	0.95	0.95	0.00
PHF Volume:	0	0	0	298	0	48	25	1165	0	0	481	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	0	298	0	48	25	1165	0	0	481	0

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	4.1	xxxx	xxxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	2.2	xxxx	xxxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxx	1115	1697	241	481	xxxx	xxxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	xxxx	xxxx	xxxxx	205	93	767	1092	xxxx	xxxxxx	xxxx	xxxx	xxxxx
Move Cap.:	xxxx	xxxx	xxxxx	202	91	767	1092	xxxx	xxxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	xxxx	xxxx	xxxx	1.48	0.00	0.06	0.02	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.1	xxxx	xxxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	8.4	xxxx	xxxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	A	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	225	xxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	21.3	xxxxx	xxxxx	xxxx	xxxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	304	xxxxx	xxxxx	xxxx	xxxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	F	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			303.6			xxxxxx			xxxxxx		
ApproachLOS:	*			F			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "B"
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.513
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 11.4
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	0	0	1	1	0	2	0	0	2

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	154	0	23	92	819	0	0	493	781
Added Vol:	0	0	0	66	0	0	0	0	0	0	28	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	220	0	23	92	819	0	0	521	781
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.87	0.87	0.87	0.94	0.94	0.94	0.94	0.94	0.00
PHF Volume:	0	0	0	253	0	27	98	871	0	0	553	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	253	0	27	98	871	0	0	553	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	253	0	27	98	871	0	0	553	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.72	1.00	0.72	0.41	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	0.90	0.00	0.10	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	1245	0	132	775	3610	0	0	3610	1900

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.00	0.00	0.00	0.20	0.00	0.20	0.13	0.24	0.00	0.00	0.15	0.00
Crit Moves:				****				****				
Green/Cycle:	0.00	0.00	0.00	0.40	0.00	0.40	0.47	0.47	0.00	0.00	0.47	0.00
Volume/Cap:	0.00	0.00	0.00	0.51	0.00	0.51	0.27	0.51	0.00	0.00	0.33	0.00
Delay/Veh:	0.0	0.0	0.0	14.6	0.0	14.6	10.0	11.4	0.0	0.0	10.1	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	14.6	0.0	14.6	10.0	11.4	0.0	0.0	10.1	0.0
LOS by Move:	A	A	A	B	A	B	B	B	A	A	B	A
HCM2kAvgQ:	0	0	0	5	0	5	1	6	0	0	4	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "B"
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.660
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 12.7
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	0	0	1	1	0	2	0	0	2

Volume Module:

Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	159	0	40	24	1120	0	0	378	651
Added Vol:	0	0	0	88	0	0	0	0	0	0	77	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	247	0	40	24	1120	0	0	455	651
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.83	0.83	0.83	0.96	0.96	0.96	0.95	0.95	0.00
PHF Volume:	0	0	0	298	0	48	25	1165	0	0	481	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	298	0	48	25	1165	0	0	481	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	298	0	48	25	1165	0	0	481	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.73	1.00	0.73	0.46	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	0.86	0.00	0.14	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	1193	0	193	868	3610	0	0	3610	1900

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.25	0.00	0.25	0.03	0.32	0.00	0.00	0.13	0.00	
Crit Moves:				****				****					
Green/Cycle:	0.00	0.00	0.00	0.38	0.00	0.38	0.49	0.49	0.00	0.00	0.49	0.00	
Volume/Cap:	0.00	0.00	0.00	0.66	0.00	0.66	0.06	0.66	0.00	0.00	0.27	0.00	
Delay/Veh:	0.0	0.0	0.0	18.6	0.0	18.6	8.1	12.5	0.0	0.0	9.1	0.0	
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
AdjDel/Veh:	0.0	0.0	0.0	18.6	0.0	18.6	8.1	12.5	0.0	0.0	9.1	0.0	
LOS by Move:	A	A	A	B	A	B	A	B	A	A	A	A	
HCM2kAvgQ:	0	0	0	7	0	7	0	10	0	0	3	0	

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "B"
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 105 Critical Vol./Cap.(X): 0.845
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 39.8
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	504	182	0	0	273	69	0	0	0	297	13	89
Added Vol:	28	0	0	0	0	0	0	0	0	35	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	532	182	0	0	273	69	0	0	0	332	13	89
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.82	0.82	0.82	0.83	0.83	0.83	1.00	1.00	1.00	0.89	0.89	0.89
PHF Volume:	648	222	0	0	328	83	0	0	0	371	15	99
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	648	222	0	0	328	83	0	0	0	371	15	99
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	648	222	0	0	328	83	0	0	0	371	15	99

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.81	0.81	0.81
Lanes:	1.00	2.00	0.00	0.00	1.59	0.41	0.00	0.00	0.00	1.00	0.13	0.87
Final Sat.:	1805	3610	0	0	2792	709	0	0	0	1548	198	1350

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.36	0.06	0.00	0.00	0.12	0.12	0.00	0.00	0.00	0.24	0.07	0.07
Crit Moves:	****			****						****		
Green/Cycle:	0.41	0.58	0.00	0.00	0.17	0.17	0.00	0.00	0.00	0.27	0.27	0.27
Volume/Cap:	0.89	0.11	0.00	0.00	0.69	0.69	0.00	0.00	0.00	0.89	0.27	0.27
Delay/Veh:	41.5	10.0	0.0	0.0	44.1	44.1	0.0	0.0	0.0	52.6	30.2	30.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	41.5	10.0	0.0	0.0	44.1	44.1	0.0	0.0	0.0	52.6	30.2	30.2
LOS by Move:	D	B	A	A	D	D	A	A	A	D	C	C
HCM2kAvgQ:	23	2	0	0	8	8	0	0	0	15	3	3

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "B"
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.043
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 73.4
 Optimal Cycle: OPTIMIZED Level Of Service: E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	378	213	0	0	350	103	0	0	0	588	41	123
Added Vol:	77	0	0	0	0	0	0	0	0	46	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	455	213	0	0	350	103	0	0	0	634	41	123
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.93	0.93	0.93	1.00	1.00	1.00	0.84	0.84	0.84
PHF Volume:	507	238	0	0	375	110	0	0	0	752	49	146
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	507	238	0	0	375	110	0	0	0	752	49	146
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	507	238	0	0	375	110	0	0	0	752	49	146

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.55	0.45	0.00	0.00	0.00	1.00	0.25	0.75
Final Sat.:	1805	3610	0	0	2695	793	0	0	0	1554	388	1165

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.28	0.07	0.00	0.00	0.14	0.14	0.00	0.00	0.00	0.48	0.13	0.13
Crit Moves:	****			****						****		
Green/Cycle:	0.26	0.41	0.00	0.00	0.15	0.15	0.00	0.00	0.00	0.45	0.45	0.45
Volume/Cap:	1.07	0.16	0.00	0.00	0.93	0.93	0.00	0.00	0.00	1.07	0.28	0.28
Delay/Veh:	104.5	22.2	0.0	0.0	73.2	73.2	0.0	0.0	0.0	82.5	20.5	20.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	104.5	22.2	0.0	0.0	73.2	73.2	0.0	0.0	0.0	82.5	20.5	20.5
LOS by Move:	F	C	A	A	E	E	A	A	A	F	C	C
HCM2kAvgQ:	27	3	0	0	13	13	0	0	0	40	5	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "B"
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 85 Critical Vol./Cap.(X): 0.735
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 28.3
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	0	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	504	182	0	0	273	69	0	0	0	297	13	89
Added Vol:	28	0	0	0	0	0	0	0	0	35	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	532	182	0	0	273	69	0	0	0	332	13	89
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.82	0.82	0.82	0.83	0.83	0.83	1.00	1.00	1.00	0.89	0.89	0.89
PHF Volume:	648	222	0	0	328	83	0	0	0	371	15	99
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	648	222	0	0	328	83	0	0	0	371	15	99
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	648	222	0	0	328	83	0	0	0	371	15	99

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.81	0.81	0.81
Lanes:	1.00	2.00	0.00	0.00	1.59	0.41	0.00	0.00	0.00	2.00	0.13	0.87
Final Sat.:	1805	3610	0	0	2792	709	0	0	0	3096	198	1350

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.36	0.06	0.00	0.00	0.12	0.12	0.00	0.00	0.00	0.12	0.07	0.07
Crit Moves:	****			****			****			****		
Green/Cycle:	0.45	0.66	0.00	0.00	0.21	0.21	0.00	0.00	0.00	0.15	0.15	0.15
Volume/Cap:	0.80	0.09	0.00	0.00	0.55	0.55	0.00	0.00	0.00	0.80	0.49	0.49
Delay/Veh:	25.7	5.2	0.0	0.0	30.8	30.8	0.0	0.0	0.0	42.2	33.5	33.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	25.7	5.2	0.0	0.0	30.8	30.8	0.0	0.0	0.0	42.2	33.5	33.5
LOS by Move:	C	A	A	A	C	C	A	A	A	D	C	C
HCM2kAvgQ:	17	1	0	0	6	6	0	0	0	7	4	4

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "B"
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

 Cycle (sec): 90 Critical Vol./Cap.(X): 0.805
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 32.9
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	0	1

Volume Module:

Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	378	213	0	0	350	103	0	0	0	588	41	123
Added Vol:	77	0	0	0	0	0	0	0	0	46	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	455	213	0	0	350	103	0	0	0	634	41	123
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.93	0.93	0.93	1.00	1.00	1.00	0.84	0.84	0.84
PHF Volume:	507	238	0	0	375	110	0	0	0	752	49	146
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	507	238	0	0	375	110	0	0	0	752	49	146
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	507	238	0	0	375	110	0	0	0	752	49	146

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.55	0.45	0.00	0.00	0.00	2.00	0.25	0.75
Final Sat.:	1805	3610	0	0	2695	793	0	0	0	3107	388	1165

Capacity Analysis Module:

Vol/Sat:	0.28	0.07	0.00	0.00	0.14	0.14	0.00	0.00	0.00	0.24	0.13	0.13
Crit Moves:	****				****					****		
Green/Cycle:	0.33	0.53	0.00	0.00	0.20	0.20	0.00	0.00	0.00	0.29	0.29	0.29
Volume/Cap:	0.84	0.12	0.00	0.00	0.70	0.70	0.00	0.00	0.00	0.84	0.44	0.44
Delay/Veh:	37.9	10.5	0.0	0.0	36.5	36.5	0.0	0.0	0.0	35.9	26.2	26.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	37.9	10.5	0.0	0.0	36.5	36.5	0.0	0.0	0.0	35.9	26.2	26.2
LOS by Move:	D	B	A	A	D	D	A	A	A	D	C	C
HCM2kAvgQ:	16	2	0	0	8	8	0	0	0	13	5	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "B"
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

Cycle (sec): 110 Critical Vol./Cap.(X): 0.894
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 38.0
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound											
Movement:	L	T	R	L	T	R	L	T	R	L	T	R									
Control:	Protected			Protected			Protected			Protected											
Rights:	Include			Include			Include			Include											
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0									
Lanes:	0	0	1	1	0	0	1	0	2	0	0	0	1	0	1	0	0	0	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	0	600	540	116	480	0	61	1	383	0	0	0
Added Vol:	0	28	16	0	35	0	0	0	66	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	628	556	116	515	0	61	1	449	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.96	0.96	0.96	0.90	0.90	0.90	1.00	1.00	1.00
PHF Volume:	0	663	587	121	539	0	67	1	499	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	663	587	121	539	0	67	1	499	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	663	587	121	539	0	67	1	499	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.06	0.94	1.00	2.00	0.00	0.98	0.02	1.00	0.00	0.00	0.00
Final Sat.:	0	1780	1577	1805	3610	0	1512	27	1539	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.00	0.37	0.37	0.07	0.15	0.00	0.04	0.04	0.32	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.00	0.41	0.41	0.09	0.50	0.00	0.36	0.36	0.36	0.00	0.00	0.00
Volume/Cap:	0.00	0.91	0.91	0.74	0.30	0.00	0.13	0.13	0.91	0.00	0.00	0.00
Delay/Veh:	0.0	40.1	40.1	65.0	16.3	0.0	23.9	23.9	51.6	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	40.1	40.1	65.0	16.3	0.0	23.9	23.9	51.6	0.0	0.0	0.0
LOS by Move:	A	D	D	E	B	A	C	C	D	A	A	A
HCM2kAvgQ:	0	25	25	6	6	0	2	2	21	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Proposed Action "B"
 Evening Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 0.981
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 54.2
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound										
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Protected			Protected			Protected			Protected										
Rights:	Include			Include			Include			Include										
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0								
Lanes:	0	0	1	1	0	1	0	2	0	0	0	1	0	1	0	0	0	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	458	321	105	692	0	88	3	558	0	0	0
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	0	496	348	114	749	0	95	3	604	0	0	0
Added Vol:	0	77	41	0	46	0	0	0	88	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	573	389	114	795	0	95	3	692	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.91	0.91	0.91	0.93	0.93	0.93	1.00	1.00	1.00
PHF Volume:	0	603	409	125	878	0	102	3	743	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	603	409	125	878	0	102	3	743	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	603	409	125	878	0	102	3	743	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.89	0.89	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.19	0.81	1.00	2.00	0.00	0.97	0.03	1.00	0.00	0.00	0.00
Final Sat.:	0	2020	1370	1805	3610	0	1490	51	1540	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.00	0.30	0.30	0.07	0.24	0.00	0.07	0.07	0.48	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.00	0.30	0.30	0.08	0.38	0.00	0.48	0.48	0.48	0.00	0.00	0.00
Volume/Cap:	0.00	1.00	1.00	0.83	0.64	0.00	0.14	0.14	1.00	0.00	0.00	0.00
Delay/Veh:	0.0	69.4	69.4	85.4	31.2	0.0	17.2	17.2	60.9	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	69.4	69.4	85.4	31.2	0.0	17.2	17.2	60.9	0.0	0.0	0.0
LOS by Move:	A	E	E	F	C	A	B	B	E	A	A	A
HCM2kAvgQ:	0	26	26	7	14	0	2	2	36	0	0	0

Note: Queue reported is the number of cars per lane.

Opening Year (2010) With Project – Alternative 1

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 1
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): 5.0 Worst Case Level Of Service: F [85.7]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Ignore			Ignore			Include		
Lanes:	1	0	0	0	0	1	1	0	0	0	0	0

Volume Module:

Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	335	190	0	0	693	41	28	0	208	0	0	0
Added Vol:	0	21	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	335	211	0	0	693	41	28	0	208	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.93	0.93	0.00	0.91	0.91	0.00	1.00	1.00	1.00
PHF Volume:	352	221	0	0	747	0	30	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	352	221	0	0	747	0	30	0	0	0	0	0

Critical Gap Module:

Critical Gp:	4.1	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	6.4	XXXX	6.2	XXXXXX	XXXX	XXXXXX
FollowUpTim:	2.2	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	3.5	XXXX	3.3	XXXXXX	XXXX	XXXXXX

Capacity Module:

Cnflict Vol:	747	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	1671	XXXX	747	XXXX	XXXX	XXXXXX
Potent Cap.:	871	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	107	XXXX	416	XXXX	XXXX	XXXXXX
Move Cap.:	871	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	73	XXXX	416	XXXX	XXXX	XXXXXX
Volume/Cap:	0.40	XXXX	XXXX	XXXX	XXXX	XXXX	0.42	XXXX	0.00	XXXX	XXXX	XXXX

Level Of Service Module:

2Way95thQ:	2.0	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	1.6	XXXX	XXXXXX	XXXX	XXXX	XXXXXX
Control Del:	11.9	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	85.7	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
LOS by Move:	B	*	*	*	*	*	F	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	XXXX	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	XXXX	XXXX	XXXXXX
SharedQueue:	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
Shrd ConDel:	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	XXXXXX			XXXXXX			85.7			XXXXXX		
ApproachLOS:	*			*			F			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 1
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): 2.7 Worst Case Level Of Service: F[62.8]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Ignore			Ignore			Include		
Lanes:	1	0	0	0	0	1	1	0	0	0	0	0

Volume Module:

Base Vol:	200	183	0	0	782	14	12	0	233	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	222	203	0	0	869	16	13	0	259	0	0	0
Added Vol:	0	59	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	222	262	0	0	869	16	13	0	259	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.89	0.89	0.89	0.97	0.97	0.00	0.86	0.86	0.00	1.00	1.00	1.00
PHF Volume:	250	296	0	0	895	0	15	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	250	296	0	0	895	0	15	0	0	0	0	0

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	xxxx	6.2	xxxxxx	xxxx	xxxxxx
FollowUpTim:	2.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	xxxx	3.3	xxxxxx	xxxx	xxxxxx

Capacity Module:

Cnflct Vol:	895	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	1691	xxxx	895	xxxx	xxxx	xxxxxx
Potent Cap.:	767	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	104	xxxx	342	xxxx	xxxx	xxxxxx
Move Cap.:	767	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	77	xxxx	342	xxxx	xxxx	xxxxxx
Volume/Cap:	0.33	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	0.20	xxxx	0.00	xxxx	xxxx	xxxxxx

Level Of Service Module:

2Way95thQ:	1.4	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	0.7	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Control Del:	12.0	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	62.8	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	B	*	*	*	*	*	F	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			62.8			xxxxxx		
ApproachLOS:	*			*			F			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 1
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Cycle (sec): 70 Critical Vol./Cap.(X): 0.729
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 16.8
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	0	24	24	0	0	0	0	0	0
Lanes:	1	0	1	0	0	1	0	0	1	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	335	190	0	0	693	41	28	0	208	0	0	0
Added Vol:	0	21	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	335	211	0	0	693	41	28	0	208	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.93	0.93	0.00	0.91	0.91	0.00	1.00	1.00	1.00
PHF Volume:	352	221	0	0	747	0	30	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	352	221	0	0	747	0	30	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	352	221	0	0	747	0	30	0	0	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	1900	1900	1900	0	1900	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.19	0.12	0.00	0.00	0.39	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.27	0.81	0.00	0.00	0.54	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.73	0.14	0.00	0.00	0.73	0.00	0.30	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	28.9	1.5	0.0	0.0	14.9	0.0	35.3	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	28.9	1.5	0.0	0.0	14.9	0.0	35.3	0.0	0.0	0.0	0.0	0.0
LOS by Move:	C	A	A	A	B	A	D	A	A	A	A	A
HCM2kAvgQ:	9	1	0	0	14	0	1	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 1
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

 Cycle (sec): 85 Critical Vol./Cap.(X): 0.719
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 14.6
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	0	24	24	0	0	0	0	0	0
Lanes:	1	0	1	0	0	1	1	0	0	0	0	0

Volume Module:

Base Vol:	200	183	0	0	782	14	12	0	233	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	222	203	0	0	869	16	13	0	259	0	0	0
Added Vol:	0	59	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	222	262	0	0	869	16	13	0	259	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.89	0.89	0.89	0.97	0.97	0.00	0.86	0.86	0.00	1.00	1.00	1.00
PHF Volume:	250	296	0	0	895	0	15	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	250	296	0	0	895	0	15	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	250	296	0	0	895	0	15	0	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	1900	1900	1900	0	1900	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.14	0.16	0.00	0.00	0.47	0.00	0.01	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.19	0.85	0.00	0.00	0.65	0.00	0.01	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.72	0.18	0.00	0.00	0.72	0.00	0.18	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	39.3	1.2	0.0	0.0	11.7	0.0	42.7	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	39.3	1.2	0.0	0.0	11.7	0.0	42.7	0.0	0.0	0.0	0.0	0.0
LOS by Move:	D	A	A	A	B	A	D	A	A	A	A	A
HCM2kAvgQ:	8	1	0	0	16	0	1	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Opening Year (2010) With Project - Alternative 1
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 37.8 Worst Case Level Of Service: F[252.9]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module:

Table with 12 columns representing different traffic metrics and 12 rows of data.

Critical Gap Module:

Table with 12 columns and 2 rows of data for Critical Gap and FollowUpTim.

Capacity Module:

Table with 12 columns and 4 rows of data for Capacity metrics.

Level Of Service Module:

Table with 12 columns and 10 rows of data for Level Of Service metrics.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 1
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 38.2 Worst Case Level Of Service: F[234.3]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Ignore		
Lanes:	0	0	0	0	0	1	0	0	0	0	0	1

Volume Module:

Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	159	0	40	24	1120	0	0	378	651
Added Vol:	0	0	0	67	0	0	0	0	0	0	59	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	226	0	40	24	1120	0	0	437	651
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.83	0.83	0.83	0.96	0.96	0.96	0.95	0.95	0.00
PHF Volume:	0	0	0	272	0	48	25	1165	0	0	462	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	0	272	0	48	25	1165	0	0	462	0

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	4.1	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
FollowUpTim:	xxxxxx	xxxx	xxxxxx	3.5	4.0	3.3	2.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx

Capacity Module:

Cnflict Vol:	xxxx	xxxx	xxxxxx	1096	1678	231	462	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Potent Cap.:	xxxx	xxxx	xxxxxx	211	96	777	1110	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Move Cap.:	xxxx	xxxx	xxxxxx	207	94	777	1110	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Volume/Cap:	xxxx	xxxx	xxxx	1.31	0.00	0.06	0.02	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	0.1	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	8.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	*	*	*	*	*	*	A	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	233	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	17.7	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	234	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	F	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			234.3			xxxxxx			xxxxxx		
ApproachLOS:	*			F			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 1
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.497
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 10.9
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	0	0	1	1	0	2	0	0	2

Volume Module:

Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	154	0	23	92	819	0	0	493	781
Added Vol:	0	0	0	50	0	0	0	0	0	0	21	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	204	0	23	92	819	0	0	514	781
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.87	0.87	0.87	0.94	0.94	0.94	0.94	0.94	0.00
PHF Volume:	0	0	0	235	0	27	98	871	0	0	545	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	235	0	27	98	871	0	0	545	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	235	0	27	98	871	0	0	545	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.73	1.00	0.73	0.42	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	0.90	0.00	0.10	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	1238	0	141	790	3610	0	0	3610	1900

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.19	0.00	0.19	0.12	0.24	0.00	0.00	0.15	0.00	
Crit Moves:				****				****					
Green/Cycle:	0.00	0.00	0.00	0.38	0.00	0.38	0.49	0.49	0.00	0.00	0.49	0.00	
Volume/Cap:	0.00	0.00	0.00	0.50	0.00	0.50	0.26	0.50	0.00	0.00	0.31	0.00	
Delay/Veh:	0.0	0.0	0.0	14.9	0.0	14.9	9.4	10.7	0.0	0.0	9.5	0.0	
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
AdjDel/Veh:	0.0	0.0	0.0	14.9	0.0	14.9	9.4	10.7	0.0	0.0	9.5	0.0	
LOS by Move:	A	A	A	B	A	B	A	B	A	A	A	A	
HCM2kAvgQ:	0	0	0	4	0	4	1	6	0	0	3	0	

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 1
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.639
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 12.0
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	0	0	1	1	0	2	0	0	2

Volume Module:

Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	159	0	40	24	1120	0	0	378	651
Added Vol:	0	0	0	67	0	0	0	0	0	0	59	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	226	0	40	24	1120	0	0	437	651
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.83	0.83	0.83	0.96	0.96	0.96	0.95	0.95	0.00
PHF Volume:	0	0	0	272	0	48	25	1165	0	0	462	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	272	0	48	25	1165	0	0	462	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	272	0	48	25	1165	0	0	462	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.73	1.00	0.73	0.47	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	0.85	0.00	0.15	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	1180	0	209	897	3610	0	0	3610	1900

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.23	0.00	0.23	0.03	0.32	0.00	0.00	0.13	0.00
Crit Moves:				****				****				
Green/Cycle:	0.00	0.00	0.00	0.36	0.00	0.36	0.51	0.51	0.00	0.00	0.51	0.00
Volume/Cap:	0.00	0.00	0.00	0.64	0.00	0.64	0.06	0.64	0.00	0.00	0.25	0.00
Delay/Veh:	0.0	0.0	0.0	18.7	0.0	18.7	7.6	11.6	0.0	0.0	8.5	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	18.7	0.0	18.7	7.6	11.6	0.0	0.0	8.5	0.0
LOS by Move:	A	A	A	B	A	B	A	B	A	A	A	A
HCM2kAvgQ:	0	0	0	6	0	6	0	9	0	0	3	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 1
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 105 Critical Vol./Cap.(X): 0.832
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 38.5
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

-----|-----|-----|-----|

Volume Module:

Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	504	182	0	0	273	69	0	0	0	297	13	89
Added Vol:	21	0	0	0	0	0	0	0	0	26	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	525	182	0	0	273	69	0	0	0	323	13	89
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.82	0.82	0.82	0.83	0.83	0.83	1.00	1.00	1.00	0.89	0.89	0.89
PHF Volume:	640	222	0	0	328	83	0	0	0	361	15	99
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	640	222	0	0	328	83	0	0	0	361	15	99
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	640	222	0	0	328	83	0	0	0	361	15	99

-----|-----|-----|-----|

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.59	0.41	0.00	0.00	0.00	1.00	0.13	0.87
Final Sat.:	1805	3610	0	0	2792	709	0	0	0	1550	198	1352

-----|-----|-----|-----|

Capacity Analysis Module:

Vol/Sat:	0.35	0.06	0.00	0.00	0.12	0.12	0.00	0.00	0.00	0.23	0.07	0.07
Crit Moves:	****				****					****		
Green/Cycle:	0.41	0.58	0.00	0.00	0.17	0.17	0.00	0.00	0.00	0.27	0.27	0.27
Volume/Cap:	0.87	0.11	0.00	0.00	0.69	0.69	0.00	0.00	0.00	0.87	0.27	0.27
Delay/Veh:	39.3	9.9	0.0	0.0	44.1	44.1	0.0	0.0	0.0	50.6	30.4	30.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	39.3	9.9	0.0	0.0	44.1	44.1	0.0	0.0	0.0	50.6	30.4	30.4
LOS by Move:	D	A	A	A	D	D	A	A	A	D	C	C
HCM2kAvgQ:	22	2	0	0	8	8	0	0	0	15	3	3

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 1
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.021
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 68.3
 Optimal Cycle: OPTIMIZED Level Of Service: E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	0	0	0	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	378	213	0	0	350	103	0	0	0	588	41	123
Added Vol:	59	0	0	0	0	0	0	0	0	35	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	437	213	0	0	350	103	0	0	0	623	41	123
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.93	0.93	0.93	1.00	1.00	1.00	0.84	0.84	0.84
PHF Volume:	487	238	0	0	375	110	0	0	0	739	49	146
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	487	238	0	0	375	110	0	0	0	739	49	146
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	487	238	0	0	375	110	0	0	0	739	49	146

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.55	0.45	0.00	0.00	0.00	1.00	0.25	0.75
Final Sat.:	1805	3610	0	0	2695	793	0	0	0	1552	388	1164

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.27	0.07	0.00	0.00	0.14	0.14	0.00	0.00	0.00	0.48	0.13	0.13
Crit Moves:	****			****			****			****		
Green/Cycle:	0.26	0.41	0.00	0.00	0.15	0.15	0.00	0.00	0.00	0.46	0.46	0.46
Volume/Cap:	1.04	0.16	0.00	0.00	0.93	0.93	0.00	0.00	0.00	1.04	0.27	0.27
Delay/Veh:	97.0	22.5	0.0	0.0	73.2	73.2	0.0	0.0	0.0	73.6	20.2	20.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	97.0	22.5	0.0	0.0	73.2	73.2	0.0	0.0	0.0	73.6	20.2	20.2
LOS by Move:	F	C	A	A	E	E	A	A	A	E	C	C
HCM2kAvgQ:	25	3	0	0	13	13	0	0	0	38	5	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 1
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

 Cycle (sec): 85 Critical Vol./Cap.(X): 0.725
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 27.8
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	0	1

Volume Module:

Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	504	182	0	0	273	69	0	0	0	297	13	89
Added Vol:	21	0	0	0	0	0	0	0	0	26	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	525	182	0	0	273	69	0	0	0	323	13	89
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.82	0.82	0.82	0.83	0.83	0.83	1.00	1.00	1.00	0.89	0.89	0.89
PHF Volume:	640	222	0	0	328	83	0	0	0	361	15	99
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	640	222	0	0	328	83	0	0	0	361	15	99
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	640	222	0	0	328	83	0	0	0	361	15	99

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.59	0.41	0.00	0.00	0.00	2.00	0.13	0.87
Final Sat.:	1805	3610	0	0	2792	709	0	0	0	3099	198	1352

Capacity Analysis Module:

Vol/Sat:	0.35	0.06	0.00	0.00	0.12	0.12	0.00	0.00	0.00	0.12	0.07	0.07
Crit Moves:	****			****						****		
Green/Cycle:	0.45	0.66	0.00	0.00	0.21	0.21	0.00	0.00	0.00	0.15	0.15	0.15
Volume/Cap:	0.78	0.09	0.00	0.00	0.55	0.55	0.00	0.00	0.00	0.78	0.50	0.50
Delay/Veh:	24.9	5.2	0.0	0.0	30.8	30.8	0.0	0.0	0.0	41.6	33.7	33.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	24.9	5.2	0.0	0.0	30.8	30.8	0.0	0.0	0.0	41.6	33.7	33.7
LOS by Move:	C	A	A	A	C	C	A	A	A	D	C	C
HCM2kAvgQ:	16	1	0	0	6	6	0	0	0	7	4	4

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 1
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 85 Critical Vol./Cap.(X): 0.797
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 31.6
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	378	213	0	0	350	103	0	0	0	588	41	123
Added Vol:	59	0	0	0	0	0	0	0	0	35	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	437	213	0	0	350	103	0	0	0	623	41	123
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.93	0.93	0.93	1.00	1.00	1.00	0.84	0.84	0.84
PHF Volume:	487	238	0	0	375	110	0	0	0	739	49	146
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	487	238	0	0	375	110	0	0	0	739	49	146
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	487	238	0	0	375	110	0	0	0	739	49	146

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.55	0.45	0.00	0.00	0.00	2.00	0.25	0.75
Final Sat.:	1805	3610	0	0	2695	793	0	0	0	3104	388	1164

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.27	0.07	0.00	0.00	0.14	0.14	0.00	0.00	0.00	0.24	0.13	0.13
Crit Moves:	****			****			****			****		
Green/Cycle:	0.32	0.53	0.00	0.00	0.21	0.21	0.00	0.00	0.00	0.28	0.28	0.28
Volume/Cap:	0.85	0.12	0.00	0.00	0.66	0.66	0.00	0.00	0.00	0.85	0.45	0.45
Delay/Veh:	38.2	10.1	0.0	0.0	32.8	32.8	0.0	0.0	0.0	35.0	25.3	25.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	38.2	10.1	0.0	0.0	32.8	32.8	0.0	0.0	0.0	35.0	25.3	25.3
LOS by Move:	D	B	A	A	C	C	A	A	A	C	C	C
HCM2kAvgQ:	15	2	0	0	7	7	0	0	0	12	5	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 1
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

Cycle (sec): 105 Critical Vol./Cap.(X): 0.883
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 36.1
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0
Lanes:	0	0	1	1	0	0	1	0	2	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	0	600	540	116	480	0	61	1	383	0	0	0
Added Vol:	0	21	12	0	26	0	0	0	50	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	621	552	116	506	0	61	1	433	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.96	0.96	0.96	0.90	0.90	0.90	1.00	1.00	1.00
PHF Volume:	0	656	583	121	530	0	67	1	481	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	656	583	121	530	0	67	1	481	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	656	583	121	530	0	67	1	481	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.06	0.94	1.00	2.00	0.00	0.98	0.02	1.00	0.00	0.00	0.00
Final Sat.:	0	1775	1579	1805	3610	0	1513	27	1540	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.00	0.37	0.37	0.07	0.15	0.00	0.04	0.04	0.31	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.00	0.41	0.41	0.10	0.50	0.00	0.34	0.34	0.34	0.00	0.00	0.00
Volume/Cap:	0.00	0.91	0.91	0.71	0.29	0.00	0.13	0.13	0.91	0.00	0.00	0.00
Delay/Veh:	0.0	38.1	38.1	58.6	15.3	0.0	23.6	23.6	50.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	38.1	38.1	58.6	15.3	0.0	23.6	23.6	50.0	0.0	0.0	0.0
LOS by Move:	A	D	D	E	B	A	C	C	D	A	A	A
HCM2kAvgQ:	0	24	24	5	5	0	2	2	19	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 1
 Evening Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 0.955
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 49.5
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound										
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Protected			Protected			Protected			Protected										
Rights:	Include			Include			Include			Include										
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0								
Lanes:	0	0	1	1	0	1	0	2	0	0	0	1	0	1	0	0	0	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	458	321	105	692	0	88	3	558	0	0	0
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	0	496	348	114	749	0	95	3	604	0	0	0
Added Vol:	0	59	31	0	35	0	0	0	67	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	555	379	114	784	0	95	3	671	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.91	0.91	0.91	0.93	0.93	0.93	1.00	1.00	1.00
PHF Volume:	0	584	399	125	866	0	102	3	720	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	584	399	125	866	0	102	3	720	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	584	399	125	866	0	102	3	720	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.89	0.89	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.19	0.81	1.00	2.00	0.00	0.97	0.03	1.00	0.00	0.00	0.00
Final Sat.:	0	2015	1375	1805	3610	0	1488	51	1539	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.00	0.29	0.29	0.07	0.24	0.00	0.07	0.07	0.47	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.00	0.30	0.30	0.08	0.38	0.00	0.48	0.48	0.48	0.00	0.00	0.00
Volume/Cap:	0.00	0.97	0.97	0.83	0.63	0.00	0.14	0.14	0.97	0.00	0.00	0.00
Delay/Veh:	0.0	62.2	62.2	85.4	31.0	0.0	17.2	17.2	53.2	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	62.2	62.2	85.4	31.0	0.0	17.2	17.2	53.2	0.0	0.0	0.0
LOS by Move:	A	E	E	F	C	A	B	B	D	A	A	A
HCM2kAvgQ:	0	24	24	7	14	0	2	2	33	0	0	0

 Note: Queue reported is the number of cars per lane.

Opening Year (2010) With Project – Alternative 2

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 2
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): 5.0 Worst Case Level Of Service: F [83.7]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Ignore			Ignore			Include		
Lanes:	1	0	0	0	0	1	1	0	0	0	0	0

Volume Module:

Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	335	190	0	0	693	41	28	0	208	0	0	0
Added Vol:	0	10	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	335	200	0	0	693	41	28	0	208	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.93	0.93	0.00	0.91	0.91	0.00	1.00	1.00	1.00
PHF Volume:	352	210	0	0	747	0	30	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	352	210	0	0	747	0	30	0	0	0	0	0

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	xxxx	6.2	xxxxxx	xxxx	xxxxxx
FollowUpTim:	2.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	xxxx	3.3	xxxxxx	xxxx	xxxxxx

Capacity Module:

Cnflict Vol:	747	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	1660	xxxx	747	xxxx	xxxx	xxxxxx
Potent Cap.:	871	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	108	xxxx	416	xxxx	xxxx	xxxxxx
Move Cap.:	871	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	74	xxxx	416	xxxx	xxxx	xxxxxx
Volume/Cap:	0.40	xxxx	xxxx	xxxx	xxxx	xxxx	0.41	xxxx	0.00	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	2.0	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	1.6	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	11.9	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	83.7	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	B	*	*	*	*	*	F	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			83.7			xxxxxx		
ApproachLOS:	*			*			F			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 2
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): 2.8 Worst Case Level Of Service: F[58.5]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Ignore			Ignore			Include		
Lanes:	1	0	0	0	0	1	1	0	0	0	0	0

Volume Module:

Base Vol:	200	183	0	0	782	14	12	0	233	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	222	203	0	0	869	16	13	0	259	0	0	0
Added Vol:	0	20	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	222	223	0	0	869	16	13	0	259	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.89	0.89	0.89	0.97	0.97	0.00	0.86	0.86	0.00	1.00	1.00	1.00
PHF Volume:	250	252	0	0	895	0	15	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	250	252	0	0	895	0	15	0	0	0	0	0

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	xxxx	6.2	xxxxxx	xxxx	xxxxxx
FollowUpTim:	2.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	xxxx	3.3	xxxxxx	xxxx	xxxxxx

Capacity Module:

Cnflct Vol:	895	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	1647	xxxx	895	xxxx	xxxx	xxxxxx
Potent Cap.:	767	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	110	xxxx	342	xxxx	xxxx	xxxxxx
Move Cap.:	767	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	82	xxxx	342	xxxx	xxxx	xxxxxx
Volume/Cap:	0.33	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	0.19	xxxx	0.00	xxxx	xxxx	xxxxxx

Level Of Service Module:

2Way95thQ:	1.4	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	0.6	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	12.0	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	58.5	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	B	*	*	*	*	*	F	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			58.5			xxxxxx		
ApproachLOS:	*			*			F			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 2
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Cycle (sec): 65 Critical Vol./Cap.(X): 0.741
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 16.6
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	0	24	24	0	0	0	0	0	0
Lanes:	1	0	1	0	0	1	1	0	0	0	0	0

Volume Module:

Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	335	190	0	0	693	41	28	0	208	0	0	0
Added Vol:	0	10	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	335	200	0	0	693	41	28	0	208	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.93	0.93	0.00	0.91	0.91	0.00	1.00	1.00	1.00
PHF Volume:	352	210	0	0	747	0	30	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	352	210	0	0	747	0	30	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	352	210	0	0	747	0	30	0	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	1900	1900	1900	0	1900	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.19	0.11	0.00	0.00	0.39	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.26	0.79	0.00	0.00	0.53	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.74	0.14	0.00	0.00	0.74	0.00	0.27	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	28.1	1.6	0.0	0.0	14.8	0.0	32.6	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	28.1	1.6	0.0	0.0	14.8	0.0	32.6	0.0	0.0	0.0	0.0	0.0
LOS by Move:	C	A	A	A	B	A	C	A	A	A	A	A
HCM2kAvgQ:	8	1	0	0	13	0	1	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 2
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.735
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 14.5
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	0	24	24	0	0	0	0	0	0
Lanes:	1	0	1	0	0	1	1	0	0	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	200	183	0	0	782	14	12	0	233	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	222	203	0	0	869	16	13	0	259	0	0	0
Added Vol:	0	20	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	222	223	0	0	869	16	13	0	259	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.89	0.89	0.89	0.97	0.97	0.00	0.86	0.86	0.00	1.00	1.00	1.00
PHF Volume:	250	252	0	0	895	0	15	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	250	252	0	0	895	0	15	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	250	252	0	0	895	0	15	0	0	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	1900	1900	1900	0	1900	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.14	0.13	0.00	0.00	0.47	0.00	0.01	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.19	0.83	0.00	0.00	0.64	0.00	0.01	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.74	0.16	0.00	0.00	0.74	0.00	0.16	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	36.8	1.3	0.0	0.0	11.5	0.0	37.5	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	36.8	1.3	0.0	0.0	11.5	0.0	37.5	0.0	0.0	0.0	0.0	0.0
LOS by Move:	D	A	A	A	B	A	D	A	A	A	A	A
HCM2kAvgQ:	7	1	0	0	15	0	1	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 2
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 23.0 Worst Case Level Of Service: F[170.7]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Ignore		
Lanes:	0	0	0	0	0	1	0	2	0	0	2	0

Volume Module:

Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	154	0	23	92	819	0	0	493	781
Added Vol:	0	0	0	21	0	0	0	0	0	0	10	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	175	0	23	92	819	0	0	503	781
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.87	0.87	0.87	0.94	0.94	0.94	0.94	0.94	0.00
PHF Volume:	0	0	0	202	0	27	98	871	0	0	534	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	0	202	0	27	98	871	0	0	534	0

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflict Vol:	xxxx	xxxx	xxxxx	1165	1601	267	534	xxxx	xxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	xxxx	xxxx	xxxxx	190	107	737	1044	xxxx	xxxxx	xxxx	xxxx	xxxxx
Move Cap.:	xxxx	xxxx	xxxxx	177	97	737	1044	xxxx	xxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	xxxx	xxxx	xxxx	1.14	0.00	0.04	0.09	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.3	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	8.8	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	A	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	194	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	11.7	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	171	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	F	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			170.7			xxxxxx			xxxxxx		
ApproachLOS:	*			F			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 2
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 13.9 Worst Case Level Of Service: F[100.1]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Ignore		
Lanes:	0	0	0	0	0	1	0	2	0	0	2	1

Volume Module:

Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	159	0	40	24	1120	0	0	378	651
Added Vol:	0	0	0	14	0	0	0	0	0	0	20	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	173	0	40	24	1120	0	0	398	651
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.83	0.83	0.83	0.96	0.96	0.96	0.95	0.95	0.00
PHF Volume:	0	0	0	209	0	48	25	1165	0	0	421	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	0	209	0	48	25	1165	0	0	421	0

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflict Vol:	xxxx	xxxx	xxxxx	1054	1637	210	421	xxxx	xxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	xxxx	xxxx	xxxxx	224	102	801	1149	xxxx	xxxxx	xxxx	xxxx	xxxxx
Move Cap.:	xxxx	xxxx	xxxxx	221	99	801	1149	xxxx	xxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	xxxx	xxxx	xxxx	0.95	0.00	0.06	0.02	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.1	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	8.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	A	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	255	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	9.9	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	100	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	F	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			100.1			xxxxxx			xxxxxx		
ApproachLOS:	*			F			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 2
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.469
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 9.9
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	0	0	1	1	0	2	0	0	2

-----|-----|-----|-----|

Volume Module:

Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	154	0	23	92	819	0	0	493	781
Added Vol:	0	0	0	21	0	0	0	0	0	0	10	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	175	0	23	92	819	0	0	503	781
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.87	0.87	0.87	0.94	0.94	0.94	0.94	0.94	0.00
PHF Volume:	0	0	0	202	0	27	98	871	0	0	534	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	202	0	27	98	871	0	0	534	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	202	0	27	98	871	0	0	534	0

-----|-----|-----|-----|

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.73	1.00	0.73	0.43	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	0.88	0.00	0.12	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	1219	0	162	815	3610	0	0	3610	1900

-----|-----|-----|-----|

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.17	0.00	0.17	0.12	0.24	0.00	0.00	0.15	0.00	
Crit Moves:				****				****					
Green/Cycle:	0.00	0.00	0.00	0.35	0.00	0.35	0.51	0.51	0.00	0.00	0.51	0.00	
Volume/Cap:	0.00	0.00	0.00	0.47	0.00	0.47	0.23	0.47	0.00	0.00	0.29	0.00	
Delay/Veh:	0.0	0.0	0.0	15.8	0.0	15.8	8.3	9.5	0.0	0.0	8.4	0.0	
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
AdjDel/Veh:	0.0	0.0	0.0	15.8	0.0	15.8	8.3	9.5	0.0	0.0	8.4	0.0	
LOS by Move:	A	A	A	B	A	B	A	A	A	A	A	A	
HCM2kAvgQ:	0	0	0	4	0	4	1	6	0	0	3	0	

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 2
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.584
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 10.1
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	0	0	1	1	0	2	0	0	1

Volume Module:

Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	159	0	40	24	1120	0	0	378	651
Added Vol:	0	0	0	14	0	0	0	0	0	0	20	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	173	0	40	24	1120	0	0	398	651
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.83	0.83	0.83	0.96	0.96	0.96	0.95	0.95	0.00
PHF Volume:	0	0	0	209	0	48	25	1165	0	0	421	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	209	0	48	25	1165	0	0	421	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	209	0	48	25	1165	0	0	421	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.74	1.00	0.74	0.50	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	0.81	0.00	0.19	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	1136	0	263	958	3610	0	0	3610	1900

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.18	0.00	0.18	0.03	0.32	0.00	0.00	0.12	0.00
Crit Moves:				****				****				
Green/Cycle:	0.00	0.00	0.00	0.31	0.00	0.31	0.55	0.55	0.00	0.00	0.55	0.00
Volume/Cap:	0.00	0.00	0.00	0.58	0.00	0.58	0.05	0.58	0.00	0.00	0.21	0.00
Delay/Veh:	0.0	0.0	0.0	19.3	0.0	19.3	6.2	9.3	0.0	0.0	6.9	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	19.3	0.0	19.3	6.2	9.3	0.0	0.0	6.9	0.0
LOS by Move:	A	A	A	B	A	B	A	A	A	A	A	A
HCM2kAvgQ:	0	0	0	5	0	5	0	8	0	0	2	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 2
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.818
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 36.6
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	504	182	0	0	273	69	0	0	0	297	13	89
Added Vol:	10	0	0	0	0	0	0	0	0	12	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	514	182	0	0	273	69	0	0	0	309	13	89
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.82	0.82	0.82	0.83	0.83	0.83	1.00	1.00	1.00	0.89	0.89	0.89
PHF Volume:	626	222	0	0	328	83	0	0	0	346	15	99
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	626	222	0	0	328	83	0	0	0	346	15	99
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	626	222	0	0	328	83	0	0	0	346	15	99

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.59	0.41	0.00	0.00	0.00	1.00	0.13	0.87
Final Sat.:	1805	3610	0	0	2792	709	0	0	0	1550	198	1352

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.35	0.06	0.00	0.00	0.12	0.12	0.00	0.00	0.00	0.22	0.07	0.07
Crit Moves:	****			****			****			****		
Green/Cycle:	0.40	0.58	0.00	0.00	0.18	0.18	0.00	0.00	0.00	0.26	0.26	0.26
Volume/Cap:	0.86	0.11	0.00	0.00	0.65	0.65	0.00	0.00	0.00	0.86	0.28	0.28
Delay/Veh:	37.9	9.3	0.0	0.0	40.5	40.5	0.0	0.0	0.0	49.1	29.8	29.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	37.9	9.3	0.0	0.0	40.5	40.5	0.0	0.0	0.0	49.1	29.8	29.8
LOS by Move:	D	A	A	A	D	D	A	A	A	D	C	C
HCM2kAvgQ:	21	2	0	0	7	7	0	0	0	14	3	3

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 2
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 115 Critical Vol./Cap.(X): 0.976
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 57.9
 Optimal Cycle: OPTIMIZED Level Of Service: E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

-----|-----|-----|-----|

Volume Module:

Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	378	213	0	0	350	103	0	0	0	588	41	123
Added Vol:	20	0	0	0	0	0	0	0	0	9	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	398	213	0	0	350	103	0	0	0	597	41	123
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.93	0.93	0.93	1.00	1.00	1.00	0.84	0.84	0.84
PHF Volume:	443	238	0	0	375	110	0	0	0	708	49	146
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	443	238	0	0	375	110	0	0	0	708	49	146
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	443	238	0	0	375	110	0	0	0	708	49	146

-----|-----|-----|-----|

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.55	0.45	0.00	0.00	0.00	1.00	0.25	0.75
Final Sat.:	1805	3610	0	0	2695	793	0	0	0	1554	388	1165

-----|-----|-----|-----|

Capacity Analysis Module:

Vol/Sat:	0.25	0.07	0.00	0.00	0.14	0.14	0.00	0.00	0.00	0.46	0.13	0.13
Crit Moves:	****			****						****		
Green/Cycle:	0.25	0.40	0.00	0.00	0.16	0.16	0.00	0.00	0.00	0.46	0.46	0.46
Volume/Cap:	1.00	0.16	0.00	0.00	0.89	0.89	0.00	0.00	0.00	1.00	0.27	0.27
Delay/Veh:	84.7	22.0	0.0	0.0	63.8	63.8	0.0	0.0	0.0	59.8	19.4	19.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	84.7	22.0	0.0	0.0	63.8	63.8	0.0	0.0	0.0	59.8	19.4	19.4
LOS by Move:	F	C	A	A	E	E	A	A	A	E	B	B
HCM2kAvgQ:	21	3	0	0	12	12	0	0	0	33	4	4

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 2
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 80 Critical Vol./Cap.(X): 0.720
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 26.9
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	0	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	504	182	0	0	273	69	0	0	0	297	13	89
Added Vol:	10	0	0	0	0	0	0	0	0	12	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	514	182	0	0	273	69	0	0	0	309	13	89
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.82	0.82	0.82	0.83	0.83	0.83	1.00	1.00	1.00	0.89	0.89	0.89
PHF Volume:	626	222	0	0	328	83	0	0	0	346	15	99
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	626	222	0	0	328	83	0	0	0	346	15	99
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	626	222	0	0	328	83	0	0	0	346	15	99

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.59	0.41	0.00	0.00	0.00	2.00	0.13	0.87
Final Sat.:	1805	3610	0	0	2792	709	0	0	0	3100	198	1352

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.35	0.06	0.00	0.00	0.12	0.12	0.00	0.00	0.00	0.11	0.07	0.07
Crit Moves:	****			****						****		
Green/Cycle:	0.44	0.66	0.00	0.00	0.23	0.23	0.00	0.00	0.00	0.14	0.14	0.14
Volume/Cap:	0.80	0.09	0.00	0.00	0.52	0.52	0.00	0.00	0.00	0.80	0.53	0.53
Delay/Veh:	25.3	4.9	0.0	0.0	27.9	27.9	0.0	0.0	0.0	41.0	32.5	32.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	25.3	4.9	0.0	0.0	27.9	27.9	0.0	0.0	0.0	41.0	32.5	32.5
LOS by Move:	C	A	A	A	C	C	A	A	A	D	C	C
HCM2kAvgQ:	15	1	0	0	5	5	0	0	0	7	4	4

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 2
 Evening Peak Hour - With Improvements

Level of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 80 Critical Vol./Cap.(X): 0.766
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 29.0
 Optimal Cycle: OPTIMIZED Level of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	0	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	378	213	0	0	350	103	0	0	0	588	41	123
Added Vol:	20	0	0	0	0	0	0	0	0	9	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	398	213	0	0	350	103	0	0	0	597	41	123
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.93	0.93	0.93	1.00	1.00	1.00	0.84	0.84	0.84
PHF Volume:	443	238	0	0	375	110	0	0	0	708	49	146
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	443	238	0	0	375	110	0	0	0	708	49	146
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	443	238	0	0	375	110	0	0	0	708	49	146

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.55	0.45	0.00	0.00	0.00	2.00	0.25	0.75
Final Sat.:	1805	3610	0	0	2695	793	0	0	0	3108	388	1165

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.25	0.07	0.00	0.00	0.14	0.14	0.00	0.00	0.00	0.23	0.13	0.13
Crit Moves:	****			****			****			****		
Green/Cycle:	0.30	0.52	0.00	0.00	0.23	0.23	0.00	0.00	0.00	0.28	0.28	0.28
Volume/Cap:	0.82	0.13	0.00	0.00	0.62	0.62	0.00	0.00	0.00	0.82	0.45	0.45
Delay/Veh:	36.0	9.8	0.0	0.0	29.4	29.4	0.0	0.0	0.0	32.2	24.1	24.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	36.0	9.8	0.0	0.0	29.4	29.4	0.0	0.0	0.0	32.2	24.1	24.1
LOS by Move:	D	A	A	A	C	C	A	A	A	C	C	C
HCM2kAvgQ:	13	2	0	0	7	7	0	0	0	11	5	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 2
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.861

Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 33.0

Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0
Lanes:	0	0	1	1	0	0	1	0	2	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	0	600	540	116	480	0	61	1	383	0	0	0
Added Vol:	0	10	7	0	12	0	0	0	21	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	610	547	116	492	0	61	1	404	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.96	0.96	0.96	0.90	0.90	0.90	1.00	1.00	1.00
PHF Volume:	0	644	578	121	515	0	67	1	449	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	644	578	121	515	0	67	1	449	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	644	578	121	515	0	67	1	449	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.05	0.95	1.00	2.00	0.00	0.98	0.02	1.00	0.00	0.00	0.00
Final Sat.:	0	1767	1586	1805	3610	0	1513	27	1541	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.00	0.36	0.36	0.07	0.14	0.00	0.04	0.04	0.29	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.00	0.41	0.41	0.10	0.51	0.00	0.33	0.33	0.33	0.00	0.00	0.00
Volume/Cap:	0.00	0.89	0.89	0.67	0.28	0.00	0.14	0.14	0.89	0.00	0.00	0.00
Delay/Veh:	0.0	34.5	34.5	52.9	14.0	0.0	23.6	23.6	46.9	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	34.5	34.5	52.9	14.0	0.0	23.6	23.6	46.9	0.0	0.0	0.0
LOS by Move:	A	C	C	D	B	A	A	C	C	D	A	A
HCM2kAvgQ:	0	22	22	5	5	0	2	2	17	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 2
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

Cycle (sec): 105 Critical Vol./Cap.(X): 0.911
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 40.8
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0
Lanes:	0	0	1	1	0	0	1	0	2	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	458	321	105	692	0	88	3	558	0	0	0
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	0	496	348	114	749	0	95	3	604	0	0	0
Added Vol:	0	20	12	0	9	0	0	0	14	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	516	360	114	758	0	95	3	618	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.91	0.91	0.91	0.93	0.93	0.93	1.00	1.00	1.00
PHF Volume:	0	543	379	125	837	0	102	3	663	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	543	379	125	837	0	102	3	663	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	543	379	125	837	0	102	3	663	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.89	0.89	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.18	0.82	1.00	2.00	0.00	0.97	0.03	1.00	0.00	0.00	0.00
Final Sat.:	0	1995	1391	1805	3610	0	1490	51	1541	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.00	0.27	0.27	0.07	0.23	0.00	0.07	0.07	0.43	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.00	0.29	0.29	0.10	0.39	0.00	0.46	0.46	0.46	0.00	0.00	0.00
Volume/Cap:	0.00	0.93	0.93	0.73	0.60	0.00	0.15	0.15	0.93	0.00	0.00	0.00
Delay/Veh:	0.0	51.5	51.5	60.9	26.4	0.0	16.4	16.4	44.2	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	51.5	51.5	60.9	26.4	0.0	16.4	16.4	44.2	0.0	0.0	0.0
LOS by Move:	A	D	D	E	C	A	B	B	D	A	A	A
HCM2kAvgQ:	0	20	20	6	12	0	2	2	26	0	0	0

Note: Queue reported is the number of cars per lane.

Opening Year (2010) With Project – Alternative 3

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 3
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): 5.0 Worst Case Level Of Service: F [84.2]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Ignore			Ignore			Include		
Lanes:	1	0	0	0	0	1	0	0	1	0	0	0

Volume Module:

Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	335	190	0	0	693	41	28	0	208	0	0	0
Added Vol:	0	13	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	335	203	0	0	693	41	28	0	208	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.93	0.93	0.00	0.91	0.91	0.00	1.00	1.00	1.00
PHF Volume:	352	213	0	0	747	0	30	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	352	213	0	0	747	0	30	0	0	0	0	0

Critical Gap Module:

Critical Gp:	4.1	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	6.4	XXXX	6.2	XXXXXX	XXXX	XXXXXX
FollowUpTim:	2.2	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	3.5	XXXX	3.3	XXXXXX	XXXX	XXXXXX

Capacity Module:

Cnflct Vol:	747	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	1663	XXXX	747	XXXX	XXXX	XXXXXX
Potent Cap.:	871	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	108	XXXX	416	XXXX	XXXX	XXXXXX
Move Cap.:	871	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	74	XXXX	416	XXXX	XXXX	XXXXXX
Volume/Cap:	0.40	XXXX	XXXX	XXXX	XXXX	XXXX	0.41	XXXX	0.00	XXXX	XXXX	XXXX

Level Of Service Module:

2Way95thQ:	2.0	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	1.6	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
Control Del:	11.9	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	84.2	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
LOS by Move:	B	*	*	*	*	*	F	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	XXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXX	XXXX	XXXXXX	XXXX	XXXX	XXXXXX
SharedQueue:	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
Shrd ConDel:	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX	XXXXXX	XXXX	XXXXXX
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	XXXXXX			XXXXXX			84.2			XXXXXX		
ApproachLOS:	*			*			F			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Opening Year (2010) With Project - Alternative 3
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): 2.7 Worst Case Level Of Service: F[60.8]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 13 columns representing different traffic volumes and adjustment factors.

Critical Gap Module: Table with 13 columns showing critical gap values and follow-up times.

Capacity Module: Table with 13 columns showing capacity-related metrics like Cnflict Vol, Potent Cap, etc.

Level Of Service Module: Table with 13 columns showing Level of Service (LOS) for different movements and approaches.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 3
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Cycle (sec): 65 Critical Vol./Cap.(X): 0.741
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 16.6
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	0	24	24	0	0	0	0	0	0
Lanes:	1	0	1	0	0	1	1	0	0	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	335	190	0	0	693	41	28	0	208	0	0	0
Added Vol:	0	13	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	335	203	0	0	693	41	28	0	208	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.93	0.93	0.00	0.91	0.91	0.00	1.00	1.00	1.00
PHF Volume:	352	213	0	0	747	0	30	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	352	213	0	0	747	0	30	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	352	213	0	0	747	0	30	0	0	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	1900	1900	1900	0	1900	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.19	0.11	0.00	0.00	0.39	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.26	0.79	0.00	0.00	0.53	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.74	0.14	0.00	0.00	0.74	0.00	0.27	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	28.1	1.6	0.0	0.0	14.8	0.0	32.6	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	28.1	1.6	0.0	0.0	14.8	0.0	32.6	0.0	0.0	0.0	0.0	0.0
LOS by Move:	C	A	A	A	B	A	C	A	A	A	A	A
HCM2kAvgQ:	8	1	0	0	13	0	1	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 3
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Cycle (sec): 85 Critical Vol./Cap.(X): 0.719
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 14.8
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	0	24	24	0	0	0	0	0	0
Lanes:	1	0	1	0	0	1	1	0	0	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	200	183	0	0	782	14	12	0	233	0	0	0
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	222	203	0	0	869	16	13	0	259	0	0	0
Added Vol:	0	41	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	222	244	0	0	869	16	13	0	259	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.89	0.89	0.89	0.97	0.97	0.00	0.86	0.86	0.00	1.00	1.00	1.00
PHF Volume:	250	275	0	0	895	0	15	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	250	275	0	0	895	0	15	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	250	275	0	0	895	0	15	0	0	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	1900	0	0	1900	1900	1900	0	1900	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.14	0.14	0.00	0.00	0.47	0.00	0.01	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.19	0.85	0.00	0.00	0.65	0.00	0.01	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.72	0.17	0.00	0.00	0.72	0.00	0.18	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	39.3	1.2	0.0	0.0	11.7	0.0	42.7	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	39.3	1.2	0.0	0.0	11.7	0.0	42.7	0.0	0.0	0.0	0.0	0.0
LOS by Move:	D	A	A	A	B	A	D	A	A	A	A	A
HCM2kAvgQ:	8	1	0	0	16	0	1	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 3
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 21.3 Worst Case Level Of Service: F[161.6]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Ignore		
Lanes:	0	0	0	0	0	1	0	2	0	0	2	1

Volume Module:

Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	154	0	23	92	819	0	0	493	781
Added Vol:	0	0	0	16	0	0	0	0	0	0	13	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	170	0	23	92	819	0	0	506	781
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.87	0.87	0.87	0.94	0.94	0.94	0.94	0.94	0.00
PHF Volume:	0	0	0	196	0	27	98	871	0	0	537	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	0	196	0	27	98	871	0	0	537	0

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxx	1169	1604	268	537	xxxx	xxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	xxxx	xxxx	xxxxx	189	107	736	1041	xxxx	xxxxx	xxxx	xxxx	xxxxx
Move Cap.:	xxxx	xxxx	xxxxx	176	96	736	1041	xxxx	xxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	xxxx	xxxx	xxxx	1.11	0.00	0.04	0.09	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.3	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	8.8	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	A	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	193	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	11.1	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	162	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	F	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			161.6			xxxxxx			xxxxxx		
ApproachLOS:	*			F			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Opening Year (2010) With Project - Alternative 3
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 24.4 Worst Case Level Of Service: F[161.7]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control (Stop Sign, Uncontrolled), Rights (Include, Ignore), and Lanes (0 0 0 0, 0 0 1! 0 0, 1 0 2 0 0, 0 0 2 0 1).

Volume Module: Table with 12 columns for different volume types (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume) and 4 rows for North, South, East, and West bounds.

Critical Gap Module: Table with 12 columns for gap types and 2 rows for Critical Gp and FollowUpTim.

Capacity Module: Table with 12 columns for capacity types (Cnflct Vol, Potent Cap., Move Cap., Volume/Cap) and 4 rows for North, South, East, and West bounds.

Level Of Service Module: Table with 12 columns for LOS types (2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS) and 4 rows for North, South, East, and West bounds.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 3
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.464
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 9.8
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	0	0	1	1	0	2	0	0	2

-----|-----|-----|-----|

Volume Module:

Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	154	0	23	92	819	0	0	493	781
Added Vol:	0	0	0	16	0	0	0	0	0	0	13	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	170	0	23	92	819	0	0	506	781
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.87	0.87	0.87	0.94	0.94	0.94	0.94	0.94	0.00
PHF Volume:	0	0	0	196	0	27	98	871	0	0	537	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	196	0	27	98	871	0	0	537	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	196	0	27	98	871	0	0	537	0

-----|-----|-----|-----|

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.73	1.00	0.73	0.43	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	0.88	0.00	0.12	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	1217	0	167	815	3610	0	0	3610	1900

-----|-----|-----|-----|

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.16	0.00	0.16	0.12	0.24	0.00	0.00	0.15	0.00	
Crit Moves:				****				****					
Green/Cycle:	0.00	0.00	0.00	0.35	0.00	0.35	0.52	0.52	0.00	0.00	0.52	0.00	
Volume/Cap:	0.00	0.00	0.00	0.46	0.00	0.46	0.23	0.46	0.00	0.00	0.29	0.00	
Delay/Veh:	0.0	0.0	0.0	16.0	0.0	16.0	8.1	9.3	0.0	0.0	8.2	0.0	
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
AdjDel/Veh:	0.0	0.0	0.0	16.0	0.0	16.0	8.1	9.3	0.0	0.0	8.2	0.0	
LOS by Move:	A	A	A	B	A	B	A	A	A	A	A	A	
HCM2kAvgQ:	0	0	0	4	0	4	1	6	0	0	3	0	

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 3
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.612
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 11.1
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	0	0	1	1	0	2	0	0	2

-----|-----|-----|-----|

Volume Module:

Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Initial Bse:	0	0	0	159	0	40	24	1120	0	0	378	651
Added Vol:	0	0	0	41	0	0	0	0	0	0	41	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	200	0	40	24	1120	0	0	419	651
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	0.83	0.83	0.83	0.96	0.96	0.96	0.95	0.95	0.00
PHF Volume:	0	0	0	241	0	48	25	1165	0	0	443	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	241	0	48	25	1165	0	0	443	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	241	0	48	25	1165	0	0	443	0

-----|-----|-----|-----|

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.73	1.00	0.73	0.49	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	0.83	0.00	0.17	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	1160	0	232	929	3610	0	0	3610	1900

-----|-----|-----|-----|

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.21	0.00	0.21	0.03	0.32	0.00	0.00	0.12	0.00
Crit Moves:				****				****				
Green/Cycle:	0.00	0.00	0.00	0.34	0.00	0.34	0.53	0.53	0.00	0.00	0.53	0.00
Volume/Cap:	0.00	0.00	0.00	0.61	0.00	0.61	0.05	0.61	0.00	0.00	0.23	0.00
Delay/Veh:	0.0	0.0	0.0	18.9	0.0	18.9	6.9	10.5	0.0	0.0	7.7	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	18.9	0.0	18.9	6.9	10.5	0.0	0.0	7.7	0.0
LOS by Move:	A	A	A	B	A	B	A	B	A	A	A	A
HCM2kAvgQ:	0	0	0	6	0	6	0	9	0	0	2	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 3
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.817
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 36.4
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

Volume Module:

Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	504	182	0	0	273	69	0	0	0	297	13	89
Added Vol:	13	0	0	0	0	0	0	0	0	8	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	517	182	0	0	273	69	0	0	0	305	13	89
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.82	0.82	0.82	0.83	0.83	0.83	1.00	1.00	1.00	0.89	0.89	0.89
PHF Volume:	630	222	0	0	328	83	0	0	0	341	15	99
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	630	222	0	0	328	83	0	0	0	341	15	99
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	630	222	0	0	328	83	0	0	0	341	15	99

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.59	0.41	0.00	0.00	0.00	1.00	0.13	0.87
Final Sat.:	1805	3610	0	0	2792	709	0	0	0	1550	198	1352

Capacity Analysis Module:

Vol/Sat:	0.35	0.06	0.00	0.00	0.12	0.12	0.00	0.00	0.00	0.22	0.07	0.07
Crit Moves:	****			****						****		
Green/Cycle:	0.40	0.58	0.00	0.00	0.18	0.18	0.00	0.00	0.00	0.26	0.26	0.26
Volume/Cap:	0.86	0.11	0.00	0.00	0.65	0.65	0.00	0.00	0.00	0.86	0.29	0.29
Delay/Veh:	37.5	9.2	0.0	0.0	40.5	40.5	0.0	0.0	0.0	49.2	30.0	30.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	37.5	9.2	0.0	0.0	40.5	40.5	0.0	0.0	0.0	49.2	30.0	30.0
LOS by Move:	D	A	A	A	D	D	A	A	A	D	C	C
HCM2kAvgQ:	21	2	0	0	7	7	0	0	0	14	3	3

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 3
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 115 Critical Vol./Cap.(X): 1.001
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 62.8
 Optimal Cycle: OPTIMIZED Level Of Service: E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	378	213	0	0	350	103	0	0	0	588	41	123
Added Vol:	41	0	0	0	0	0	0	0	0	20	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	419	213	0	0	350	103	0	0	0	608	41	123
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.93	0.93	0.93	1.00	1.00	1.00	0.84	0.84	0.84
PHF Volume:	466	238	0	0	375	110	0	0	0	721	49	146
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	466	238	0	0	375	110	0	0	0	721	49	146
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	466	238	0	0	375	110	0	0	0	721	49	146

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.55	0.45	0.00	0.00	0.00	1.00	0.25	0.75
Final Sat.:	1805	3610	0	0	2695	793	0	0	0	1554	388	1165

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.26	0.07	0.00	0.00	0.14	0.14	0.00	0.00	0.00	0.46	0.13	0.13
Crit Moves:	****			****						****		
Green/Cycle:	0.25	0.41	0.00	0.00	0.16	0.16	0.00	0.00	0.00	0.45	0.45	0.45
Volume/Cap:	1.03	0.16	0.00	0.00	0.89	0.89	0.00	0.00	0.00	1.03	0.28	0.28
Delay/Veh:	92.0	21.6	0.0	0.0	63.8	63.8	0.0	0.0	0.0	68.4	19.8	19.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	92.0	21.6	0.0	0.0	63.8	63.8	0.0	0.0	0.0	68.4	19.8	19.8
LOS by Move:	F	C	A	A	E	E	A	A	A	E	B	B
HCM2kAvgQ:	23	3	0	0	12	12	0	0	0	35	4	4

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 3
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 80 Critical Vol./Cap.(X): 0.721
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 26.9
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	0	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	504	182	0	0	273	69	0	0	0	297	13	89
Added Vol:	13	0	0	0	0	0	0	0	0	8	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	517	182	0	0	273	69	0	0	0	305	13	89
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.82	0.82	0.82	0.83	0.83	0.83	1.00	1.00	1.00	0.89	0.89	0.89
PHF Volume:	630	222	0	0	328	83	0	0	0	341	15	99
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	630	222	0	0	328	83	0	0	0	341	15	99
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	630	222	0	0	328	83	0	0	0	341	15	99

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.59	0.41	0.00	0.00	0.00	2.00	0.13	0.87
Final Sat.:	1805	3610	0	0	2792	709	0	0	0	3100	198	1352

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.35	0.06	0.00	0.00	0.12	0.12	0.00	0.00	0.00	0.11	0.07	0.07
Crit Moves:	****			****			****			****		
Green/Cycle:	0.44	0.66	0.00	0.00	0.23	0.23	0.00	0.00	0.00	0.14	0.14	0.14
Volume/Cap:	0.80	0.09	0.00	0.00	0.52	0.52	0.00	0.00	0.00	0.80	0.53	0.53
Delay/Veh:	25.2	4.9	0.0	0.0	27.9	27.9	0.0	0.0	0.0	41.2	32.7	32.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	25.2	4.9	0.0	0.0	27.9	27.9	0.0	0.0	0.0	41.2	32.7	32.7
LOS by Move:	C	A	A	A	C	C	A	A	A	D	C	C
HCM2kAvgQ:	15	1	0	0	5	5	0	0	0	7	4	4

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 3
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 85 Critical Vol./Cap.(X): 0.776
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 30.3
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	0	0	0	1	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	378	213	0	0	350	103	0	0	0	588	41	123
Added Vol:	41	0	0	0	0	0	0	0	0	20	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	419	213	0	0	350	103	0	0	0	608	41	123
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.93	0.93	0.93	1.00	1.00	1.00	0.84	0.84	0.84
PHF Volume:	466	238	0	0	375	110	0	0	0	721	49	146
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	466	238	0	0	375	110	0	0	0	721	49	146
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	466	238	0	0	375	110	0	0	0	721	49	146

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.55	0.45	0.00	0.00	0.00	2.00	0.25	0.75
Final Sat.:	1805	3610	0	0	2695	793	0	0	0	3108	388	1165

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.26	0.07	0.00	0.00	0.14	0.14	0.00	0.00	0.00	0.23	0.13	0.13
Crit Moves:	****			****			****			****		
Green/Cycle:	0.32	0.53	0.00	0.00	0.21	0.21	0.00	0.00	0.00	0.28	0.28	0.28
Volume/Cap:	0.82	0.12	0.00	0.00	0.66	0.66	0.00	0.00	0.00	0.82	0.44	0.44
Delay/Veh:	35.8	10.2	0.0	0.0	32.8	32.8	0.0	0.0	0.0	33.2	25.1	25.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	35.8	10.2	0.0	0.0	32.8	32.8	0.0	0.0	0.0	33.2	25.1	25.1
LOS by Move:	D	B	A	A	C	C	A	A	A	C	C	C
HCM2kAvgQ:	14	2	0	0	7	7	0	0	0	12	5	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 3
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.858
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 32.8
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0
Lanes:	0	0	1	1	0	0	1	0	2	0	0	0

Volume Module:

Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	0	600	540	116	480	0	61	1	383	0	0	0
Added Vol:	0	13	7	0	8	0	0	0	16	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	613	547	116	488	0	61	1	399	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.96	0.96	0.96	0.90	0.90	0.90	1.00	1.00	1.00
PHF Volume:	0	647	578	121	511	0	67	1	443	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	647	578	121	511	0	67	1	443	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	647	578	121	511	0	67	1	443	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.06	0.94	1.00	2.00	0.00	0.98	0.02	1.00	0.00	0.00	0.00
Final Sat.:	0	1772	1582	1805	3610	0	1512	27	1539	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.37	0.37	0.07	0.14	0.00	0.04	0.04	0.29	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.00	0.41	0.41	0.10	0.51	0.00	0.33	0.33	0.33	0.00	0.00	0.00
Volume/Cap:	0.00	0.88	0.88	0.67	0.28	0.00	0.14	0.14	0.88	0.00	0.00	0.00
Delay/Veh:	0.0	34.1	34.1	52.9	13.8	0.0	23.8	23.8	46.7	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	34.1	34.1	52.9	13.8	0.0	23.8	23.8	46.7	0.0	0.0	0.0
LOS by Move:	A	C	C	D	B	A	A	C	C	D	A	A
HCM2kAvgQ:	0	22	22	5	5	0	2	2	17	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Opening Year (2010) With Project - Alternative 3
 Evening Peak Hour

Level of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 115 Critical Vol./Cap.(X): 0.929
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 44.8
 Optimal Cycle: OPTIMIZED Level of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0
Lanes:	0	0	1	1	0	0	1	0	2	0	0	0

Volume Module:

Base Vol:	0	458	321	105	692	0	88	3	558	0	0	0
Growth Adj:	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
Initial Bse:	0	496	348	114	749	0	95	3	604	0	0	0
Added Vol:	0	41	20	0	20	0	0	0	41	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	537	368	114	769	0	95	3	645	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.91	0.91	0.91	0.93	0.93	0.93	1.00	1.00	1.00
PHF Volume:	0	565	387	125	849	0	102	3	692	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	565	387	125	849	0	102	3	692	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	565	387	125	849	0	102	3	692	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.89	0.89	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.19	0.81	1.00	2.00	0.00	0.97	0.03	1.00	0.00	0.00	0.00
Final Sat.:	0	2012	1378	1805	3610	0	1490	51	1541	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.28	0.28	0.07	0.24	0.00	0.07	0.07	0.45	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.00	0.30	0.30	0.09	0.38	0.00	0.48	0.48	0.48	0.00	0.00	0.00
Volume/Cap:	0.00	0.94	0.94	0.80	0.61	0.00	0.14	0.14	0.94	0.00	0.00	0.00
Delay/Veh:	0.0	55.9	55.9	76.0	29.3	0.0	16.9	16.9	47.3	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	55.9	55.9	76.0	29.3	0.0	16.9	16.9	47.3	0.0	0.0	0.0
LOS by Move:	A	E	E	E	C	A	B	B	D	A	A	A
HCM2kAvgQ:	0	22	22	6	13	0	2	2	29	0	0	0

 Note: Queue reported is the number of cars per lane.

Year 2025 Without Project – No Action

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 Without Project - No Action
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Ignore			Ignore			Include		
Lanes:	1	0	0	1	0	0	1	0	0	1	0	0

Volume Module:

Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	738	418	0	0	1526	90	61	0	457	0	0	0
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	738	418	0	0	1526	90	61	0	457	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.00	0.95	0.95	0.00	0.95	0.95	0.95
PHF Volume:	777	440	0	0	1606	0	64	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	777	440	0	0	1606	0	64	0	0	0	0	0

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	xxxx	6.2	xxxxxx	xxxx	xxxxxx
FollowUpTim:	2.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	xxxx	3.3	xxxxxx	xxxx	xxxxxx

Capacity Module:

Cnflict Vol:	1606	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3601	xxxx	1606	xxxx	xxxx	xxxxxx
Potent Cap.:	412	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6	xxxx	131	xxxx	xxxx	xxxxxx
Move Cap.:	412	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	0	xxxx	131	xxxx	xxxx	xxxxxx
Volume/Cap:	1.88	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxx	xxxx	0.00	xxxx	xxxx	xxxxxx

Level Of Service Module:

2Way95thQ:	51.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	429.6	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	F	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:		*			*			F			*	

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 Without Project - No Action
 Evening Peak Hour

Level Of Service Computation Report
 2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): OVERFLOW Worst Case Level of Service: F[xxxxxx]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Ignore			Ignore			Include		
Lanes:	1	0	0	1	0	0	1	0	0	1	0	0

Volume Module:

Base Vol:	200	183	0	0	782	14	12	0	233	0	0	0
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	489	447	0	0	1912	34	29	0	570	0	0	0
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	489	447	0	0	1912	34	29	0	570	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.00	0.95	0.95	0.00	0.95	0.95	0.95
PHF Volume:	515	471	0	0	2013	0	31	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	515	471	0	0	2013	0	31	0	0	0	0	0

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	xxxx	6.2	xxxxxx	xxxx	xxxxxx
FollowUpTim:	2.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	xxxx	3.3	xxxxxx	xxxx	xxxxxx

Capacity Module:

Cnflct Vol:	2013	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	3513	xxxx	2013	xxxx	xxxx	xxxxxx
Potent Cap.:	287	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	7	xxxx	75	xxxx	xxxx	xxxxxx
Move Cap.:	287	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	0	xxxx	75	xxxx	xxxx	xxxxxx
Volume/Cap:	1.79	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.00	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	34.1	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	400.0	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	F	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:		*			*			F			*	

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 Without Project - No Action
 Morning Peak Hour - With Improvements

Level of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Cycle (sec): 65 Critical Vol./Cap.(X): 0.859
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 19.6
 Optimal Cycle: OPTIMIZED Level of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	0	24	24	0	0	0	0	0	0
Lanes:	2	0	1	0	0	2	0	0	1	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	738	418	0	0	1526	90	61	0	457	0	0	0
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	738	418	0	0	1526	90	61	0	457	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.00	0.95	0.95	0.00	0.95	0.95	0.95
PHF Volume:	777	440	0	0	1606	0	64	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	777	440	0	0	1606	0	64	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	777	440	0	0	1606	0	64	0	0	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	3502	1900	0	0	3610	1900	1900	0	1900	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.22	0.23	0.00	0.00	0.44	0.00	0.03	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.26	0.78	0.00	0.00	0.52	0.00	0.04	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.86	0.30	0.00	0.00	0.86	0.00	0.58	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	31.3	2.2	0.0	0.0	17.9	0.0	38.3	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	31.3	2.2	0.0	0.0	17.9	0.0	38.3	0.0	0.0	0.0	0.0	0.0
LOS by Move:	C	A	A	A	B	A	D	A	A	A	A	A
HCM2kAvgQ:	11	3	0	0	18	0	2	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 Without Project - No Action
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

 Cycle (sec): 75 Critical Vol./Cap.(X): 0.858
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 17.0
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	24	24	0	0	0	0	0	0	0
Lanes:	2	0	1	0	0	2	0	0	1	0	0	0

Volume Module:

Base Vol:	200	183	0	0	782	14	12	0	233	0	0	0
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	489	447	0	0	1912	34	29	0	570	0	0	0
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	489	447	0	0	1912	34	29	0	570	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.00	0.95	0.95	0.00	0.95	0.95	0.95
PHF Volume:	515	471	0	0	2013	0	31	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	515	471	0	0	2013	0	31	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	515	471	0	0	2013	0	31	0	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	3502	1900	0	0	3610	1900	1900	0	1900	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.15	0.25	0.00	0.00	0.56	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****				****		****					
Green/Cycle:	0.17	0.82	0.00	0.00	0.65	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.86	0.30	0.00	0.00	0.86	0.00	0.32	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	42.0	1.7	0.0	0.0	13.8	0.0	38.3	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	42.0	1.7	0.0	0.0	13.8	0.0	38.3	0.0	0.0	0.0	0.0	0.0
LOS by Move:	D	A	A	A	B	A	D	A	A	A	A	A
HCM2kAvgQ:	9	3	0	0	22	0	1	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 Without Project - No Action
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 1041.6 Worst Case Level Of Service: F[9262.8]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Ignore		
Lanes:	0	0	0	0	0	1! 0	0	0	2	0	0	1

Volume Module:

Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	0	0	0	340	0	51	203	1802	0	0	1086	1719
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	340	0	51	203	1802	0	0	1086	1719
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.00
PHF Volume:	0	0	0	358	0	54	214	1897	0	0	1143	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	0	358	0	54	214	1897	0	0	1143	0

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxx	2518	3467	571	1143	xxxx	xxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	xxxx	xxxx	xxxxx	24	7	469	619	xxxx	xxxxx	xxxx	xxxx	xxxxx
Move Cap.:	xxxx	xxxx	xxxxx	17	4	469	619	xxxx	xxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	xxxx	xxxx	xxxx	20.64	0.00	0.12	0.35	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	1.5	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	13.9	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	B	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	20	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	52.0	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	9263	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	F	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			9262.8			xxxxxx			xxxxxx		
ApproachLOS:	*			F			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Year 2025 Without Project - No Action
Evening Peak Hour

Level Of Service Computation Report
2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 571.4 Worst Case Level Of Service: F[4943.8]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume) across four directions.

Critical Gap Module: Table with 12 columns for gap metrics (Critical Gp, FollowUpTim) across four directions.

Capacity Module: Table with 12 columns for capacity metrics (Cnflct Vol, Potent Cap., Move Cap., Volume/Cap) across four directions.

Level Of Service Module: Table with 12 columns for LOS metrics (2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS) across four directions.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 Without Project - No Action
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.858
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 13.5
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	1	0	0	1	0	2	0	0	2

Volume Module:												
Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	0	0	0	340	0	51	203	1802	0	0	1086	1719
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	340	0	51	203	1802	0	0	1086	1719
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.00
PHF Volume:	0	0	0	358	0	54	214	1897	0	0	1143	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	358	0	54	214	1897	0	0	1143	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	358	0	54	214	1897	0	0	1143	0

Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.95	1.00	0.85	0.21	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	1805	0	1615	391	3610	0	0	3610	1900

Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.20	0.00	0.03	0.55	0.53	0.00	0.00	0.32	0.00
Crit Moves:				****				****				
Green/Cycle:	0.00	0.00	0.00	0.22	0.00	0.22	0.65	0.65	0.00	0.00	0.65	0.00
Volume/Cap:	0.00	0.00	0.00	0.89	0.00	0.15	0.85	0.81	0.00	0.00	0.49	0.00
Delay/Veh:	0.0	0.0	0.0	44.3	0.0	19.0	30.7	10.3	0.0	0.0	5.7	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	44.3	0.0	19.0	30.7	10.3	0.0	0.0	5.7	0.0
LOS by Move:	A	A	A	D	A	B	C	B	A	A	A	A
HCM2kAvgQ:	0	0	0	10	0	1	6	16	0	0	6	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 Without Project - No Action
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 0.988
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 30.8
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	1	0	0	1	0	2	0	0	2

Volume Module:

Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	0	0	0	350	0	88	54	2465	0	0	831	1433
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	350	0	88	54	2465	0	0	831	1433
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.00
PHF Volume:	0	0	0	368	0	93	57	2594	0	0	875	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	368	0	93	57	2594	0	0	875	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	368	0	93	57	2594	0	0	875	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.95	1.00	0.85	0.30	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	1805	0	1615	562	3610	0	0	3610	1900

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.20	0.00	0.06	0.10	0.72	0.00	0.00	0.24	0.00	
Crit Moves:				****				****					
Green/Cycle:	0.00	0.00	0.00	0.21	0.00	0.21	0.73	0.73	0.00	0.00	0.73	0.00	
Volume/Cap:	0.00	0.00	0.00	0.99	0.00	0.28	0.14	0.99	0.00	0.00	0.33	0.00	
Delay/Veh:	0.0	0.0	0.0	90.9	0.0	40.6	5.1	30.8	0.0	0.0	6.0	0.0	
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
AdjDel/Veh:	0.0	0.0	0.0	90.9	0.0	40.6	5.1	30.8	0.0	0.0	6.0	0.0	
LOS by Move:	A	A	A	F	A	D	A	C	A	A	A	A	
HCM2kAvgQ:	0	0	0	19	0	3	1	56	0	0	6	0	

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 Without Project - No Action
 Morning Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

 Cycle (sec): 110 Critical Vol./Cap.(X): 1.271
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 136.7
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	915	330	0	0	496	126	0	0	0	539	24	161
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	915	330	0	0	496	126	0	0	0	539	24	161
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	963	348	0	0	522	133	0	0	0	567	25	170
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	963	348	0	0	522	133	0	0	0	567	25	170
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	963	348	0	0	522	133	0	0	0	567	25	170

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.59	0.41	0.00	0.00	0.00	1.00	0.13	0.87
Final Sat.:	1805	3610	0	0	2792	709	0	0	0	1550	198	1352

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.53	0.10	0.00	0.00	0.19	0.19	0.00	0.00	0.00	0.37	0.13	0.13
Crit Moves:	****			****						****		
Green/Cycle:	0.41	0.57	0.00	0.00	0.16	0.16	0.00	0.00	0.00	0.28	0.28	0.28
Volume/Cap:	1.30	0.17	0.00	0.00	1.14	1.14	0.00	0.00	0.00	1.30	0.45	0.45
Delay/Veh:	178.1	11.1	0.0	0.0	129	129.2	0.0	0.0	0.0	187.6	32.7	32.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	178.1	11.1	0.0	0.0	129	129.2	0.0	0.0	0.0	187.6	32.7	32.7
LOS by Move:	F	B	A	A	F	F	A	A	A	F	C	C
HCM2kAvgQ:	61	3	0	0	20	20	0	0	0	39	6	6

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 Without Project - No Action
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 105 Critical Vol./Cap.(X): 1.619
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 250.5
 Optimal Cycle: OPTIMIZED Level of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	687	388	0	0	635	187	0	0	0	1068	75	224
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	687	388	0	0	635	187	0	0	0	1068	75	224
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	723	408	0	0	669	197	0	0	0	1124	79	236
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	723	408	0	0	669	197	0	0	0	1124	79	236
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	723	408	0	0	669	197	0	0	0	1124	79	236

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.55	0.45	0.00	0.00	0.00	1.00	0.25	0.75
Final Sat.:	1805	3610	0	0	2695	793	0	0	0	1554	388	1165

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.40	0.11	0.00	0.00	0.25	0.25	0.00	0.00	0.00	0.72	0.20	0.20
Crit Moves:	****			****						****		
Green/Cycle:	0.24	0.41	0.00	0.00	0.17	0.17	0.00	0.00	0.00	0.44	0.44	0.44
Volume/Cap:	1.66	0.27	0.00	0.00	1.45	1.45	0.00	0.00	0.00	1.66	0.47	0.47
Delay/Veh:	347.9	20.5	0.0	0.0	254	254.3	0.0	0.0	0.0	332.7	21.1	21.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	347.9	20.5	0.0	0.0	254	254.3	0.0	0.0	0.0	332.7	21.1	21.1
LOS by Move:	F	C	A	A	F	F	A	A	A	F	C	C
HCM2kAvgQ:	59	5	0	0	34	34	0	0	0	96	8	8

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Year 2025 Without Project - No Action
Morning Peak Hour - With Improvements

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.748
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 26.5
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns for volume metrics across four directions. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with 12 columns for saturation flow metrics across four directions. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with 12 columns for capacity analysis metrics across four directions. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Year 2025 Without Project - No Action
Evening Peak Hour - With Improvements

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.916
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 37.7
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns representing different traffic movements and 12 rows of volume-related metrics like Base Vol, Growth Adj, etc.

Saturation Flow Module: Table with 12 columns and 4 rows showing saturation flow rates and adjustments.

Capacity Analysis Module: Table with 12 columns and 10 rows showing capacity analysis metrics like Vol/Sat, Crit Moves, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 Without Project - No Action
 Morning Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 1.441
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 186.4
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0
Lanes:	0	0	1	1	0	0	1	0	2	0	0	0

Volume Module:

Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	1090	982	210	871	0	110	2	696	0	0	0
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1090	982	210	871	0	110	2	696	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	1147	1033	222	917	0	116	2	733	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1147	1033	222	917	0	116	2	733	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	1147	1033	222	917	0	116	2	733	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.05	0.95	1.00	2.00	0.00	0.98	0.02	1.00	0.00	0.00	0.00
Final Sat.:	0	1764	1589	1805	3610	0	1514	27	1541	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.65	0.65	0.12	0.25	0.00	0.08	0.08	0.48	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.00	0.45	0.45	0.09	0.54	0.00	0.33	0.33	0.33	0.00	0.00	0.00
Volume/Cap:	0.00	1.44	1.44	1.44	0.47	0.00	0.23	0.23	1.44	0.00	0.00	0.00
Delay/Veh:	0.0	235	235.1	286.1	17.5	0.0	29.2	29.2	248.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	235	235.1	286.1	17.5	0.0	29.2	29.2	248.0	0.0	0.0	0.0
LOS by Move:	A	F	F	F	B	A	C	C	F	A	A	A
HCM2kAvgQ:	0	85	85	18	11	0	3	3	58	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 Without Project - No Action
 Evening Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 1.554
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 213.7
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0
Lanes:	0	0	1 1 0	1	0	2 0 0	0	1	0 1 0	0	0	0 0 0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	458	321	105	692	0	88	3	558	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	901	631	207	1361	0	173	6	1098	0	0	0
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	901	631	207	1361	0	173	6	1098	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	948	665	217	1433	0	182	6	1155	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	948	665	217	1433	0	182	6	1155	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	948	665	217	1433	0	182	6	1155	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.89	0.89	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.18	0.82	1.00	2.00	0.00	0.97	0.03	1.00	0.00	0.00	0.00
Final Sat.:	0	1991	1395	1805	3610	0	1490	51	1541	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.00	0.48	0.48	0.12	0.40	0.00	0.12	0.12	0.75	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.00	0.30	0.30	0.08	0.39	0.00	0.48	0.48	0.48	0.00	0.00	0.00
Volume/Cap:	0.00	1.57	1.57	1.45	1.02	0.00	0.26	0.26	1.57	0.00	0.00	0.00
Delay/Veh:	0.0	301	300.9	288.7	67.1	0.0	18.6	18.6	291.4	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	301	300.9	288.7	67.1	0.0	18.6	18.6	291.4	0.0	0.0	0.0
LOS by Move:	A	F	F	F	E	A	A	B	B	F	A	A
HCM2kAvgQ:	0	69	69	18	36	0	4	4	98	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 Without Project - No Action
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 100 Critical Vol./Cap.(X): 0.862
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 29.0
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound										
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Protected			Protected			Protected			Protected										
Rights:	Include			Include			Include			Include										
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0								
Lanes:	0	0	1	1	1	2	0	2	0	0	1	0	0	1	1	0	0	0	0	0

Volume Module:

Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	1090	982	210	871	0	110	2	696	0	0	0
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1090	982	210	871	0	110	2	696	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	1147	1033	222	917	0	116	2	733	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1147	1033	222	917	0	116	2	733	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	1147	1033	222	917	0	116	2	733	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.92	0.95	1.00	0.85	0.85	0.85	1.00	1.00	1.00
Lanes:	0.00	1.58	1.42	2.00	2.00	0.00	1.00	0.01	1.99	0.00	0.00	0.00
Final Sat.:	0	2647	2384	3502	3610	0	1615	9	3221	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.43	0.43	0.06	0.25	0.00	0.07	0.23	0.23	0.00	0.00	0.00	
Crit Moves:	****			****			****						
Green/Cycle:	0.00	0.49	0.49	0.10	0.59	0.00	0.25	0.25	0.25	0.00	0.00	0.00	
Volume/Cap:	0.00	0.89	0.89	0.63	0.43	0.00	0.28	0.89	0.89	0.00	0.00	0.00	
Delay/Veh:	0.0	28.0	28.0	47.0	11.7	0.0	30.3	48.1	48.1	0.0	0.0	0.0	
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
AdjDel/Veh:	0.0	28.0	28.0	47.0	11.7	0.0	30.3	48.1	48.1	0.0	0.0	0.0	
LOS by Move:	A	C	C	D	B	A	A	C	D	D	A	A	A
HCM2kAvgQ:	0	25	25	5	8	0	3	14	14	0	0	0	

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 Without Project - No Action
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 100 Critical Vol./Cap.(X): 0.880
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 36.4
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
	L	T	R	L	T	R	L	T	R	L	T	R
Movement:												
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0
Lanes:	0	0	1 1 1	2	0	2 0 0	1	0	0 1 1	0	0	0 0 0

Volume Module:

Base Vol:	0	458	321	105	692	0	88	3	558	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	901	631	207	1361	0	173	6	1098	0	0	0
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	901	631	207	1361	0	173	6	1098	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	948	665	217	1433	0	182	6	1155	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	948	665	217	1433	0	182	6	1155	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	948	665	217	1433	0	182	6	1155	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.89	0.89	0.92	0.95	1.00	0.85	0.85	0.85	1.00	1.00	1.00
Lanes:	0.00	1.76	1.24	2.00	2.00	0.00	1.00	0.01	1.99	0.00	0.00	0.00
Final Sat.:	0	2986	2093	3502	3610	0	1615	17	3217	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.32	0.32	0.06	0.40	0.00	0.11	0.36	0.36	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.00	0.35	0.35	0.10	0.45	0.00	0.39	0.39	0.39	0.00	0.00	0.00
Volume/Cap:	0.00	0.91	0.91	0.62	0.89	0.00	0.29	0.91	0.91	0.00	0.00	0.00
Delay/Veh:	0.0	39.1	39.1	46.6	31.7	0.0	21.0	39.1	39.1	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	39.1	39.1	46.6	31.7	0.0	21.0	39.1	39.1	0.0	0.0	0.0
LOS by Move:	A	D	D	D	C	A	C	D	D	A	A	A
HCM2kAvgQ:	0	21	21	4	25	0	4	21	21	0	0	0

 Note: Queue reported is the number of cars per lane.

Year 2025 With Project – Proposed Action “A”

Horseshoe Grande Property (Freeway Analysis)
Year 2025 With Project - Proposed Action "A"
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 13 columns for volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume) across four directions.

Critical Gap Module: Table with 13 columns for critical gap and follow-up time metrics across four directions.

Capacity Module: Table with 13 columns for capacity metrics (Cnflct Vol, Potent Cap., Move Cap., Volume/Cap) across four directions.

Level Of Service Module: Table with 13 columns for LOS metrics (2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS) across four directions.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Year 2025 With Project - Proposed Action "A"
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 13 columns representing different traffic volumes and adjustment factors.

Critical Gap Module: Table with 13 columns showing critical gap values and follow-up times.

Capacity Module: Table with 13 columns showing capacity-related metrics like conflict volume and potential capacity.

Level Of Service Module: Table with 13 columns detailing level of service, control delay, and shared queue metrics.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "A"
 Morning Peak Hour - With Improvements

Level of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Cycle (sec): 80 Critical Vol./Cap.(X): 0.825
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 21.2
 Optimal Cycle: OPTIMIZED Level of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	0	24	24	0	0	0	0	0	0
Lanes:	2	0	1	0	0	2	0	0	1	0	0	0

Volume Module:

Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	738	418	0	0	1526	90	61	0	457	0	0	0
Added Vol:	0	29	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	738	447	0	0	1526	90	61	0	457	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.00	0.95	0.95	0.00	0.95	0.95	0.95
PHF Volume:	777	471	0	0	1606	0	64	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	777	471	0	0	1606	0	64	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	777	471	0	0	1606	0	64	0	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	0.95	1.00	0.98	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	3502	1900	0	0	3610	1900	1858	0	1900	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.22	0.25	0.00	0.00	0.44	0.00	0.03	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.27	0.80	0.00	0.00	0.53	0.00	0.05	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.83	0.31	0.00	0.00	0.83	0.00	0.68	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	34.3	2.3	0.0	0.0	19.0	0.0	55.2	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	34.3	2.3	0.0	0.0	19.0	0.0	55.2	0.0	0.0	0.0	0.0	0.0
LOS by Move:	C	A	A	A	B	A	E	A	A	A	A	A
HCM2kAvgQ:	12	3	0	0	20	0	3	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "A"
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Cycle (sec): 110 Critical Vol./Cap.(X): 0.809
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 18.3
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	24	24	0	0	0	0	0	0	0
Lanes:	2	0	1	0	0	2	0	0	1	0	0	0

Volume Module:

Base Vol:	200	183	0	0	782	14	12	0	233	0	0	0
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	489	447	0	0	1912	34	29	0	570	0	0	0
Added Vol:	0	80	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	489	527	0	0	1912	34	29	0	570	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.00	0.95	0.95	0.00	0.95	0.95	0.95
PHF Volume:	515	555	0	0	2013	0	31	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	515	555	0	0	2013	0	31	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	515	555	0	0	2013	0	31	0	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	3502	1900	0	0	3610	1900	1900	0	1900	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.15	0.29	0.00	0.00	0.56	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.18	0.87	0.00	0.00	0.69	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.81	0.34	0.00	0.00	0.81	0.00	0.47	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	50.8	1.4	0.0	0.0	14.1	0.0	58.6	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	50.8	1.4	0.0	0.0	14.1	0.0	58.6	0.0	0.0	0.0	0.0	0.0
LOS by Move:	D	A	A	A	B	A	E	A	A	A	A	A
HCM2kAvgQ:	11	4	0	0	27	0	2	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "A"
 Morning Peak Hour

Level Of Service Computation Report
 2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 1517.9 Worst Case Level Of Service: F[11823.2]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Ignore		
Lanes:	0	0	0	0	0	1	0	0	0	0	0	1

Volume Module:

Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	0	0	0	340	0	51	203	1802	0	0	1086	1719
Added Vol:	0	0	0	68	0	0	0	0	0	0	29	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	408	0	51	203	1802	0	0	1115	1719
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.00
PHF Volume:	0	0	0	429	0	54	214	1897	0	0	1173	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	0	429	0	54	214	1897	0	0	1173	0

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxx	2549	3497	587	1173	xxxx	xxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	xxxx	xxxx	xxxxx	23	6	458	603	xxxx	xxxxx	xxxx	xxxx	xxxxx
Move Cap.:	xxxx	xxxx	xxxxx	16	4	458	603	xxxx	xxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	xxxx	xxxx	xxxx	26.26	0.00	0.12	0.35	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	1.6	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	14.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	B	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	18	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	61.1	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	F	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	*			F			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "A"
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 970.3 Worst Case Level Of Service: F[7264.4]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Ignore		
Lanes:	0	0	0	0	0	1!0	0	0	2	0	0	2

Volume Module:

Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	0	0	0	350	0	88	54	2465	0	0	831	1433
Added Vol:	0	0	0	91	0	0	0	0	0	0	80	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	441	0	88	54	2465	0	0	911	1433
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.00
PHF Volume:	0	0	0	464	0	93	57	2594	0	0	959	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	0	464	0	93	57	2594	0	0	959	0

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxx	2370	3667	480	959	xxxx	xxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	xxxx	xxxx	xxxxx	30	5	538	725	xxxx	xxxxx	xxxx	xxxx	xxxxx
Move Cap.:	xxxx	xxxx	xxxxx	28	5	538	725	xxxx	xxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	xxxx	xxxx	xxxx	16.47	0.00	0.17	0.08	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.3	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	10.4	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	B	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	33	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	68.4	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	7264	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	F	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			7264.4			xxxxxx			xxxxxx		
ApproachLOS:	*			F			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "A"
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.748
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 9.3
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
	L	T	R	L	T	R	L	T	R	L	T	R
Movement:												
Control:	Permitted			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	2	0	0	1	0	2	0	0	1

Volume Module:

Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	0	0	0	340	0	51	203	1802	0	0	1086	1719
Added Vol:	0	0	0	68	0	0	0	0	0	0	29	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	408	0	51	203	1802	0	0	1115	1719
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.00
PHF Volume:	0	0	0	429	0	54	214	1897	0	0	1173	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	429	0	54	214	1897	0	0	1173	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	429	0	54	214	1897	0	0	1173	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.85	0.21	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	3502	0	1615	405	3610	0	0	3610	1900

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.12	0.00	0.03	0.53	0.53	0.00	0.00	0.33	0.00
Crit Moves:				****			****					
Green/Cycle:	0.00	0.00	0.00	0.15	0.00	0.15	0.71	0.71	0.00	0.00	0.71	0.00
Volume/Cap:	0.00	0.00	0.00	0.79	0.00	0.22	0.74	0.74	0.00	0.00	0.46	0.00
Delay/Veh:	0.0	0.0	0.0	32.4	0.0	22.6	15.1	6.4	0.0	0.0	3.8	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	32.4	0.0	22.6	15.1	6.4	0.0	0.0	3.8	0.0
LOS by Move:	A	A	A	C	A	C	B	A	A	A	A	A
HCM2kAvgQ:	0	0	0	6	0	1	4	13	0	0	5	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "A"
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.934
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 18.1
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	2	0	0	1	0	2	0	0	2

Volume Module:												
Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	0	0	0	350	0	88	54	2465	0	0	831	1433
Added Vol:	0	0	0	91	0	0	0	0	0	0	80	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	441	0	88	54	2465	0	0	911	1433
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.00
PHF Volume:	0	0	0	464	0	93	57	2594	0	0	959	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	464	0	93	57	2594	0	0	959	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	464	0	93	57	2594	0	0	959	0

Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.85	0.28	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	3502	0	1615	536	3610	0	0	3610	1900

Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.13	0.00	0.06	0.11	0.72	0.00	0.00	0.27	0.00
Crit Moves:				****				****				
Green/Cycle:	0.00	0.00	0.00	0.14	0.00	0.14	0.77	0.77	0.00	0.00	0.77	0.00
Volume/Cap:	0.00	0.00	0.00	0.93	0.00	0.40	0.14	0.93	0.00	0.00	0.35	0.00
Delay/Veh:	0.0	0.0	0.0	63.2	0.0	36.3	2.8	15.2	0.0	0.0	3.3	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	63.2	0.0	36.3	2.8	15.2	0.0	0.0	3.3	0.0
LOS by Move:	A	A	A	E	A	D	A	B	A	A	A	A
HCM2kAvgQ:	0	0	0	10	0	3	1	36	0	0	4	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "A"
 Morning Peak Hour

Level of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 115 Critical Vol./Cap.(X): 1.311

Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 152.6

Optimal Cycle: OPTIMIZED Level of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	915	330	0	0	496	126	0	0	0	539	24	161
Added Vol:	29	0	0	0	0	0	0	0	0	36	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	944	330	0	0	496	126	0	0	0	575	24	161
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	993	348	0	0	522	133	0	0	0	605	25	170
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	993	348	0	0	522	133	0	0	0	605	25	170
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	993	348	0	0	522	133	0	0	0	605	25	170

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.81	0.81	0.81
Lanes:	1.00	2.00	0.00	0.00	1.59	0.41	0.00	0.00	0.00	1.00	0.13	0.87
Final Sat.:	1805	3610	0	0	2792	709	0	0	0	1548	198	1350

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.55	0.10	0.00	0.00	0.19	0.19	0.00	0.00	0.00	0.39	0.13	0.13
Crit Moves:	****			****						****		
Green/Cycle:	0.41	0.57	0.00	0.00	0.16	0.16	0.00	0.00	0.00	0.29	0.29	0.29
Volume/Cap:	1.34	0.17	0.00	0.00	1.19	1.19	0.00	0.00	0.00	1.34	0.43	0.43
Delay/Veh:	194.3	11.9	0.0	0.0	153	152.7	0.0	0.0	0.0	203.2	33.1	33.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	194.3	11.9	0.0	0.0	153	152.7	0.0	0.0	0.0	203.2	33.1	33.1
LOS by Move:	F	B	A	A	F	F	A	A	A	F	C	C
HCM2kAvgQ:	67	3	0	0	22	22	0	0	0	44	6	6

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "A"
 Evening Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

 Cycle (sec): 110 Critical Vol./Cap.(X): 1.698
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 282.1
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	687	388	0	0	635	187	0	0	0	1068	75	224
Added Vol:	80	0	0	0	0	0	0	0	0	47	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	767	388	0	0	635	187	0	0	0	1115	75	224
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	807	408	0	0	669	197	0	0	0	1174	79	236
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	807	408	0	0	669	197	0	0	0	1174	79	236
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	807	408	0	0	669	197	0	0	0	1174	79	236

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.55	0.45	0.00	0.00	0.00	1.00	0.25	0.75
Final Sat.:	1805	3610	0	0	2695	793	0	0	0	1554	388	1165

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.45	0.11	0.00	0.00	0.25	0.25	0.00	0.00	0.00	0.76	0.20	0.20
Crit Moves:	****			****			****			****		
Green/Cycle:	0.26	0.42	0.00	0.00	0.16	0.16	0.00	0.00	0.00	0.43	0.43	0.43
Volume/Cap:	1.74	0.27	0.00	0.00	1.52	1.52	0.00	0.00	0.00	1.74	0.47	0.47
Delay/Veh:	382.9	20.9	0.0	0.0	287	287.5	0.0	0.0	0.0	369.2	22.2	22.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	382.9	20.9	0.0	0.0	287	287.5	0.0	0.0	0.0	369.2	22.2	22.2
LOS by Move:	F	C	A	A	F	F	A	A	A	F	C	C
HCM2kAvgQ:	70	5	0	0	36	36	0	0	0	106	8	8

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Year 2025 With Project - Proposed Action "A"
Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 80 Critical Vol./Cap.(X): 0.762
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 27.7
Optimal Cycle: OPTIMIZED Level Of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L, T, R), Control (Protected), Rights (Include), Min. Green, and Lanes.

Volume Module: Table with 12 columns representing different volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume).

Saturation Flow Module: Table with 12 columns representing saturation flow metrics (Sat/Lane, Adjustment, Lanes, Final Sat.).

Capacity Analysis Module: Table with 12 columns representing capacity analysis metrics (Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ).

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "A"
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 95 Critical Vol./Cap.(X): 0.954
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 43.8
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	2	0	2	0	0	2	0	0	0	1	1	0

Volume Module:

Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	687	388	0	0	635	187	0	0	0	1068	75	224
Added Vol:	80	0	0	0	0	0	0	0	0	47	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	767	388	0	0	635	187	0	0	0	1115	75	224
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	807	408	0	0	669	197	0	0	0	1174	79	236
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	807	408	0	0	669	197	0	0	0	1174	79	236
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	807	408	0	0	669	197	0	0	0	1174	79	236

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.95	1.00	1.00	0.95	0.85	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	2.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	2.00	0.25	0.75
Final Sat.:	3502	3610	0	0	3610	1615	0	0	0	3108	388	1165

Capacity Analysis Module:

Vol/Sat:	0.23	0.11	0.00	0.00	0.19	0.12	0.00	0.00	0.00	0.38	0.20	0.20
Crit Moves:	****			****						****		
Green/Cycle:	0.24	0.44	0.00	0.00	0.19	0.19	0.00	0.00	0.00	0.40	0.40	0.40
Volume/Cap:	0.95	0.26	0.00	0.00	0.95	0.63	0.00	0.00	0.00	0.95	0.51	0.51
Delay/Veh:	56.1	17.1	0.0	0.0	61.1	39.1	0.0	0.0	0.0	41.3	21.9	21.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	56.1	17.1	0.0	0.0	61.1	39.1	0.0	0.0	0.0	41.3	21.9	21.9
LOS by Move:	E	B	A	A	E	D	A	A	A	D	C	C
HCM2kAvgQ:	17	4	0	0	15	6	0	0	0	23	8	8

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "A"
 Morning Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 1.511
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 210.9
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound										
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Protected			Protected			Protected			Protected										
Rights:	Include			Include			Include			Include										
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0								
Lanes:	0	0	1	1	0	1	0	2	0	0	0	1	0	1	0	0	0	0	0	0

Volume Module:

Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	1090	982	210	871	0	110	2	696	0	0	0
Added Vol:	0	29	16	0	36	0	0	0	68	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1119	998	210	907	0	110	2	764	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	1178	1050	222	955	0	116	2	805	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1178	1050	222	955	0	116	2	805	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	1178	1050	222	955	0	116	2	805	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.06	0.94	1.00	2.00	0.00	0.98	0.02	1.00	0.00	0.00	0.00
Final Sat.:	0	1773	1581	1805	3610	0	1512	27	1539	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.66	0.66	0.12	0.26	0.00	0.08	0.08	0.52	0.00	0.00	0.00	
Crit Moves:	****			****			****			****			
Green/Cycle:	0.00	0.44	0.44	0.08	0.52	0.00	0.35	0.35	0.35	0.00	0.00	0.00	
Volume/Cap:	0.00	1.52	1.52	1.47	0.51	0.00	0.22	0.22	1.52	0.00	0.00	0.00	
Delay/Veh:	0.0	269	269.2	300.2	18.9	0.0	27.9	27.9	279.7	0.0	0.0	0.0	
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
AdjDel/Veh:	0.0	269	269.2	300.2	18.9	0.0	27.9	27.9	279.7	0.0	0.0	0.0	
LOS by Move:	A	F	F	F	B	A	A	C	C	F	A	A	A
HCM2kAvgQ:	0	92	92	19	12	0	3	3	67	0	0	0	

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "A"
 Evening Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 1.669
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 254.4
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound										
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Protected			Protected			Protected			Protected										
Rights:	Include			Include			Include			Include										
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0								
Lanes:	0	0	1	1	0	1	0	2	0	0	0	1	0	1	0	0	0	0	0	0

Volume Module:

Base Vol:	0	458	321	105	692	0	88	3	558	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	901	631	207	1361	0	173	6	1098	0	0	0
Added Vol:	0	80	42	0	47	0	0	0	91	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	981	673	207	1408	0	173	6	1189	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	1033	709	217	1482	0	182	6	1251	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1033	709	217	1482	0	182	6	1251	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1033	709	217	1482	0	182	6	1251	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.89	0.89	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.19	0.81	1.00	2.00	0.00	0.97	0.03	1.00	0.00	0.00	0.00
Final Sat.:	0	2010	1380	1805	3610	0	1490	51	1541	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.51	0.51	0.12	0.41	0.00	0.12	0.12	0.81	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.00	0.30	0.30	0.08	0.39	0.00	0.48	0.48	0.48	0.00	0.00	0.00
Volume/Cap:	0.00	1.69	1.69	1.45	1.06	0.00	0.25	0.25	1.69	0.00	0.00	0.00
Delay/Veh:	0.0	358	357.7	288.7	79.0	0.0	18.5	18.5	348.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	358	357.7	288.7	79.0	0.0	18.5	18.5	348.0	0.0	0.0	0.0
LOS by Move:	A	F	F	F	E	A	B	B	F	A	A	A
HCM2kAvgQ:	0	80	80	18	39	0	4	4	114	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "A"
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 110 Critical Vol./Cap.(X): 0.885
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 32.3
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound										
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Protected			Protected			Protected			Protected										
Rights:	Include			Include			Include			Include										
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0								
Lanes:	0	0	1	1	1	2	0	2	0	0	1	0	0	1	1	0	0	0	0	0

Volume Module:

Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	1090	982	210	871	0	110	2	696	0	0	0
Added Vol:	0	29	16	0	36	0	0	0	68	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1119	998	210	907	0	110	2	764	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	1178	1050	222	955	0	116	2	805	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1178	1050	222	955	0	116	2	805	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	1178	1050	222	955	0	116	2	805	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.92	0.95	1.00	0.85	0.85	0.85	1.00	1.00	1.00
Lanes:	0.00	1.59	1.41	2.00	2.00	0.00	1.00	0.01	1.99	0.00	0.00	0.00
Final Sat.:	0	2659	2371	3502	3610	0	1615	8	3222	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.44	0.44	0.06	0.26	0.00	0.07	0.25	0.25	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.00	0.49	0.49	0.09	0.58	0.00	0.28	0.28	0.28	0.00	0.00	0.00
Volume/Cap:	0.00	0.91	0.91	0.70	0.46	0.00	0.26	0.91	0.91	0.00	0.00	0.00
Delay/Veh:	0.0	31.2	31.2	55.1	13.4	0.0	31.4	51.3	51.3	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	31.2	31.2	55.1	13.4	0.0	31.4	51.3	51.3	0.0	0.0	0.0
LOS by Move:	A	C	C	E	B	A	C	D	D	A	A	A
HCM2kAvgQ:	0	28	28	5	10	0	3	17	17	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "A"
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 115 Critical Vol./Cap.(X): 0.922
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 43.4
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound										
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Protected			Protected			Protected			Protected										
Rights:	Include			Include			Include			Include										
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0								
Lanes:	0	0	1	1	1	2	0	2	0	0	1	0	0	1	1	0	0	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	458	321	105	692	0	88	3	558	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	901	631	207	1361	0	173	6	1098	0	0	0
Added Vol:	0	80	42	0	47	0	0	0	91	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	981	673	207	1408	0	173	6	1189	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	1033	709	217	1482	0	182	6	1251	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1033	709	217	1482	0	182	6	1251	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	1033	709	217	1482	0	182	6	1251	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.89	0.89	0.92	0.95	1.00	0.85	0.85	0.85	1.00	1.00	1.00
Lanes:	0.00	1.78	1.22	2.00	2.00	0.00	1.00	0.01	1.99	0.00	0.00	0.00
Final Sat.:	0	3015	2070	3502	3610	0	1615	16	3218	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.00	0.34	0.34	0.06	0.41	0.00	0.11	0.39	0.39	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.00	0.36	0.36	0.09	0.45	0.00	0.41	0.41	0.41	0.00	0.00	0.00
Volume/Cap:	0.00	0.94	0.94	0.71	0.91	0.00	0.27	0.94	0.94	0.00	0.00	0.00
Delay/Veh:	0.0	46.2	46.2	58.9	37.9	0.0	22.7	46.2	46.2	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	46.2	46.2	58.9	37.9	0.0	22.7	46.2	46.2	0.0	0.0	0.0
LOS by Move:	A	D	D	E	D	A	C	D	D	A	A	A
HCM2kAvgQ:	0	26	26	5	30	0	4	26	26	0	0	0

Note: Queue reported is the number of cars per lane.

Year 2025 With Project – Proposed Action “B”

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "B"
 Morning Peak Hour

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Ignore			Ignore			Include		
Lanes:	1	0	0	0	0	1	0	0	1	0	0	0

Volume Module:

Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	738	418	0	0	1526	90	61	0	457	0	0	0
Added Vol:	0	28	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	738	446	0	0	1526	90	61	0	457	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.00	0.95	0.95	0.00	0.95	0.95	0.95
PHF Volume:	777	470	0	0	1606	0	64	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	777	470	0	0	1606	0	64	0	0	0	0	0

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	xxxx	6.2	xxxxxx	xxxx	xxxxxx
FollowUpTim:	2.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	xxxx	3.3	xxxxxx	xxxx	xxxxxx

Capacity Module:

Cnflct Vol:	1606	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	3630	xxxx	1606	xxxx	xxxx	xxxxxx
Potent Cap.:	412	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	6	xxxx	131	xxxx	xxxx	xxxxxx
Move Cap.:	412	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	0	xxxx	131	xxxx	xxxx	xxxxxx
Volume/Cap:	1.88	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.00	xxxx	xxxx	xxxx

Level of Service Module:

2Way95thQ:	51.3	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	429.6	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	F	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	*			*			F			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "B"
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxxx]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Ignore			Ignore			Include		
Lanes:	1	0	0	0	0	1	1	0	0	0	0	0

Volume Module:

Base Vol:	200	183	0	0	782	14	12	0	233	0	0	0
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	489	447	0	0	1912	34	29	0	570	0	0	0
Added Vol:	0	77	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	489	524	0	0	1912	34	29	0	570	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.00	0.95	0.95	0.00	0.95	0.95	0.95
PHF Volume:	515	552	0	0	2013	0	31	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	515	552	0	0	2013	0	31	0	0	0	0	0

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	xxxx	6.2	xxxxxx	xxxx	xxxxxx
FollowUpTim:	2.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	xxxx	3.3	xxxxxx	xxxx	xxxxxx

Capacity Module:

Cnflct Vol:	2013	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3594	xxxx	2013	xxxx	xxxx	xxxxxx
Potent Cap.:	287	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6	xxxx	75	xxxx	xxxx	xxxxxx
Move Cap.:	287	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	0	xxxx	75	xxxx	xxxx	xxxxxx
Volume/Cap:	1.79	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.00	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	34.1	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	400.0	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	F	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	*			*			F			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "B"
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

 Cycle (sec): 80 Critical Vol./Cap.(X): 0.825
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 21.2
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	0	24	24	0	0	0	0	0	0
Lanes:	2	0	1	0	0	2	0	0	1	0	0	0

Volume Module:

Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	738	418	0	0	1526	90	61	0	457	0	0	0
Added Vol:	0	28	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	738	446	0	0	1526	90	61	0	457	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.00	0.95	0.95	0.00	0.95	0.95	0.95
PHF Volume:	777	470	0	0	1606	0	64	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	777	470	0	0	1606	0	64	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	777	470	0	0	1606	0	64	0	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	0.95	1.00	0.98	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	3502	1900	0	0	3610	1900	1858	0	1900	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.22	0.25	0.00	0.00	0.44	0.00	0.03	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.27	0.80	0.00	0.00	0.53	0.00	0.05	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.83	0.31	0.00	0.00	0.83	0.00	0.68	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	34.3	2.3	0.0	0.0	19.0	0.0	55.2	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	34.3	2.3	0.0	0.0	19.0	0.0	55.2	0.0	0.0	0.0	0.0	0.0
LOS by Move:	C	A	A	A	B	A	E	A	A	A	A	A
HCM2kAvgQ:	12	3	0	0	20	0	3	0	0	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "B"
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Cycle (sec): 105 Critical Vol./Cap.(X): 0.814
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 18.0
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	24	24	0	0	0	0	0	0	0
Lanes:	2	0	1	0	0	2	0	0	1	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	200	183	0	0	782	14	12	0	233	0	0	0
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	489	447	0	0	1912	34	29	0	570	0	0	0
Added Vol:	0	77	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	489	524	0	0	1912	34	29	0	570	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.00	0.95	0.95	0.00	0.95	0.95	0.95
PHF Volume:	515	552	0	0	2013	0	31	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	515	552	0	0	2013	0	31	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	515	552	0	0	2013	0	31	0	0	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	3502	1900	0	0	3610	1900	1900	0	1900	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.15	0.29	0.00	0.00	0.56	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.18	0.87	0.00	0.00	0.69	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.81	0.34	0.00	0.00	0.81	0.00	0.45	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	49.3	1.5	0.0	0.0	13.9	0.0	55.5	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	49.3	1.5	0.0	0.0	13.9	0.0	55.5	0.0	0.0	0.0	0.0	0.0
LOS by Move:	D	A	A	A	B	A	E	A	A	A	A	A
HCM2kAvgQ:	11	4	0	0	26	0	2	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "B"
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 1502.0 Worst Case Level Of Service: F[11740.4]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Ignore		
Lanes:	0	0	0	0	0	1	0	0	0	0	0	1

Volume Module:

Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	0	0	0	340	0	51	203	1802	0	0	1086	1719
Added Vol:	0	0	0	66	0	0	0	0	0	0	28	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	406	0	51	203	1802	0	0	1114	1719
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.00
PHF Volume:	0	0	0	427	0	54	214	1897	0	0	1172	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	0	427	0	54	214	1897	0	0	1172	0

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxx	2548	3496	586	1172	xxxx	xxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	xxxx	xxxx	xxxxx	23	6	459	603	xxxx	xxxxx	xxxx	xxxx	xxxxx
Move Cap.:	xxxx	xxxx	xxxxx	16	4	459	603	xxxx	xxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	xxxx	xxxx	xxxxx	26.08	0.00	0.12	0.35	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	1.6	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	14.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	B	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	18	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	60.8	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	F	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	*			F			*			*		

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Year 2025 With Project - Proposed Action "B"
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 954.4 Worst Case Level Of Service: F[7174.7]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume components. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume.

Critical Gap Module: Table with 12 columns for gap metrics. Rows include Critical Gp and FollowUpTim.

Capacity Module: Table with 12 columns for capacity metrics. Rows include Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with 12 columns for LOS metrics. Rows include 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "B"
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.747
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 9.2
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	2	0	0	1	0	2	0	0	1

Volume Module:

Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	0	0	0	340	0	51	203	1802	0	0	1086	1719
Added Vol:	0	0	0	66	0	0	0	0	0	0	28	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	406	0	51	203	1802	0	0	1114	1719
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.00
PHF Volume:	0	0	0	427	0	54	214	1897	0	0	1172	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	427	0	54	214	1897	0	0	1172	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	427	0	54	214	1897	0	0	1172	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.85	0.21	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	3502	0	1615	405	3610	0	0	3610	1900

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.12	0.00	0.03	0.53	0.53	0.00	0.00	0.32	0.00
Crit Moves:				****			****					
Green/Cycle:	0.00	0.00	0.00	0.15	0.00	0.15	0.71	0.71	0.00	0.00	0.71	0.00
Volume/Cap:	0.00	0.00	0.00	0.79	0.00	0.22	0.74	0.74	0.00	0.00	0.46	0.00
Delay/Veh:	0.0	0.0	0.0	32.1	0.0	22.6	15.1	6.4	0.0	0.0	3.8	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	32.1	0.0	22.6	15.1	6.4	0.0	0.0	3.8	0.0
LOS by Move:	A	A	A	C	A	C	B	A	A	A	A	A
HCM2kAvgQ:	0	0	0	6	0	1	4	13	0	0	5	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "B"
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

 Cycle (sec): 90 Critical Vol./Cap.(X): 0.933
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 18.0
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	2	0	0	1	0	2	0	0	1

Volume Module:

Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	0	0	0	350	0	88	54	2465	0	0	831	1433
Added Vol:	0	0	0	88	0	0	0	0	0	0	77	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	438	0	88	54	2465	0	0	908	1433
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.00
PHF Volume:	0	0	0	461	0	93	57	2594	0	0	956	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	461	0	93	57	2594	0	0	956	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
FinalVolume:	0	0	0	461	0	93	57	2594	0	0	956	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.85	0.28	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	3502	0	1615	540	3610	0	0	3610	1900

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.13	0.00	0.06	0.10	0.72	0.00	0.00	0.26	0.00	
Crit Moves:				****				****					
Green/Cycle:	0.00	0.00	0.00	0.14	0.00	0.14	0.77	0.77	0.00	0.00	0.77	0.00	
Volume/Cap:	0.00	0.00	0.00	0.93	0.00	0.41	0.14	0.93	0.00	0.00	0.34	0.00	
Delay/Veh:	0.0	0.0	0.0	63.1	0.0	36.4	2.8	15.1	0.0	0.0	3.3	0.0	
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
AdjDel/Veh:	0.0	0.0	0.0	63.1	0.0	36.4	2.8	15.1	0.0	0.0	3.3	0.0	
LOS by Move:	A	A	A	E	A	D	A	B	A	A	A	A	
HCM2kAvgQ:	0	0	0	10	0	3	1	36	0	0	4	0	

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "B"
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 115 Critical Vol./Cap.(X): 1.309
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 151.9
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	915	330	0	0	496	126	0	0	0	539	24	161
Added Vol:	28	0	0	0	0	0	0	0	0	35	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	943	330	0	0	496	126	0	0	0	574	24	161
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	992	348	0	0	522	133	0	0	0	604	25	170
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	992	348	0	0	522	133	0	0	0	604	25	170
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	992	348	0	0	522	133	0	0	0	604	25	170

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.59	0.41	0.00	0.00	0.00	1.00	0.13	0.87
Final Sat.:	1805	3610	0	0	2792	709	0	0	0	1550	198	1352

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.55	0.10	0.00	0.00	0.19	0.19	0.00	0.00	0.00	0.39	0.13	0.13
Crit Moves:	****			****						****		
Green/Cycle:	0.41	0.57	0.00	0.00	0.16	0.16	0.00	0.00	0.00	0.29	0.29	0.29
Volume/Cap:	1.33	0.17	0.00	0.00	1.19	1.19	0.00	0.00	0.00	1.33	0.43	0.43
Delay/Veh:	193.2	11.9	0.0	0.0	153	152.7	0.0	0.0	0.0	202.2	33.1	33.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	193.2	11.9	0.0	0.0	153	152.7	0.0	0.0	0.0	202.2	33.1	33.1
LOS by Move:	F	B	A	A	F	F	A	A	A	F	C	C
HCM2kAvgQ:	66	3	0	0	22	22	0	0	0	44	6	6

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "B"
 Evening Peak Hour

Level of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 110 Critical Vol./Cap.(X): 1.695
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 281.1
 Optimal Cycle: OPTIMIZED Level of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	687	388	0	0	635	187	0	0	0	1068	75	224
Added Vol:	77	0	0	0	0	0	0	0	0	46	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	764	388	0	0	635	187	0	0	0	1114	75	224
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	804	408	0	0	669	197	0	0	0	1173	79	236
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	804	408	0	0	669	197	0	0	0	1173	79	236
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	804	408	0	0	669	197	0	0	0	1173	79	236

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.55	0.45	0.00	0.00	0.00	1.00	0.25	0.75
Final Sat.:	1805	3610	0	0	2695	793	0	0	0	1554	388	1165

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.45	0.11	0.00	0.00	0.25	0.25	0.00	0.00	0.00	0.75	0.20	0.20
Crit Moves:	****			****			****			****		
Green/Cycle:	0.26	0.42	0.00	0.00	0.16	0.16	0.00	0.00	0.00	0.43	0.43	0.43
Volume/Cap:	1.74	0.27	0.00	0.00	1.52	1.52	0.00	0.00	0.00	1.74	0.47	0.47
Delay/Veh:	381.4	21.0	0.0	0.0	287	287.5	0.0	0.0	0.0	367.6	22.2	22.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	381.4	21.0	0.0	0.0	287	287.5	0.0	0.0	0.0	367.6	22.2	22.2
LOS by Move:	F	C	A	A	F	F	A	A	A	F	C	C
HCM2kAvgQ:	69	5	0	0	36	36	0	0	0	105	8	8

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "B"
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 80 Critical Vol./Cap.(X): 0.760
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 27.7
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	2	0	2	0	0	2	1	0	0	0	1	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	915	330	0	0	496	126	0	0	0	539	24	161
Added Vol:	28	0	0	0	0	0	0	0	0	35	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	943	330	0	0	496	126	0	0	0	574	24	161
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	992	348	0	0	522	133	0	0	0	604	25	170
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	992	348	0	0	522	133	0	0	0	604	25	170
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	992	348	0	0	522	133	0	0	0	604	25	170

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.95	1.00	1.00	0.88	0.88	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	2.00	2.00	0.00	0.00	2.39	0.61	0.00	0.00	0.00	2.00	0.13	0.87
Final Sat.:	3502	3610	0	0	4012	1019	0	0	0	3100	198	1352

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.28	0.10	0.00	0.00	0.13	0.13	0.00	0.00	0.00	0.19	0.13	0.13
Crit Moves:	****			****			****			****		
Green/Cycle:	0.34	0.57	0.00	0.00	0.23	0.23	0.00	0.00	0.00	0.23	0.23	0.23
Volume/Cap:	0.83	0.17	0.00	0.00	0.58	0.58	0.00	0.00	0.00	0.83	0.54	0.54
Delay/Veh:	29.4	8.4	0.0	0.0	28.4	28.4	0.0	0.0	0.0	35.4	27.2	27.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	29.4	8.4	0.0	0.0	28.4	28.4	0.0	0.0	0.0	35.4	27.2	27.2
LOS by Move:	C	A	A	A	C	C	A	A	A	D	C	C
HCM2kAvgQ:	14	2	0	0	6	6	0	0	0	10	5	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "B"
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 95 Critical Vol./Cap.(X): 0.953
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 43.5
 Optimal Cycle: OPTIMIZED Level of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	2	0	2	0	0	2	0	0	0	1	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	687	388	0	0	635	187	0	0	0	1068	75	224
Added Vol:	77	0	0	0	0	0	0	0	0	46	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	764	388	0	0	635	187	0	0	0	1114	75	224
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	804	408	0	0	669	197	0	0	0	1173	79	236
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	804	408	0	0	669	197	0	0	0	1173	79	236
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	804	408	0	0	669	197	0	0	0	1173	79	236

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.95	1.00	1.00	0.95	0.85	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	2.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	2.00	0.25	0.75
Final Sat.:	3502	3610	0	0	3610	1615	0	0	0	3108	388	1165

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.23	0.11	0.00	0.00	0.19	0.12	0.00	0.00	0.00	0.38	0.20	0.20
Crit Moves:	****			****						****		
Green/Cycle:	0.24	0.44	0.00	0.00	0.19	0.19	0.00	0.00	0.00	0.40	0.40	0.40
Volume/Cap:	0.95	0.26	0.00	0.00	0.95	0.63	0.00	0.00	0.00	0.95	0.51	0.51
Delay/Veh:	55.8	17.2	0.0	0.0	60.8	39.1	0.0	0.0	0.0	41.0	21.9	21.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	55.8	17.2	0.0	0.0	60.8	39.1	0.0	0.0	0.0	41.0	21.9	21.9
LOS by Move:	E	B	A	A	E	D	A	A	A	D	C	C
HCM2kAvgQ:	17	4	0	0	15	6	0	0	0	23	8	8

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "B"
 Morning Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 1.509
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 210.2
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0
Lanes:	0	0	1	1	0	0	1	0	2	0	0	0

Volume Module:

Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	1090	982	210	871	0	110	2	696	0	0	0
Added Vol:	0	28	16	0	35	0	0	0	66	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1118	998	210	906	0	110	2	762	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	1177	1050	222	954	0	116	2	802	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1177	1050	222	954	0	116	2	802	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	1177	1050	222	954	0	116	2	802	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.06	0.94	1.00	2.00	0.00	0.98	0.02	1.00	0.00	0.00	0.00
Final Sat.:	0	1772	1582	1805	3610	0	1512	27	1539	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.66	0.66	0.12	0.26	0.00	0.08	0.08	0.52	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.00	0.44	0.44	0.08	0.52	0.00	0.34	0.34	0.34	0.00	0.00	0.00
Volume/Cap:	0.00	1.51	1.51	1.47	0.51	0.00	0.22	0.22	1.51	0.00	0.00	0.00
Delay/Veh:	0.0	268	268.2	300.2	18.9	0.0	27.9	27.9	278.8	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	268	268.2	300.2	18.9	0.0	27.9	27.9	278.8	0.0	0.0	0.0
LOS by Move:	A	F	F	F	B	A	A	C	C	F	A	A
HCM2kAvgQ:	0	92	92	19	12	0	3	3	67	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "B"
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.665
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 253.1
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound											
Movement:	L	T	R	L	T	R	L	T	R	L	T	R									
Control:	Protected			Protected			Protected			Protected											
Rights:	Include			Include			Include			Include											
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0									
Lanes:	0	0	1	1	0	0	1	0	2	0	0	0	1	0	1	0	0	0	0	0	0

-----|-----|-----|-----|

Volume Module:

Base Vol:	0	458	321	105	692	0	88	3	558	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	901	631	207	1361	0	173	6	1098	0	0	0
Added Vol:	0	77	41	0	46	0	0	0	88	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	978	672	207	1407	0	173	6	1186	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	1029	708	217	1481	0	182	6	1248	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1029	708	217	1481	0	182	6	1248	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	1029	708	217	1481	0	182	6	1248	0	0	0

-----|-----|-----|-----|

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.89	0.89	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.19	0.81	1.00	2.00	0.00	0.97	0.03	1.00	0.00	0.00	0.00
Final Sat.:	0	2009	1381	1805	3610	0	1490	51	1541	0	0	0

-----|-----|-----|-----|

Capacity Analysis Module:

Vol/Sat:	0.00	0.51	0.51	0.12	0.41	0.00	0.12	0.12	0.81	0.00	0.00	0.00	
Crit Moves:	****			****			****			****			
Green/Cycle:	0.00	0.30	0.30	0.08	0.39	0.00	0.48	0.48	0.48	0.00	0.00	0.00	
Volume/Cap:	0.00	1.69	1.69	1.45	1.06	0.00	0.25	0.25	1.69	0.00	0.00	0.00	
Delay/Veh:	0.0	356	355.8	288.7	78.8	0.0	18.5	18.5	346.1	0.0	0.0	0.0	
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
AdjDel/Veh:	0.0	356	355.8	288.7	78.8	0.0	18.5	18.5	346.1	0.0	0.0	0.0	
LOS by Move:	A	F	F	F	E	A	A	B	B	F	A	A	A
HCM2kAvgQ:	0	79	79	18	39	0	4	4	113	0	0	0	

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "B"
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

Cycle (sec): 110 Critical Vol./Cap.(X): 0.884
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 32.2
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound										
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Protected			Protected			Protected			Protected										
Rights:	Include			Include			Include			Include										
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0								
Lanes:	0	0	1	1	1	2	0	2	0	0	1	0	0	1	1	0	0	0	0	0

Volume Module:

Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	1090	982	210	871	0	110	2	696	0	0	0
Added Vol:	0	28	16	0	35	0	0	0	66	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1118	998	210	906	0	110	2	762	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	1177	1050	222	954	0	116	2	802	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1177	1050	222	954	0	116	2	802	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	1177	1050	222	954	0	116	2	802	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.92	0.95	1.00	0.85	0.85	0.85	1.00	1.00	1.00
Lanes:	0.00	1.59	1.41	2.00	2.00	0.00	1.00	0.01	1.99	0.00	0.00	0.00
Final Sat.:	0	2658	2372	3502	3610	0	1615	8	3222	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.44	0.44	0.06	0.26	0.00	0.07	0.25	0.25	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.00	0.49	0.49	0.09	0.58	0.00	0.27	0.27	0.27	0.00	0.00	0.00
Volume/Cap:	0.00	0.91	0.91	0.70	0.46	0.00	0.26	0.91	0.91	0.00	0.00	0.00
Delay/Veh:	0.0	31.1	31.1	55.1	13.4	0.0	31.5	51.2	51.2	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	31.1	31.1	55.1	13.4	0.0	31.5	51.2	51.2	0.0	0.0	0.0
LOS by Move:	A	C	C	E	B	A	C	D	D	A	A	A
HCM2kAvgQ:	0	28	28	5	10	0	3	17	17	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Proposed Action "B"
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 115 Critical Vol./Cap.(X): 0.920
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 43.1
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound										
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Protected			Protected			Protected			Protected										
Rights:	Include			Include			Include			Include										
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0								
Lanes:	0	0	1	1	1	2	0	2	0	0	1	0	0	1	1	0	0	0	0	0

Volume Module:

Base Vol:	0	458	321	105	692	0	88	3	558	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	901	631	207	1361	0	173	6	1098	0	0	0
Added Vol:	0	77	41	0	46	0	0	0	88	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	978	672	207	1407	0	173	6	1186	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	1029	708	217	1481	0	182	6	1248	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1029	708	217	1481	0	182	6	1248	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	1029	708	217	1481	0	182	6	1248	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.89	0.89	0.92	0.95	1.00	0.85	0.85	0.85	1.00	1.00	1.00
Lanes:	0.00	1.78	1.22	2.00	2.00	0.00	1.00	0.01	1.99	0.00	0.00	0.00
Final Sat.:	0	3013	2072	3502	3610	0	1615	16	3218	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.34	0.34	0.06	0.41	0.00	0.11	0.39	0.39	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.00	0.36	0.36	0.09	0.45	0.00	0.41	0.41	0.41	0.00	0.00	0.00
Volume/Cap:	0.00	0.94	0.94	0.71	0.91	0.00	0.27	0.94	0.94	0.00	0.00	0.00
Delay/Veh:	0.0	45.9	45.9	58.9	37.8	0.0	22.7	45.9	45.9	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	45.9	45.9	58.9	37.8	0.0	22.7	45.9	45.9	0.0	0.0	0.0
LOS by Move:	A	D	D	E	D	A	C	D	D	A	A	A
HCM2kAvgQ:	0	26	26	5	30	0	4	26	26	0	0	0

Note: Queue reported is the number of cars per lane.

Year 2025 With Project – Alternative 1

Horseshoe Grande Property (Freeway Analysis)
Year 2025 With Project - Alternative 1
Morning Peak Hour

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): OVERFLOW Worst Case Level of Service: F[xxxxx]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns representing different traffic volumes and adjustment factors like Base Vol, Growth Adj, Initial Bse, etc.

Critical Gap Module: Table with 12 columns showing critical gap values and follow-up times for different movements.

Capacity Module: Table with 12 columns showing capacity-related metrics like Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level of Service Module: Table with 12 columns showing Level of Service (LOS) by movement, shared capacity, and approach details.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Year 2025 With Project - Alternative 1
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx]

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, Lanes.

Volume Module: Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume.

Critical Gap Module: Table with columns: Critical Gp, FollowUpTim.

Capacity Module: Table with columns: Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module: Table with columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 1
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Cycle (sec): 75 Critical Vol./Cap.(X): 0.834
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 20.6
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	0	24	24	0	0	0	0	0	0
Lanes:	2	0	1	0	0	2	0	0	1	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	738	418	0	0	1526	90	61	0	457	0	0	0
Added Vol:	0	21	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	738	439	0	0	1526	90	61	0	457	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.00	0.95	0.95	0.00	0.95	0.95	0.95
PHF Volume:	777	462	0	0	1606	0	64	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	777	462	0	0	1606	0	64	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	777	462	0	0	1606	0	64	0	0	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	3502	1900	0	0	3610	1900	1900	0	1900	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.22	0.24	0.00	0.00	0.44	0.00	0.03	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.26	0.79	0.00	0.00	0.53	0.00	0.05	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.84	0.31	0.00	0.00	0.84	0.00	0.67	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	33.1	2.2	0.0	0.0	18.5	0.0	52.0	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	33.1	2.2	0.0	0.0	18.5	0.0	52.0	0.0	0.0	0.0	0.0	0.0
LOS by Move:	C	A	A	A	B	A	D	A	A	A	A	A
HCM2kAvgQ:	12	3	0	0	19	0	3	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 1
 Evening Peak Hour - With Improvements

Level of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.819
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 17.7
 Optimal Cycle: OPTIMIZED Level of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	24	24	0	0	0	0	0	0	0
Lanes:	2	0	1	0	0	2	0	0	1	0	0	0

Volume Module:

Base Vol:	200	183	0	0	782	14	12	0	233	0	0	0
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	489	447	0	0	1912	34	29	0	570	0	0	0
Added Vol:	0	59	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	489	506	0	0	1912	34	29	0	570	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.00	0.95	0.95	0.00	0.95	0.95	0.95
PHF Volume:	515	533	0	0	2013	0	31	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	515	533	0	0	2013	0	31	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	515	533	0	0	2013	0	31	0	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	3502	1900	0	0	3610	1900	1900	0	1900	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.15	0.28	0.00	0.00	0.56	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.18	0.86	0.00	0.00	0.68	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.82	0.33	0.00	0.00	0.82	0.00	0.43	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	47.8	1.5	0.0	0.0	13.8	0.0	52.5	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	47.8	1.5	0.0	0.0	13.8	0.0	52.5	0.0	0.0	0.0	0.0	0.0
LOS by Move:	D	A	A	A	B	A	D	A	A	A	A	A
HCM2kAvgQ:	10	3	0	0	25	0	2	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 1
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 1380.8 Worst Case Level Of Service: F[11111.9]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Ignore		
Lanes:	0	0	0	0	0	1!0	0	0	2	0	0	2

Volume Module:

Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	0	0	0	340	0	51	203	1802	0	0	1086	1719
Added Vol:	0	0	0	50	0	0	0	0	0	0	21	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	390	0	51	203	1802	0	0	1107	1719
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.00
PHF Volume:	0	0	0	410	0	54	214	1897	0	0	1165	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	0	410	0	54	214	1897	0	0	1165	0

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxx	2541	3489	582	1165	xxxx	xxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	xxxx	xxxx	xxxxx	23	7	461	607	xxxx	xxxxx	xxxx	xxxx	xxxxx
Move Cap.:	xxxx	xxxx	xxxxx	17	4	461	607	xxxx	xxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	xxxx	xxxx	xxxx	24.70	0.00	0.12	0.35	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	1.6	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	14.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	B	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	19	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	58.7	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	F	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	*			F			*			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 1
 Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 851.6 Worst Case Level Of Service: F[6602.6]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Ignore		
Lanes:	0	0	0	0	0	1!0	0	0	2	0	0	2

Volume Module:

Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	0	0	0	350	0	88	54	2465	0	0	831	1433
Added Vol:	0	0	0	67	0	0	0	0	0	0	59	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	417	0	88	54	2465	0	0	890	1433
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.00
PHF Volume:	0	0	0	439	0	93	57	2594	0	0	937	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	0	0	439	0	93	57	2594	0	0	937	0

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxx	2348	3645	469	937	xxxx	xxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	xxxx	xxxx	xxxxx	31	5	547	739	xxxx	xxxxx	xxxx	xxxx	xxxxx
Move Cap.:	xxxx	xxxx	xxxxx	29	5	547	739	xxxx	xxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	xxxx	xxxx	xxxx	15.02	0.00	0.17	0.08	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.2	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	10.3	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	B	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	35	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	65.1	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	6603	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	F	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			6602.6			xxxxxx			xxxxxx		
ApproachLOS:	*			F			*			*		

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 1
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 60 Critical Vol./Cap.(X): 0.742
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 8.9
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	2	0	0	1	0	2	0	0	2

Volume Module:

Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	0	0	0	340	0	51	203	1802	0	0	1086	1719
Added Vol:	0	0	0	50	0	0	0	0	0	0	21	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	390	0	51	203	1802	0	0	1107	1719
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.00
PHF Volume:	0	0	0	410	0	54	214	1897	0	0	1165	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	410	0	54	214	1897	0	0	1165	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	410	0	54	214	1897	0	0	1165	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.85	0.22	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	3502	0	1615	410	3610	0	0	3610	1900

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.12	0.00	0.03	0.52	0.53	0.00	0.00	0.32	0.00
Crit Moves:				****				****				
Green/Cycle:	0.00	0.00	0.00	0.15	0.00	0.15	0.72	0.72	0.00	0.00	0.72	0.00
Volume/Cap:	0.00	0.00	0.00	0.77	0.00	0.22	0.73	0.73	0.00	0.00	0.45	0.00
Delay/Veh:	0.0	0.0	0.0	31.5	0.0	22.8	13.9	6.2	0.0	0.0	3.7	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	31.5	0.0	22.8	13.9	6.2	0.0	0.0	3.7	0.0
LOS by Move:	A	A	A	C	A	C	B	A	A	A	A	A
HCM2kAvgQ:	0	0	0	6	0	1	4	13	0	0	5	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 1
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.904
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 18.0
 Optimal Cycle: 110 Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	2	0	0	1	0	2	0	0	2

-----|-----|-----|-----|

Volume Module:

Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	0	0	0	350	0	88	54	2465	0	0	831	1433
Added Vol:	0	0	0	67	0	0	0	0	0	0	59	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	417	0	88	54	2465	0	0	890	1433
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.00
PHF Volume:	0	0	0	439	0	93	57	2594	0	0	937	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	439	0	93	57	2594	0	0	937	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	439	0	93	57	2594	0	0	937	0

-----|-----|-----|-----|

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.85	0.29	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	3502	0	1615	549	3610	0	0	3610	1900

-----|-----|-----|-----|

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.13	0.00	0.06	0.10	0.72	0.00	0.00	0.26	0.00
Crit Moves:				****			****					
Green/Cycle:	0.00	0.00	0.00	0.14	0.00	0.14	0.79	0.79	0.00	0.00	0.79	0.00
Volume/Cap:	0.00	0.00	0.00	0.90	0.00	0.41	0.13	0.90	0.00	0.00	0.33	0.00
Delay/Veh:	0.0	0.0	0.0	71.1	0.0	48.5	3.0	13.5	0.0	0.0	3.5	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	71.1	0.0	48.5	3.0	13.5	0.0	0.0	3.5	0.0
LOS by Move:	A	A	A	E	A	D	A	B	A	A	A	A
HCM2kAvgQ:	0	0	0	11	0	4	1	40	0	0	5	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 1
 Morning Peak Hour

Level of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 115 Critical Vol./Cap.(X): 1.297
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 148.1
 Optimal Cycle: OPTIMIZED Level of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	0	0	0	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	915	330	0	0	496	126	0	0	0	539	24	161
Added Vol:	21	0	0	0	0	0	0	0	0	26	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	936	330	0	0	496	126	0	0	0	565	24	161
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	985	348	0	0	522	133	0	0	0	595	25	170
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	985	348	0	0	522	133	0	0	0	595	25	170
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	985	348	0	0	522	133	0	0	0	595	25	170

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.59	0.41	0.00	0.00	0.00	1.00	0.13	0.87
Final Sat.:	1805	3610	0	0	2792	709	0	0	0	1550	198	1352

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.55	0.10	0.00	0.00	0.19	0.19	0.00	0.00	0.00	0.38	0.13	0.13
Crit Moves:	****			****						****		
Green/Cycle:	0.41	0.57	0.00	0.00	0.16	0.16	0.00	0.00	0.00	0.29	0.29	0.29
Volume/Cap:	1.32	0.17	0.00	0.00	1.19	1.19	0.00	0.00	0.00	1.32	0.43	0.43
Delay/Veh:	186.9	11.8	0.0	0.0	153	152.7	0.0	0.0	0.0	196.1	33.2	33.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	186.9	11.8	0.0	0.0	153	152.7	0.0	0.0	0.0	196.1	33.2	33.2
LOS by Move:	F	B	A	A	F	F	A	A	A	F	C	C
HCM2kAvgQ:	65	3	0	0	22	22	0	0	0	42	6	6

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 1
 Evening Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

 Cycle (sec): 110 Critical Vol./Cap.(X): 1.674
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 273.9
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	687	388	0	0	635	187	0	0	0	1068	75	224
Added Vol:	59	0	0	0	0	0	0	0	0	35	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	746	388	0	0	635	187	0	0	0	1103	75	224
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	785	408	0	0	669	197	0	0	0	1161	79	236
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	785	408	0	0	669	197	0	0	0	1161	79	236
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	785	408	0	0	669	197	0	0	0	1161	79	236

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.55	0.45	0.00	0.00	0.00	1.00	0.25	0.75
Final Sat.:	1805	3610	0	0	2695	793	0	0	0	1554	388	1165

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.43	0.11	0.00	0.00	0.25	0.25	0.00	0.00	0.00	0.75	0.20	0.20
Crit Moves:	****			****						****		
Green/Cycle:	0.25	0.42	0.00	0.00	0.16	0.16	0.00	0.00	0.00	0.44	0.44	0.44
Volume/Cap:	1.71	0.27	0.00	0.00	1.52	1.52	0.00	0.00	0.00	1.71	0.46	0.46
Delay/Veh:	370.1	21.1	0.0	0.0	287	287.5	0.0	0.0	0.0	355.8	22.0	22.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	370.1	21.1	0.0	0.0	287	287.5	0.0	0.0	0.0	355.8	22.0	22.0
LOS by Move:	F	C	A	A	F	F	A	A	A	F	C	C
HCM2kAvgQ:	67	5	0	0	36	36	0	0	0	103	8	8

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 1
 Morning Peak Hour - With Improvements

Level of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

 Cycle (sec): 80 Critical Vol./Cap.(X): 0.754
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 27.4
 Optimal Cycle: OPTIMIZED Level of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	2	0	2	0	0	2	1	0	0	0	0	1

Volume Module:

Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	915	330	0	0	496	126	0	0	0	539	24	161
Added Vol:	21	0	0	0	0	0	0	0	0	26	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	936	330	0	0	496	126	0	0	0	565	24	161
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	985	348	0	0	522	133	0	0	0	595	25	170
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	985	348	0	0	522	133	0	0	0	595	25	170
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	985	348	0	0	522	133	0	0	0	595	25	170

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.95	1.00	1.00	0.88	0.88	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	2.00	2.00	0.00	0.00	2.39	0.61	0.00	0.00	0.00	2.00	0.13	0.87
Final Sat.:	3502	3610	0	0	4012	1019	0	0	0	3100	198	1352

Capacity Analysis Module:

Vol/Sat:	0.28	0.10	0.00	0.00	0.13	0.13	0.00	0.00	0.00	0.19	0.13	0.13
Crit Moves:	****			****						****		
Green/Cycle:	0.34	0.57	0.00	0.00	0.23	0.23	0.00	0.00	0.00	0.23	0.23	0.23
Volume/Cap:	0.82	0.17	0.00	0.00	0.58	0.58	0.00	0.00	0.00	0.82	0.54	0.54
Delay/Veh:	28.8	8.3	0.0	0.0	28.4	28.4	0.0	0.0	0.0	34.9	27.3	27.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	28.8	8.3	0.0	0.0	28.4	28.4	0.0	0.0	0.0	34.9	27.3	27.3
LOS by Move:	C	A	A	A	C	C	A	A	A	C	C	C
HCM2kAvgQ:	14	2	0	0	6	6	0	0	0	10	5	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 1
 Evening Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

 Cycle (sec): 95 Critical Vol./Cap.(X): 0.942
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 42.0
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	2	0	2	0	0	2	0	0	0	1	1	0

Volume Module:

Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	687	388	0	0	635	187	0	0	0	1068	75	224
Added Vol:	59	0	0	0	0	0	0	0	0	35	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	746	388	0	0	635	187	0	0	0	1103	75	224
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	785	408	0	0	669	197	0	0	0	1161	79	236
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	785	408	0	0	669	197	0	0	0	1161	79	236
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	785	408	0	0	669	197	0	0	0	1161	79	236

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.95	1.00	1.00	0.95	0.85	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	2.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	2.00	0.25	0.75
Final Sat.:	3502	3610	0	0	3610	1615	0	0	0	3108	388	1165

Capacity Analysis Module:

Vol/Sat:	0.22	0.11	0.00	0.00	0.19	0.12	0.00	0.00	0.00	0.37	0.20	0.20
Crit Moves:	****			****						****		
Green/Cycle:	0.24	0.43	0.00	0.00	0.20	0.20	0.00	0.00	0.00	0.40	0.40	0.40
Volume/Cap:	0.94	0.26	0.00	0.00	0.94	0.62	0.00	0.00	0.00	0.94	0.51	0.51
Delay/Veh:	54.1	17.2	0.0	0.0	58.3	38.6	0.0	0.0	0.0	39.2	21.8	21.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	54.1	17.2	0.0	0.0	58.3	38.6	0.0	0.0	0.0	39.2	21.8	21.8
LOS by Move:	D	B	A	A	E	D	A	A	A	D	C	C
HCM2kAvgQ:	16	4	0	0	15	6	0	0	0	22	8	8

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 1
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.492
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 204.1
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0
Lanes:	0	0	1	1	0	0	1	0	2	0	0	0

Volume Module:

Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	1090	982	210	871	0	110	2	696	0	0	0
Added Vol:	0	21	12	0	26	0	0	0	50	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1111	994	210	897	0	110	2	746	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	1169	1046	222	945	0	116	2	786	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1169	1046	222	945	0	116	2	786	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	1169	1046	222	945	0	116	2	786	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.06	0.94	1.00	2.00	0.00	0.98	0.02	1.00	0.00	0.00	0.00
Final Sat.:	0	1770	1583	1805	3610	0	1513	27	1541	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.66	0.66	0.12	0.26	0.00	0.08	0.08	0.51	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.00	0.44	0.44	0.08	0.53	0.00	0.34	0.34	0.34	0.00	0.00	0.00
Volume/Cap:	0.00	1.49	1.49	1.47	0.50	0.00	0.22	0.22	1.49	0.00	0.00	0.00
Delay/Veh:	0.0	259	259.5	300.2	18.5	0.0	28.2	28.2	270.6	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	259	259.5	300.2	18.5	0.0	28.2	28.2	270.6	0.0	0.0	0.0
LOS by Move:	A	F	F	F	B	A	C	C	F	A	A	A
HCM2kAvgQ:	0	90	90	19	12	0	3	3	65	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Year 2025 With Project - Alternative 1
Evening Peak Hour

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.639
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 243.5
Optimal Cycle: OPTIMIZED Level Of Service: F

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns for different traffic movements. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with 12 columns for different traffic movements. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with 12 columns for different traffic movements. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 1
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 105 Critical Vol./Cap.(X): 0.882
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 31.4
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
	L	T	R	L	T	R	L	T	R	L	T	R
Movement:												
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0
Lanes:	0	0	1	1	1	1	2	0	2	0	0	0

Volume Module:

Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	1090	982	210	871	0	110	2	696	0	0	0
Added Vol:	0	21	12	0	26	0	0	0	50	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1111	994	210	897	0	110	2	746	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	1169	1046	222	945	0	116	2	786	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1169	1046	222	945	0	116	2	786	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	1169	1046	222	945	0	116	2	786	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.92	0.95	1.00	0.85	0.85	0.85	1.00	1.00	1.00
Lanes:	0.00	1.58	1.42	2.00	2.00	0.00	1.00	0.01	1.99	0.00	0.00	0.00
Final Sat.:	0	2655	2375	3502	3610	0	1615	8	3222	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.44	0.44	0.06	0.26	0.00	0.07	0.24	0.24	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.00	0.48	0.48	0.10	0.58	0.00	0.27	0.27	0.27	0.00	0.00	0.00
Volume/Cap:	0.00	0.91	0.91	0.66	0.45	0.00	0.27	0.91	0.91	0.00	0.00	0.00
Delay/Veh:	0.0	30.5	30.5	50.9	12.7	0.0	30.6	50.6	50.6	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	30.5	30.5	50.9	12.7	0.0	30.6	50.6	50.6	0.0	0.0	0.0
LOS by Move:	A	C	C	D	B	A	A	C	D	D	A	A
HCM2kAvgQ:	0	27	27	5	9	0	3	16	16	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 1
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 110 Critical Vol./Cap.(X): 0.912
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 41.3
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound										
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Protected			Protected			Protected			Protected										
Rights:	Include			Include			Include			Include										
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0								
Lanes:	0	0	1	1	1	2	0	2	0	0	1	0	0	1	1	0	0	0	0	0

Volume Module:

Base Vol:	0	458	321	105	692	0	88	3	558	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	901	631	207	1361	0	173	6	1098	0	0	0
Added Vol:	0	59	31	0	35	0	0	0	67	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	960	662	207	1396	0	173	6	1165	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	1010	697	217	1470	0	182	6	1226	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1010	697	217	1470	0	182	6	1226	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	1010	697	217	1470	0	182	6	1226	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.89	0.89	0.92	0.95	1.00	0.85	0.85	0.85	1.00	1.00	1.00
Lanes:	0.00	1.78	1.22	2.00	2.00	0.00	1.00	0.01	1.99	0.00	0.00	0.00
Final Sat.:	0	3009	2076	3502	3610	0	1615	16	3217	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.34	0.34	0.06	0.41	0.00	0.11	0.38	0.38	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.00	0.36	0.36	0.09	0.45	0.00	0.41	0.41	0.41	0.00	0.00	0.00
Volume/Cap:	0.00	0.94	0.94	0.68	0.91	0.00	0.28	0.94	0.94	0.00	0.00	0.00
Delay/Veh:	0.0	44.2	44.2	54.5	36.0	0.0	22.1	44.3	44.3	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	44.2	44.2	54.5	36.0	0.0	22.1	44.3	44.3	0.0	0.0	0.0
LOS by Move:	A	D	D	D	D	A	C	D	D	A	A	A
HCM2kAvgQ:	0	24	24	5	28	0	4	25	25	0	0	0

 Note: Queue reported is the number of cars per lane.

Year 2025 With Project – Alternative 2

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 2
 Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx]

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Ignore			Ignore			Include		
Lanes:	1	0	0	0	1	0	1	0	0	0	0	0

Volume Module:

Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	738	418	0	0	1526	90	61	0	457	0	0	0
Added Vol:	0	10	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	738	428	0	0	1526	90	61	0	457	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.00	0.95	0.95	0.00	0.95	0.95	0.95
PHF Volume:	777	451	0	0	1606	0	64	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	777	451	0	0	1606	0	64	0	0	0	0	0

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	xxxx	6.2	xxxxxx	xxxx	xxxxxx
FollowUpTim:	2.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	xxxx	3.3	xxxxxx	xxxx	xxxxxx

Capacity Module:

Cnflict Vol:	1606	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3611	xxxx	1606	xxxx	xxxx	xxxxxx
Potent Cap.:	412	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6	xxxx	131	xxxx	xxxx	xxxxxx
Move Cap.:	412	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	0	xxxx	131	xxxx	xxxx	xxxxxx
Volume/Cap:	1.88	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxx	xxxx	0.00	xxxx	xxxx	xxxxxx

Level Of Service Module:

2Way95thQ:	51.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	429.6	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	F	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	*			*			F			*		

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Year 2025 With Project - Alternative 2
Evening Peak Hour

Level Of Service Computation Report
2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): OVERFLOW Worst Case Level of Service: F[xxxxxx]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 13 columns representing different volume metrics and 13 rows of data.

Critical Gap Module: Table with 13 columns for gap metrics and 2 rows of data.

Capacity Module: Table with 13 columns for capacity metrics and 4 rows of data.

Level of Service Module: Table with 13 columns for LOS metrics and 8 rows of data.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 2
 Morning Peak Hour - With Improvements

Level of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Cycle (sec): 70 Critical Vol./Cap.(X): 0.846
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 20.0
 Optimal Cycle: OPTIMIZED Level of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	0	24	24	0	0	0	0	0	0
Lanes:	2	0	1	0	0	2	0	0	1	0	0	0

Volume Module:

Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	738	418	0	0	1526	90	61	0	457	0	0	0
Added Vol:	0	10	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	738	428	0	0	1526	90	61	0	457	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.00	0.95	0.95	0.00	0.95	0.95	0.95
PHF Volume:	777	451	0	0	1606	0	64	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	777	451	0	0	1606	0	64	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	777	451	0	0	1606	0	64	0	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	3502	1900	0	0	3610	1900	1900	0	1900	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.22	0.24	0.00	0.00	0.44	0.00	0.03	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.26	0.79	0.00	0.00	0.52	0.00	0.04	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.85	0.30	0.00	0.00	0.85	0.00	0.63	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	32.1	2.2	0.0	0.0	18.1	0.0	44.5	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	32.1	2.2	0.0	0.0	18.1	0.0	44.5	0.0	0.0	0.0	0.0	0.0
LOS by Move:	C	A	A	A	B	A	D	A	A	A	A	A
HCM2kAvgQ:	11	3	0	0	19	0	3	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 2
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

 Cycle (sec): 85 Critical Vol./Cap.(X): 0.839
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 17.1
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	24	24	0	0	0	0	0	0	0
Lanes:	2	0	1	0	0	2	0	0	1	0	0	0

Volume Module:

Base Vol:	200	183	0	0	782	14	12	0	233	0	0	0
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	489	447	0	0	1912	34	29	0	570	0	0	0
Added Vol:	0	20	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	489	467	0	0	1912	34	29	0	570	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.00	0.95	0.95	0.00	0.95	0.95	0.95
PHF Volume:	515	492	0	0	2013	0	31	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	515	492	0	0	2013	0	31	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	515	492	0	0	2013	0	31	0	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	3502	1900	0	0	3610	1900	1900	0	1900	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.15	0.26	0.00	0.00	0.56	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.18	0.84	0.00	0.00	0.66	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.84	0.31	0.00	0.00	0.84	0.00	0.36	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	43.9	1.6	0.0	0.0	13.6	0.0	43.8	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	43.9	1.6	0.0	0.0	13.6	0.0	43.8	0.0	0.0	0.0	0.0	0.0
LOS by Move:	D	A	A	A	B	A	D	A	A	A	A	A
HCM2kAvgQ:	9	3	0	0	23	0	1	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Year 2025 With Project - Alternative 2
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 1179.8 Worst Case Level Of Service: F[10047.4]

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, Lanes.

Volume Module: Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume.

Critical Gap Module: Table with columns: Critical Gp, FollowUpTim.

Capacity Module: Table with columns: Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module: Table with columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Year 2025 With Project - Alternative 2
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 13 columns representing different traffic volumes and adjustment factors.

Critical Gap Module: Table with 13 columns showing critical gap values and follow-up times.

Capacity Module: Table with 13 columns showing capacity-related metrics like conflict volume and potential capacity.

Level Of Service Module: Table with 13 columns showing level of service metrics like 2Way95thQ, control delay, and shared queue.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 2
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 65 Critical Vol./Cap.(X): 0.878
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 14.8
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	1	0	0	1	0	2	0	0	2

Volume Module:

Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	0	0	0	340	0	51	203	1802	0	0	1086	1719
Added Vol:	0	0	0	21	0	0	0	0	0	0	10	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	361	0	51	203	1802	0	0	1096	1719
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.00
PHF Volume:	0	0	0	380	0	54	214	1897	0	0	1153	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	380	0	54	214	1897	0	0	1153	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	380	0	54	214	1897	0	0	1153	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.95	1.00	0.85	0.20	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	1805	0	1615	382	3610	0	0	3610	1900

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.21	0.00	0.03	0.56	0.53	0.00	0.00	0.32	0.00
Crit Moves:				****			****					
Green/Cycle:	0.00	0.00	0.00	0.23	0.00	0.23	0.65	0.65	0.00	0.00	0.65	0.00
Volume/Cap:	0.00	0.00	0.00	0.91	0.00	0.15	0.86	0.81	0.00	0.00	0.49	0.00
Delay/Veh:	0.0	0.0	0.0	48.9	0.0	20.1	34.9	10.8	0.0	0.0	6.1	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	48.9	0.0	20.1	34.9	10.8	0.0	0.0	6.1	0.0
LOS by Move:	A	A	A	D	A	C	C	B	A	A	A	A
HCM2kAvgQ:	0	0	0	12	0	1	7	17	0	0	7	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 2
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 0.997
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 32.8
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	1	0	0	1	0	2	0	0	2

Volume Module:

Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	0	0	0	350	0	88	54	2465	0	0	831	1433
Added Vol:	0	0	0	14	0	0	0	0	0	0	20	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	364	0	88	54	2465	0	0	851	1433
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.00
PHF Volume:	0	0	0	383	0	93	57	2594	0	0	896	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	383	0	93	57	2594	0	0	896	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	383	0	93	57	2594	0	0	896	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.95	1.00	0.85	0.29	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	1805	0	1615	547	3610	0	0	3610	1900

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.21	0.00	0.06	0.10	0.72	0.00	0.00	0.25	0.00	
Crit Moves:				****				****					
Green/Cycle:	0.00	0.00	0.00	0.21	0.00	0.21	0.72	0.72	0.00	0.00	0.72	0.00	
Volume/Cap:	0.00	0.00	0.00	1.00	0.00	0.27	0.14	1.00	0.00	0.00	0.34	0.00	
Delay/Veh:	0.0	0.0	0.0	92.3	0.0	39.9	5.4	33.6	0.0	0.0	6.3	0.0	
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
AdjDel/Veh:	0.0	0.0	0.0	92.3	0.0	39.9	5.4	33.6	0.0	0.0	6.3	0.0	
LOS by Move:	A	A	A	F	A	D	A	C	A	A	A	A	
HCM2kAvgQ:	0	0	0	20	0	3	1	58	0	0	6	0	

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 2
 Morning Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

 Cycle (sec): 110 Critical Vol./Cap.(X): 1.288
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 142.0
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

Volume Module:

Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	915	330	0	0	496	126	0	0	0	539	24	161
Added Vol:	10	0	0	0	0	0	0	0	0	12	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	925	330	0	0	496	126	0	0	0	551	24	161
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	973	348	0	0	522	133	0	0	0	580	25	170
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	973	348	0	0	522	133	0	0	0	580	25	170
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	973	348	0	0	522	133	0	0	0	580	25	170

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.59	0.41	0.00	0.00	0.00	1.00	0.13	0.87
Final Sat.:	1805	3610	0	0	2792	709	0	0	0	1550	198	1352

Capacity Analysis Module:

Vol/Sat:	0.54	0.10	0.00	0.00	0.19	0.19	0.00	0.00	0.00	0.37	0.13	0.13
Crit Moves:	****			****						****		
Green/Cycle:	0.41	0.57	0.00	0.00	0.16	0.16	0.00	0.00	0.00	0.28	0.28	0.28
Volume/Cap:	1.32	0.17	0.00	0.00	1.14	1.14	0.00	0.00	0.00	1.32	0.44	0.44
Delay/Veh:	186.9	11.2	0.0	0.0	129	129.2	0.0	0.0	0.0	196.1	32.5	32.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	186.9	11.2	0.0	0.0	129	129.2	0.0	0.0	0.0	196.1	32.5	32.5
LOS by Move:	F	B	A	A	F	F	A	A	A	F	C	C
HCM2kAvgQ:	63	3	0	0	20	20	0	0	0	41	6	6

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 2
 Evening Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

 Cycle (sec): 105 Critical Vol./Cap.(X): 1.639
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 257.4
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

Volume Module:

Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	687	388	0	0	635	187	0	0	0	1068	75	224
Added Vol:	20	0	0	0	0	0	0	0	0	9	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	707	388	0	0	635	187	0	0	0	1077	75	224
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	744	408	0	0	669	197	0	0	0	1134	79	236
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	744	408	0	0	669	197	0	0	0	1134	79	236
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	744	408	0	0	669	197	0	0	0	1134	79	236

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.55	0.45	0.00	0.00	0.00	1.00	0.25	0.75
Final Sat.:	1805	3610	0	0	2695	793	0	0	0	1556	389	1167

Capacity Analysis Module:

Vol/Sat:	0.41	0.11	0.00	0.00	0.25	0.25	0.00	0.00	0.00	0.73	0.20	0.20
Crit Moves:	****			****						****		
Green/Cycle:	0.24	0.42	0.00	0.00	0.17	0.17	0.00	0.00	0.00	0.43	0.43	0.43
Volume/Cap:	1.69	0.27	0.00	0.00	1.45	1.45	0.00	0.00	0.00	1.69	0.47	0.47
Delay/Veh:	358.7	20.3	0.0	0.0	254	254.3	0.0	0.0	0.0	344.2	21.3	21.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	358.7	20.3	0.0	0.0	254	254.3	0.0	0.0	0.0	344.2	21.3	21.3
LOS by Move:	F	C	A	A	F	F	A	A	A	F	C	C
HCM2kAvgQ:	62	4	0	0	34	34	0	0	0	98	8	8

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 2
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

 Cycle (sec): 80 Critical Vol./Cap.(X): 0.744
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 26.9
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	2	0	2	0	0	2	1	0	0	0	1	1

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	915	330	0	0	496	126	0	0	0	539	24	161
Added Vol:	10	0	0	0	0	0	0	0	0	12	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	925	330	0	0	496	126	0	0	0	551	24	161
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	973	348	0	0	522	133	0	0	0	580	25	170
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	973	348	0	0	522	133	0	0	0	580	25	170
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	973	348	0	0	522	133	0	0	0	580	25	170

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.95	1.00	1.00	0.88	0.88	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	2.00	2.00	0.00	0.00	2.39	0.61	0.00	0.00	0.00	2.00	0.13	0.87
Final Sat.:	3502	3610	0	0	4012	1019	0	0	0	3100	198	1352

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.28	0.10	0.00	0.00	0.13	0.13	0.00	0.00	0.00	0.19	0.13	0.13
Crit Moves:	****			****						****		
Green/Cycle:	0.34	0.57	0.00	0.00	0.23	0.23	0.00	0.00	0.00	0.23	0.23	0.23
Volume/Cap:	0.81	0.17	0.00	0.00	0.58	0.58	0.00	0.00	0.00	0.81	0.54	0.54
Delay/Veh:	28.1	8.3	0.0	0.0	28.4	28.4	0.0	0.0	0.0	34.3	27.5	27.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	28.1	8.3	0.0	0.0	28.4	28.4	0.0	0.0	0.0	34.3	27.5	27.5
LOS by Move:	C	A	A	A	C	C	A	A	A	C	C	C
HCM2kAvgQ:	14	2	0	0	6	6	0	0	0	10	5	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Year 2025 With Project - Alternative 2
Evening Peak Hour - With Improvements

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

Cycle (sec): 90 Critical Vol./Cap. (X): 0.927
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 38.8
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns representing different traffic movements and 10 rows of volume-related metrics like Base Vol, Growth Adj, etc.

Saturation Flow Module: Table with 12 columns and 4 rows showing saturation flow rates and adjustments.

Capacity Analysis Module: Table with 12 columns and 10 rows showing capacity analysis metrics like Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 2
 Morning Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 1.464
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 194.3
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound											
Movement:	L	T	R	L	T	R	L	T	R	L	T	R									
Control:	Protected			Protected			Protected			Protected											
Rights:	Include			Include			Include			Include											
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0									
Lanes:	0	0	1	1	0	0	1	0	2	0	0	0	1	0	1	0	0	0	0	0	0

Volume Module:

Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	1090	982	210	871	0	110	2	696	0	0	0
Added Vol:	0	10	7	0	12	0	0	0	21	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1100	989	210	883	0	110	2	717	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	1158	1041	222	930	0	116	2	755	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1158	1041	222	930	0	116	2	755	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	1158	1041	222	930	0	116	2	755	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.05	0.95	1.00	2.00	0.00	0.98	0.02	1.00	0.00	0.00	0.00
Final Sat.:	0	1766	1588	1805	3610	0	1512	27	1539	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.66	0.66	0.12	0.26	0.00	0.08	0.08	0.49	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.00	0.45	0.45	0.08	0.53	0.00	0.34	0.34	0.34	0.00	0.00	0.00
Volume/Cap:	0.00	1.46	1.46	1.46	0.48	0.00	0.23	0.23	1.46	0.00	0.00	0.00
Delay/Veh:	0.0	246	245.7	296.3	17.9	0.0	28.8	28.8	257.9	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	246	245.7	296.3	17.9	0.0	28.8	28.8	257.9	0.0	0.0	0.0
LOS by Move:	A	F	F	F	B	A	C	C	F	A	A	A
HCM2kAvgQ:	0	87	87	19	11	0	3	3	61	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 2
 Evening Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 1.576
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 221.2
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0
Lanes:	0	0	1 1 0	1	0	2 0 0	0	1	0 1 0	0	0	0 0 0

Volume Module:

Base Vol:	0	458	321	105	692	0	88	3	558	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	901	631	207	1361	0	173	6	1098	0	0	0
Added Vol:	0	20	12	0	9	0	0	0	14	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	921	643	207	1370	0	173	6	1112	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	969	677	217	1442	0	182	6	1170	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	969	677	217	1442	0	182	6	1170	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	969	677	217	1442	0	182	6	1170	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.89	0.89	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.18	0.82	1.00	2.00	0.00	0.97	0.03	1.00	0.00	0.00	0.00
Final Sat.:	0	1993	1393	1805	3610	0	1490	51	1541	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.49	0.49	0.12	0.40	0.00	0.12	0.12	0.76	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.00	0.31	0.31	0.08	0.39	0.00	0.48	0.48	0.48	0.00	0.00	0.00
Volume/Cap:	0.00	1.59	1.59	1.45	1.03	0.00	0.26	0.26	1.59	0.00	0.00	0.00
Delay/Veh:	0.0	312	311.9	288.7	67.8	0.0	18.7	18.7	302.6	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	312	311.9	288.7	67.8	0.0	18.7	18.7	302.6	0.0	0.0	0.0
LOS by Move:	A	F	F	F	E	A	B	B	F	A	A	A
HCM2kAvgQ:	0	72	72	18	36	0	4	4	101	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 2
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 105 Critical Vol./Cap.(X): 0.867
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 30.1
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound										
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Protected			Protected			Protected			Protected										
Rights:	Include			Include			Include			Include										
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0								
Lanes:	0	0	1	1	1	2	0	2	0	0	1	0	0	1	1	0	0	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	1090	982	210	871	0	110	2	696	0	0	0
Added Vol:	0	10	7	0	12	0	0	0	21	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1100	989	210	883	0	110	2	717	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	1158	1041	222	930	0	116	2	755	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1158	1041	222	930	0	116	2	755	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	1158	1041	222	930	0	116	2	755	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.92	0.95	1.00	0.85	0.85	0.85	1.00	1.00	1.00
Lanes:	0.00	1.58	1.42	2.00	2.00	0.00	1.00	0.01	1.99	0.00	0.00	0.00
Final Sat.:	0	2649	2381	3502	3610	0	1615	9	3221	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.00	0.44	0.44	0.06	0.26	0.00	0.07	0.23	0.23	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.00	0.49	0.49	0.10	0.58	0.00	0.26	0.26	0.26	0.00	0.00	0.00
Volume/Cap:	0.00	0.89	0.89	0.66	0.44	0.00	0.27	0.89	0.89	0.00	0.00	0.00
Delay/Veh:	0.0	28.9	28.9	50.9	12.3	0.0	31.1	49.0	49.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	28.9	28.9	50.9	12.3	0.0	31.1	49.0	49.0	0.0	0.0	0.0
LOS by Move:	A	C	C	D	B	A	C	D	D	A	A	A
HCM2kAvgQ:	0	26	26	5	9	0	3	15	15	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Year 2025 With Project - Alternative 2
Evening Peak Hour - With Improvements

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.893
Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 37.6
Optimal Cycle: OPTIMIZED Level Of Service: D

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), and various traffic metrics like Min. Green and Lanes.

Table with columns: Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Table with columns: Sat/Lane, Adjustment, Lanes, Final Sat., and various saturation flow metrics.

Table with columns: Capacity Analysis Module, Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

Year 2025 With Project – Alternative 3

Horseshoe Grande Property (Freeway Analysis)
Year 2025 With Project - Alternative 3
Morning Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx]

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, Lanes.

Table with columns: Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume.

Table with columns: Critical Gap Module, Critical Gp, FollowUpTim.

Table with columns: Capacity Module, Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Table with columns: Level Of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Year 2025 With Project - Alternative 3
Evening Peak Hour

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control, Rights, and Lanes.

Volume Module: Table with 13 columns for volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume) across four bound directions.

Critical Gap Module: Table with 13 columns for critical gap and follow-up time metrics across four bound directions.

Capacity Module: Table with 13 columns for capacity metrics (Cnflct Vol, Potent Cap., Move Cap., Volume/Cap) across four bound directions.

Level Of Service Module: Table with 13 columns for LOS metrics (2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS) across four bound directions.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 3
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

 Cycle (sec): 70 Critical Vol./Cap.(X): 0.846
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 19.9
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	0	24	24	0	0	0	0	0	0
Lanes:	2	0	1	0	0	2	0	0	1	0	0	0

Volume Module:

Base Vol:	302	171	0	0	624	37	25	0	187	0	0	0
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	738	418	0	0	1526	90	61	0	457	0	0	0
Added Vol:	0	13	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	738	431	0	0	1526	90	61	0	457	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.00	0.95	0.95	0.00	0.95	0.95	0.95
PHF Volume:	777	454	0	0	1606	0	64	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	777	454	0	0	1606	0	64	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	777	454	0	0	1606	0	64	0	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	3502	1900	0	0	3610	1900	1900	0	1900	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.22	0.24	0.00	0.00	0.44	0.00	0.03	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.26	0.79	0.00	0.00	0.52	0.00	0.04	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.85	0.30	0.00	0.00	0.85	0.00	0.63	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	32.1	2.2	0.0	0.0	18.1	0.0	44.5	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	32.1	2.2	0.0	0.0	18.1	0.0	44.5	0.0	0.0	0.0	0.0	0.0
LOS by Move:	C	A	A	A	B	A	D	A	A	A	A	A
HCM2kAvgQ:	11	3	0	0	19	0	3	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 3
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #1 I-215 Freeway SB Ramps (NS) at Bonnie Drive (EW)

 Cycle (sec): 95 Critical Vol./Cap.(X): 0.825
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): 17.5
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Permitted			Permitted		
Rights:	Include			Ignore			Ignore			Include		
Min. Green:	10	24	0	24	24	0	0	0	0	0	0	0
Lanes:	2	0	1	0	0	2	0	0	1	0	0	0

Volume Module:

Base Vol:	200	183	0	0	782	14	12	0	233	0	0	0
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	489	447	0	0	1912	34	29	0	570	0	0	0
Added Vol:	0	41	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	489	488	0	0	1912	34	29	0	570	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.00	0.95	0.95	0.00	0.95	0.95	0.95
PHF Volume:	515	514	0	0	2013	0	31	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	515	514	0	0	2013	0	31	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Final Volume:	515	514	0	0	2013	0	31	0	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	1.00	0.00	0.00	2.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	3502	1900	0	0	3610	1900	1900	0	1900	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.15	0.27	0.00	0.00	0.56	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.18	0.85	0.00	0.00	0.68	0.00	0.02	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.82	0.32	0.00	0.00	0.83	0.00	0.41	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	46.4	1.5	0.0	0.0	13.7	0.0	49.6	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	46.4	1.5	0.0	0.0	13.7	0.0	49.6	0.0	0.0	0.0	0.0	0.0
LOS by Move:	D	A	A	A	B	A	D	A	A	A	A	A
HCM2kAvgQ:	10	3	0	0	25	0	1	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Year 2025 With Project - Alternative 3
Morning Peak Hour

Level Of Service Computation Report
2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 1157.2 Worst Case Level Of Service: F[9970.0]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control, Rights, and Lanes.

Volume Module: Table with 12 columns for volume components. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with 12 columns for gap metrics. Rows include Critical Gp and FollowUpTim.

Capacity Module: Table with 12 columns for capacity metrics. Rows include Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with 12 columns for LOS metrics. Rows include 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
Year 2025 With Project - Alternative 3
Evening Peak Hour

Level Of Service Computation Report
2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Average Delay (sec/veh): 739.8 Worst Case Level Of Service: F[5979.4]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement (L-T-R), Control (Stop Sign, Uncontrolled), Rights (Include, Ignore), and Lanes (0 0 0 0 0).

Volume Module: Table with 13 columns for volume metrics. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with 13 columns for gap metrics. Rows include Critical Gp and FollowUpTim.

Capacity Module: Table with 13 columns for capacity metrics. Rows include Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with 13 columns for LOS metrics. Rows include 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 3
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

 Cycle (sec): 60 Critical Vol./Cap.(X): 0.730
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 8.3
 Optimal Cycle: OPTIMIZED Level Of Service: A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	2	0	0	1	0	2	0	0	2

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	0	0	139	0	21	83	737	0	0	444	703
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	0	0	0	340	0	51	203	1802	0	0	1086	1719
Added Vol:	0	0	0	16	0	0	0	0	0	0	13	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	356	0	51	203	1802	0	0	1099	1719
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.00
PHF Volume:	0	0	0	375	0	54	214	1897	0	0	1156	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	375	0	54	214	1897	0	0	1156	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	375	0	54	214	1897	0	0	1156	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.85	0.22	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	3502	0	1615	418	3610	0	0	3610	1900

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.00	0.00	0.00	0.11	0.00	0.03	0.51	0.53	0.00	0.00	0.32	0.00
Crit Moves:				****				****				
Green/Cycle:	0.00	0.00	0.00	0.14	0.00	0.14	0.72	0.72	0.00	0.00	0.72	0.00
Volume/Cap:	0.00	0.00	0.00	0.75	0.00	0.23	0.71	0.73	0.00	0.00	0.44	0.00
Delay/Veh:	0.0	0.0	0.0	30.8	0.0	23.3	12.1	5.9	0.0	0.0	3.5	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	30.8	0.0	23.3	12.1	5.9	0.0	0.0	3.5	0.0
LOS by Move:	A	A	A	C	A	C	B	A	A	A	A	A
HCM2kAvgQ:	0	0	0	5	0	1	4	12	0	0	5	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 3
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 I-10 Freeway NB Ramps (NS) at SR-74 (EW)

Cycle (sec): 90 Critical Vol./Cap.(X): 0.918
 Loss Time (sec): 8 (Y+R=4.0 sec) Average Delay (sec/veh): 16.1
 Optimal Cycle: OPTIMIZED Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Ignore		
Min. Green:	0	0	0	0	0	0	24	24	0	0	24	24
Lanes:	0	0	0	2	0	0	1	0	2	0	0	2

Volume Module:

Base Vol:	0	0	0	143	0	36	22	1008	0	0	340	586
Growth Adj:	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45	2.45
Initial Bse:	0	0	0	350	0	88	54	2465	0	0	831	1433
Added Vol:	0	0	0	41	0	0	0	0	0	0	41	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	391	0	88	54	2465	0	0	872	1433
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.00
PHF Volume:	0	0	0	411	0	93	57	2594	0	0	918	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	411	0	93	57	2594	0	0	918	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Volume:	0	0	0	411	0	93	57	2594	0	0	918	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.85	0.30	0.95	1.00	1.00	0.95	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	1.00	2.00	0.00	0.00	2.00	1.00
Final Sat.:	0	0	0	3502	0	1615	564	3610	0	0	3610	1900

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.12	0.00	0.06	0.10	0.72	0.00	0.00	0.25	0.00	
Crit Moves:				****				****					
Green/Cycle:	0.00	0.00	0.00	0.13	0.00	0.13	0.78	0.78	0.00	0.00	0.78	0.00	
Volume/Cap:	0.00	0.00	0.00	0.92	0.00	0.45	0.13	0.92	0.00	0.00	0.32	0.00	
Delay/Veh:	0.0	0.0	0.0	62.4	0.0	37.8	2.5	12.9	0.0	0.0	2.9	0.0	
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
AdjDel/Veh:	0.0	0.0	0.0	62.4	0.0	37.8	2.5	12.9	0.0	0.0	2.9	0.0	
LOS by Move:	A	A	A	E	A	D	A	B	A	A	A	A	
HCM2kAvgQ:	0	0	0	9	0	3	0	34	0	0	4	0	

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 3
 Morning Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

 Cycle (sec): 110 Critical Vol./Cap.(X): 1.286
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 141.6
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	0	0	0	0	1	0

Volume Module:

Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	915	330	0	0	496	126	0	0	0	539	24	161
Added Vol:	13	0	0	0	0	0	0	0	0	8	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	928	330	0	0	496	126	0	0	0	547	24	161
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	977	348	0	0	522	133	0	0	0	576	25	170
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	977	348	0	0	522	133	0	0	0	576	25	170
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	977	348	0	0	522	133	0	0	0	576	25	170

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.59	0.41	0.00	0.00	0.00	1.00	0.13	0.87
Final Sat.:	1805	3610	0	0	2792	709	0	0	0	1550	198	1352

Capacity Analysis Module:

Vol/Sat:	0.54	0.10	0.00	0.00	0.19	0.19	0.00	0.00	0.00	0.37	0.13	0.13
Crit Moves:	****			****						****		
Green/Cycle:	0.41	0.57	0.00	0.00	0.16	0.16	0.00	0.00	0.00	0.28	0.28	0.28
Volume/Cap:	1.32	0.17	0.00	0.00	1.14	1.14	0.00	0.00	0.00	1.32	0.45	0.45
Delay/Veh:	186.2	11.1	0.0	0.0	129	129.2	0.0	0.0	0.0	195.6	32.7	32.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	186.2	11.1	0.0	0.0	129	129.2	0.0	0.0	0.0	195.6	32.7	32.7
LOS by Move:	F	B	A	A	F	F	A	A	A	F	C	C
HCM2kAvgQ:	63	3	0	0	20	20	0	0	0	40	6	6

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 3
 Evening Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

 Cycle (sec): 105 Critical Vol./Cap.(X): 1.663
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 265.7
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	1	0	2	0	0	1	1	0	0	0	1	0

Volume Module:

Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	687	388	0	0	635	187	0	0	0	1068	75	224
Added Vol:	41	0	0	0	0	0	0	0	0	20	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	728	388	0	0	635	187	0	0	0	1088	75	224
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	766	408	0	0	669	197	0	0	0	1145	79	236
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	766	408	0	0	669	197	0	0	0	1145	79	236
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	766	408	0	0	669	197	0	0	0	1145	79	236

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.92	0.92	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	1.00	2.00	0.00	0.00	1.55	0.45	0.00	0.00	0.00	1.00	0.25	0.75
Final Sat.:	1805	3610	0	0	2695	793	0	0	0	1554	388	1165

Capacity Analysis Module:

Vol/Sat:	0.42	0.11	0.00	0.00	0.25	0.25	0.00	0.00	0.00	0.74	0.20	0.20
Crit Moves:	****			****						****		
Green/Cycle:	0.25	0.42	0.00	0.00	0.17	0.17	0.00	0.00	0.00	0.43	0.43	0.43
Volume/Cap:	1.72	0.27	0.00	0.00	1.45	1.45	0.00	0.00	0.00	1.72	0.47	0.47
Delay/Veh:	371.8	20.1	0.0	0.0	254	254.3	0.0	0.0	0.0	357.9	21.6	21.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	371.8	20.1	0.0	0.0	254	254.3	0.0	0.0	0.0	357.9	21.6	21.6
LOS by Move:	F	C	A	A	F	F	A	A	A	F	C	C
HCM2kAvgQ:	65	4	0	0	34	34	0	0	0	100	8	8

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 3
 Morning Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

 Cycle (sec): 80 Critical Vol./Cap. (X): 0.743
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 26.9
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	2	0	2	0	0	2	1	0	0	0	0	1

Volume Module:

Base Vol:	465	168	0	0	252	64	0	0	0	274	12	82
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	915	330	0	0	496	126	0	0	0	539	24	161
Added Vol:	13	0	0	0	0	0	0	0	0	8	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	928	330	0	0	496	126	0	0	0	547	24	161
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	977	348	0	0	522	133	0	0	0	576	25	170
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	977	348	0	0	522	133	0	0	0	576	25	170
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	977	348	0	0	522	133	0	0	0	576	25	170

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.95	1.00	1.00	0.88	0.88	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	2.00	2.00	0.00	0.00	2.39	0.61	0.00	0.00	0.00	2.00	0.13	0.87
Final Sat.:	3502	3610	0	0	4012	1019	0	0	0	3100	198	1352

Capacity Analysis Module:

Vol/Sat:	0.28	0.10	0.00	0.00	0.13	0.13	0.00	0.00	0.00	0.19	0.13	0.13
Crit Moves:	****			****						****		
Green/Cycle:	0.35	0.57	0.00	0.00	0.23	0.23	0.00	0.00	0.00	0.23	0.23	0.23
Volume/Cap:	0.81	0.17	0.00	0.00	0.58	0.58	0.00	0.00	0.00	0.81	0.55	0.55
Delay/Veh:	27.9	8.2	0.0	0.0	28.4	28.4	0.0	0.0	0.0	34.3	27.6	27.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	27.9	8.2	0.0	0.0	28.4	28.4	0.0	0.0	0.0	34.3	27.6	27.6
LOS by Move:	C	A	A	A	C	C	A	A	A	C	C	C
HCM2kAvgQ:	14	2	0	0	6	6	0	0	0	9	5	5

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 3
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #3 Beaumont Avenue (SR-79) (NS) at I-10 Freeway WB Ramps (EW)

 Cycle (sec): 90 Critical Vol./Cap.(X): 0.940
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 40.3
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	10	18	0	0	18	18	0	0	0	0	0	0
Lanes:	2	0	2	0	0	2	0	0	0	1	1	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	349	197	0	0	323	95	0	0	0	543	38	114
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	687	388	0	0	635	187	0	0	0	1068	75	224
Added Vol:	41	0	0	0	0	0	0	0	0	20	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	728	388	0	0	635	187	0	0	0	1088	75	224
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	766	408	0	0	669	197	0	0	0	1145	79	236
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	766	408	0	0	669	197	0	0	0	1145	79	236
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	766	408	0	0	669	197	0	0	0	1145	79	236

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.95	1.00	1.00	0.95	0.85	1.00	1.00	1.00	0.82	0.82	0.82
Lanes:	2.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	2.00	0.25	0.75
Final Sat.:	3502	3610	0	0	3610	1615	0	0	0	3108	388	1165

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.22	0.11	0.00	0.00	0.19	0.12	0.00	0.00	0.00	0.37	0.20	0.20
Crit Moves:	****			****						****		
Green/Cycle:	0.23	0.43	0.00	0.00	0.20	0.20	0.00	0.00	0.00	0.39	0.39	0.39
Volume/Cap:	0.94	0.26	0.00	0.00	0.93	0.61	0.00	0.00	0.00	0.94	0.52	0.52
Delay/Veh:	53.3	16.5	0.0	0.0	53.3	36.2	0.0	0.0	0.0	38.5	21.1	21.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	53.3	16.5	0.0	0.0	53.3	36.2	0.0	0.0	0.0	38.5	21.1	21.1
LOS by Move:	D	B	A	A	D	D	A	A	A	D	C	C
HCM2kAvgQ:	15	4	0	0	14	6	0	0	0	21	7	7

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 3
 Morning Peak Hour

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 120 Critical Vol./Cap.(X): 1.461
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 193.4
 Optimal Cycle: OPTIMIZED Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound										
	L	T	R	L	T	R	L	T	R	L	T	R								
Movement:																				
Control:	Protected			Protected			Protected			Protected										
Rights:	Include			Include			Include			Include										
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0								
Lanes:	0	0	1	1	0	1	0	2	0	0	0	1	0	1	0	0	0	0	0	0

Volume Module:

Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	1090	982	210	871	0	110	2	696	0	0	0
Added Vol:	0	13	7	0	8	0	0	0	16	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1103	989	210	879	0	110	2	712	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	1161	1041	222	926	0	116	2	750	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1161	1041	222	926	0	116	2	750	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	1161	1041	222	926	0	116	2	750	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.05	0.95	1.00	2.00	0.00	0.98	0.02	1.00	0.00	0.00	0.00
Final Sat.:	0	1768	1585	1805	3610	0	1512	27	1539	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.66	0.66	0.12	0.26	0.00	0.08	0.08	0.49	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.00	0.45	0.45	0.08	0.53	0.00	0.33	0.33	0.33	0.00	0.00	0.00
Volume/Cap:	0.00	1.46	1.46	1.46	0.48	0.00	0.23	0.23	1.46	0.00	0.00	0.00
Delay/Veh:	0.0	244	244.3	295.0	17.8	0.0	28.9	28.9	256.7	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	244	244.3	295.0	17.8	0.0	28.9	28.9	256.7	0.0	0.0	0.0
LOS by Move:	A	F	F	F	B	A	C	C	F	A	A	A
HCM2kAvgQ:	0	87	87	19	11	0	3	3	60	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 3
 Evening Peak Hour

Level of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

Cycle (sec): 120 Critical Vol./Cap.(X): 1.609
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 232.6
 Optimal Cycle: OPTIMIZED Level of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0
Lanes:	0	0	1	1	0	0	1	0	2	0	0	0

Volume Module:	North Bound			South Bound			East Bound			West Bound		
Base Vol:	0	458	321	105	692	0	88	3	558	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	901	631	207	1361	0	173	6	1098	0	0	0
Added Vol:	0	41	20	0	20	0	0	0	41	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	942	651	207	1381	0	173	6	1139	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	992	686	217	1454	0	182	6	1199	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	992	686	217	1454	0	182	6	1199	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	992	686	217	1454	0	182	6	1199	0	0	0

Saturation Flow Module:	North Bound			South Bound			East Bound			West Bound		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.89	0.89	0.95	0.95	1.00	0.81	0.81	0.81	1.00	1.00	1.00
Lanes:	0.00	1.18	0.82	1.00	2.00	0.00	0.97	0.03	1.00	0.00	0.00	0.00
Final Sat.:	0	2004	1386	1805	3610	0	1488	51	1539	0	0	0

Capacity Analysis Module:	North Bound			South Bound			East Bound			West Bound		
Vol/Sat:	0.00	0.49	0.49	0.12	0.40	0.00	0.12	0.12	0.78	0.00	0.00	0.00
Crit Moves:	****			****			****			****		
Green/Cycle:	0.00	0.30	0.30	0.08	0.39	0.00	0.48	0.48	0.48	0.00	0.00	0.00
Volume/Cap:	0.00	1.63	1.63	1.45	1.04	0.00	0.26	0.26	1.63	0.00	0.00	0.00
Delay/Veh:	0.0	328	327.9	288.7	71.5	0.0	18.6	18.6	318.3	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	328	327.9	288.7	71.5	0.0	18.6	18.6	318.3	0.0	0.0	0.0
LOS by Move:	A	F	F	F	E	A	A	B	B	F	A	A
HCM2kAvgQ:	0	74	74	18	37	0	4	4	105	0	0	0

Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 3
 Morning Peak Hour - With Improvements

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

Cycle (sec): 105 Critical Vol./Cap.(X): 0.866
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 29.9
 Optimal Cycle: OPTIMIZED Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0
Lanes:	0	0	1	1	1	1	2	0	2	0	0	0

Volume Module:

Base Vol:	0	554	499	107	443	0	56	1	354	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	1090	982	210	871	0	110	2	696	0	0	0
Added Vol:	0	13	7	0	8	0	0	0	16	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1103	989	210	879	0	110	2	712	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	1161	1041	222	926	0	116	2	750	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1161	1041	222	926	0	116	2	750	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1161	1041	222	926	0	116	2	750	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.88	0.88	0.92	0.95	1.00	0.85	0.85	0.85	1.00	1.00	1.00
Lanes:	0.00	1.58	1.42	2.00	2.00	0.00	1.00	0.01	1.99	0.00	0.00	0.00
Final Sat.:	0	2653	2378	3502	3610	0	1615	9	3221	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.44	0.44	0.06	0.26	0.00	0.07	0.23	0.23	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.00	0.49	0.49	0.10	0.59	0.00	0.26	0.26	0.26	0.00	0.00	0.00
Volume/Cap:	0.00	0.89	0.89	0.66	0.44	0.00	0.27	0.89	0.89	0.00	0.00	0.00
Delay/Veh:	0.0	28.7	28.7	50.9	12.2	0.0	31.2	49.0	49.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	28.7	28.7	50.9	12.2	0.0	31.2	49.0	49.0	0.0	0.0	0.0
LOS by Move:	A	C	C	D	B	A	C	D	D	A	A	A
HCM2kAvgQ:	0	26	26	5	9	0	3	15	15	0	0	0

 Note: Queue reported is the number of cars per lane.

Horseshoe Grande Property (Freeway Analysis)
 Year 2025 With Project - Alternative 3
 Evening Peak Hour - With Improvements

Level Of Service Computation Report
 2000 HCM Operations Method (Future Volume Alternative)

 Intersection #4 Beaumont Avenue (SR-79) (NS) at I-10 Freeway EB Ramps (EW)

 Cycle (sec): 105 Critical Vol./Cap.(X): 0.902
 Loss Time (sec): 16 (Y+R=4.0 sec) Average Delay (sec/veh): 39.3
 Optimal Cycle: OPTIMIZED Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound										
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Protected			Protected			Protected			Protected										
Rights:	Include			Include			Include			Include										
Min. Green:	0	18	18	10	18	0	0	0	0	0	0	0								
Lanes:	0	0	1	1	1	2	0	2	0	0	1	0	0	1	1	0	0	0	0	0

Volume Module:

Base Vol:	0	458	321	105	692	0	88	3	558	0	0	0
Growth Adj:	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97	1.97
Initial Bse:	0	901	631	207	1361	0	173	6	1098	0	0	0
Added Vol:	0	41	20	0	20	0	0	0	41	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	942	651	207	1381	0	173	6	1139	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	0	992	686	217	1454	0	182	6	1199	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	992	686	217	1454	0	182	6	1199	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	0	992	686	217	1454	0	182	6	1199	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.89	0.89	0.92	0.95	1.00	0.85	0.85	0.85	1.00	1.00	1.00
Lanes:	0.00	1.77	1.23	2.00	2.00	0.00	1.00	0.01	1.99	0.00	0.00	0.00
Final Sat.:	0	3006	2079	3502	3610	0	1615	17	3217	0	0	0

Capacity Analysis Module:

Vol/Sat:	0.00	0.33	0.33	0.06	0.40	0.00	0.11	0.37	0.37	0.00	0.00	0.00
Crit Moves:	****			****			****					
Green/Cycle:	0.00	0.35	0.35	0.10	0.45	0.00	0.40	0.40	0.40	0.00	0.00	0.00
Volume/Cap:	0.00	0.93	0.93	0.65	0.90	0.00	0.28	0.93	0.93	0.00	0.00	0.00
Delay/Veh:	0.0	42.3	42.3	50.4	33.8	0.0	21.6	42.6	42.6	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	42.3	42.3	50.4	33.8	0.0	21.6	42.6	42.6	0.0	0.0	0.0
LOS by Move:	A	D	D	D	C	A	C	D	D	A	A	A
HCM2kAvgQ:	0	23	23	5	26	0	4	23	23	0	0	0

Note: Queue reported is the number of cars per lane.

APPENDIX D

Traffic Signal Warrant Worksheets

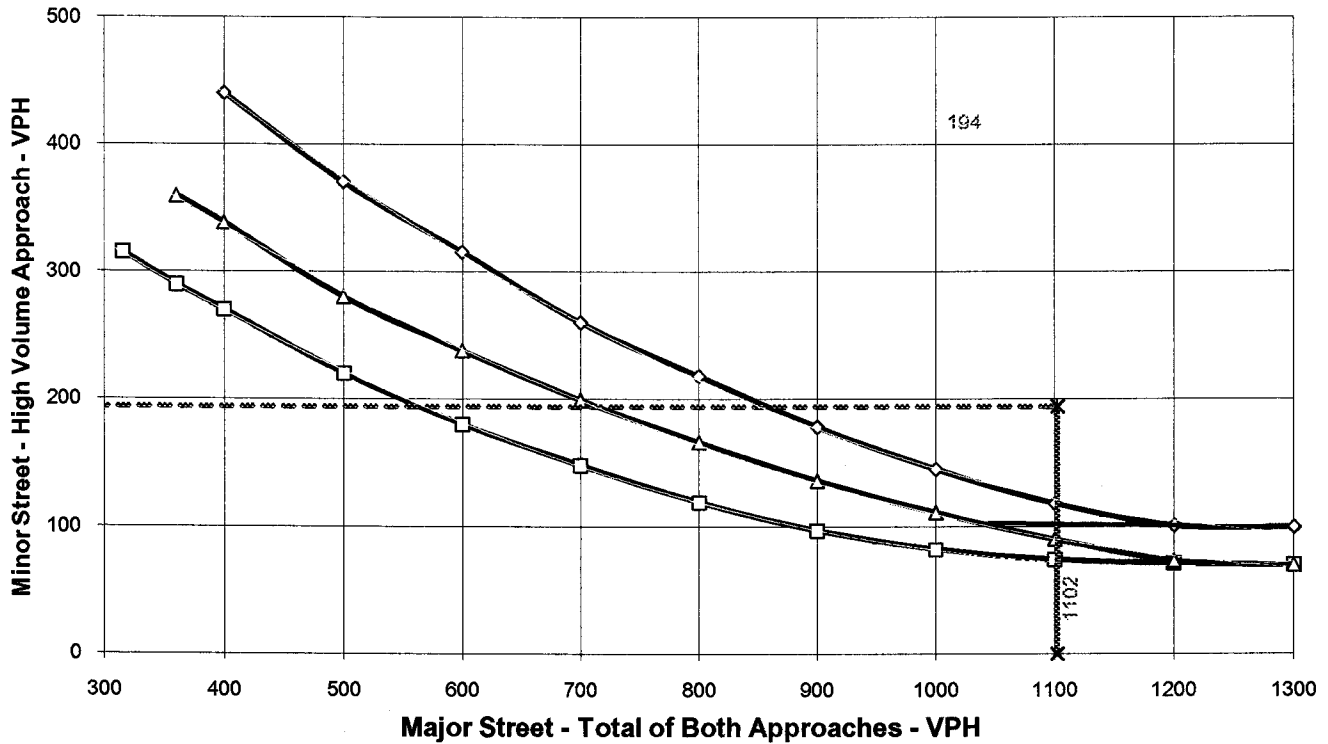
PEAK HOUR VOLUME WARRANT (Rural Areas)

Existing

Major Street Name = **State Street/Gilman Springs Road** of Both Approaches (VPH) = **1102**
Number of Approach Lanes Major Street = **1**

Minor Street Name = **Soboba Road** High Volume Approach (VPH) = **194**
Number of Approach Lanes Minor Street = **1**

WARRANTED FOR A SIGNAL



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- X— Major Street Approaches
- *— Minor Street Approaches

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

PEAK HOUR VOLUME WARRANT (Rural Areas)

Existing

Major Street Name = **Ramona Expressway**

Total of Both Approaches (VPH) = **1327**

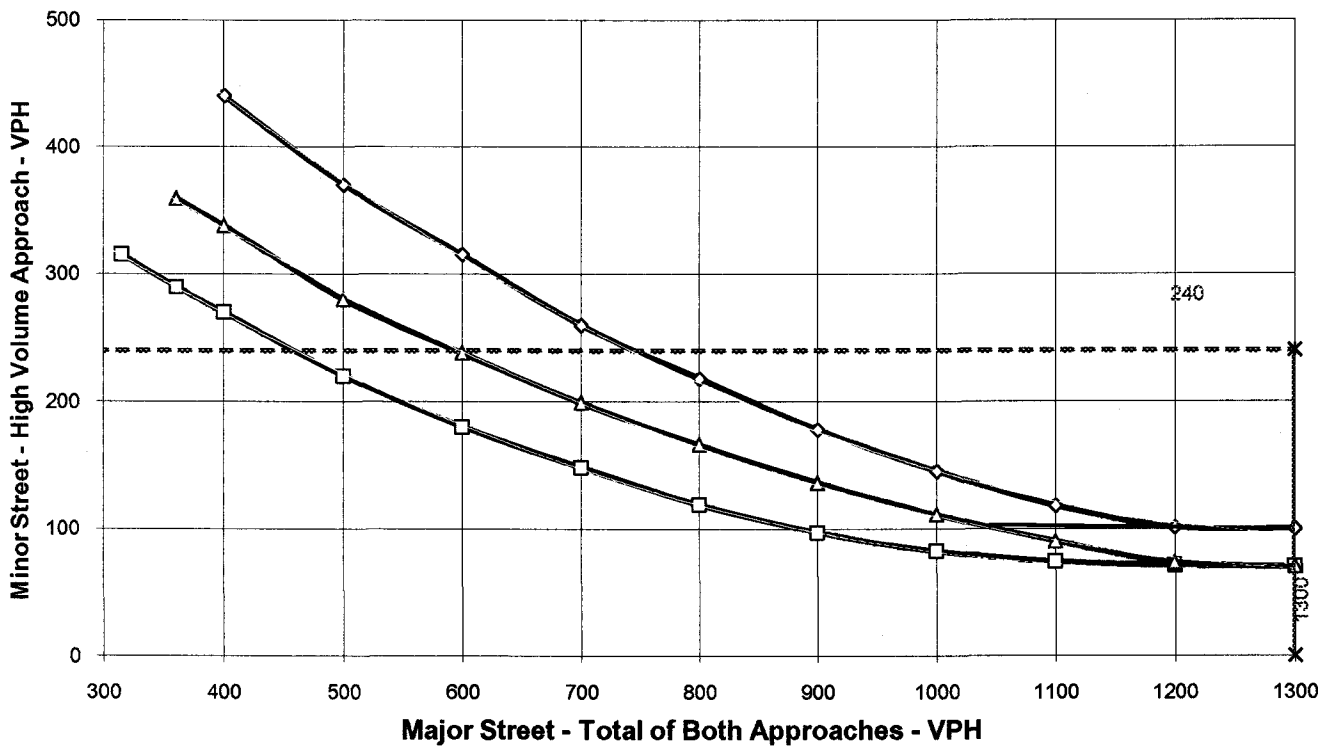
Number of Approach Lanes Major Street = **2**

Minor Street Name = **7th Street**

High Volume Approach (VPH) = **240**

Number of Approach Lanes Minor Street = **1**

WARRANTED FOR A SIGNAL



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- ×— Major Street Approaches
- *--- Minor Street Approaches

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

PEAK HOUR VOLUME WARRANT (Rural Areas)

Existing

Major Street Name = **Soboba Road**

Total of Both Approaches (VPH) = **614**

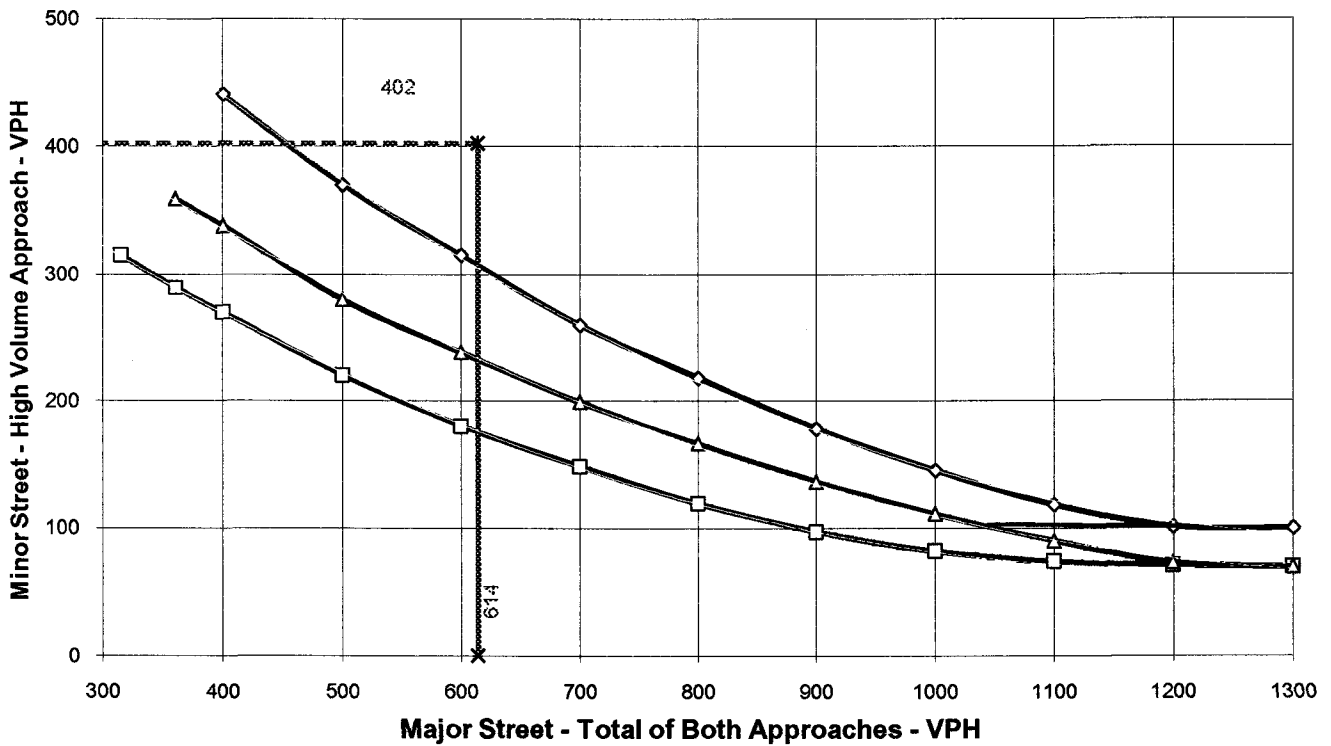
Number of Approach Lanes Major Street = **2**

Minor Street Name = **Lake Park Drive**

High Volume Approach (VPH) = **402**

Number of Approach Lanes Minor Street = **1**

WARRANTED FOR A SIGNAL



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- x— Major Street Approaches
- *--- Minor Street Approaches

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

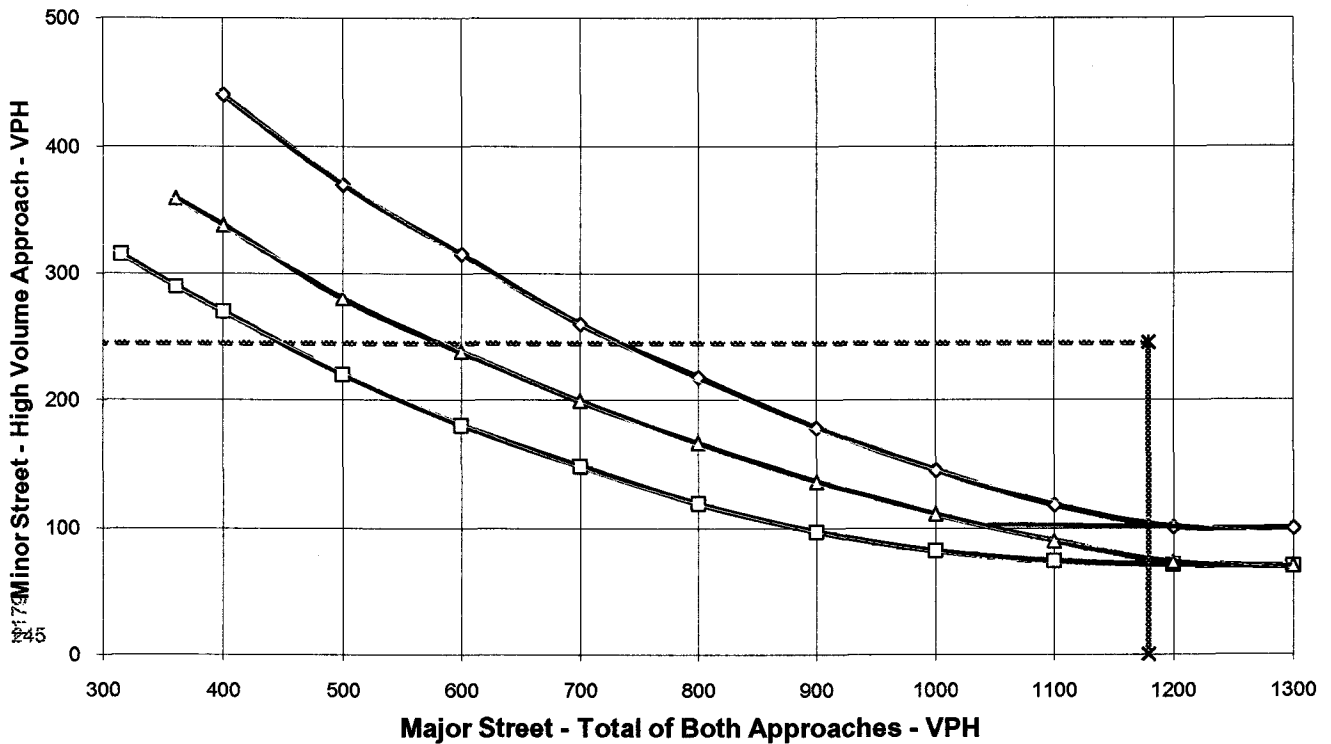
PEAK HOUR VOLUME WARRANT (Rural Areas)

Existing

Major Street Name = **I-215 Freeway SB Ramps** Total of Both Approaches (VPH) = **1179**
 Number of Approach Lanes Major Street = **1**

Minor Street Name = **Bonnie Drive** High Volume Approach (VPH) = **245**
 Number of Approach Lanes Minor Street = **1**

WARRANTED FOR A SIGNAL



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- x— Major Street Approaches
- *--- Minor Street Approaches

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

PEAK HOUR VOLUME WARRANT (Rural Areas)

Existing

Major Street Name = **SR-74**

Total of Both Approaches (VPH) = **1956**

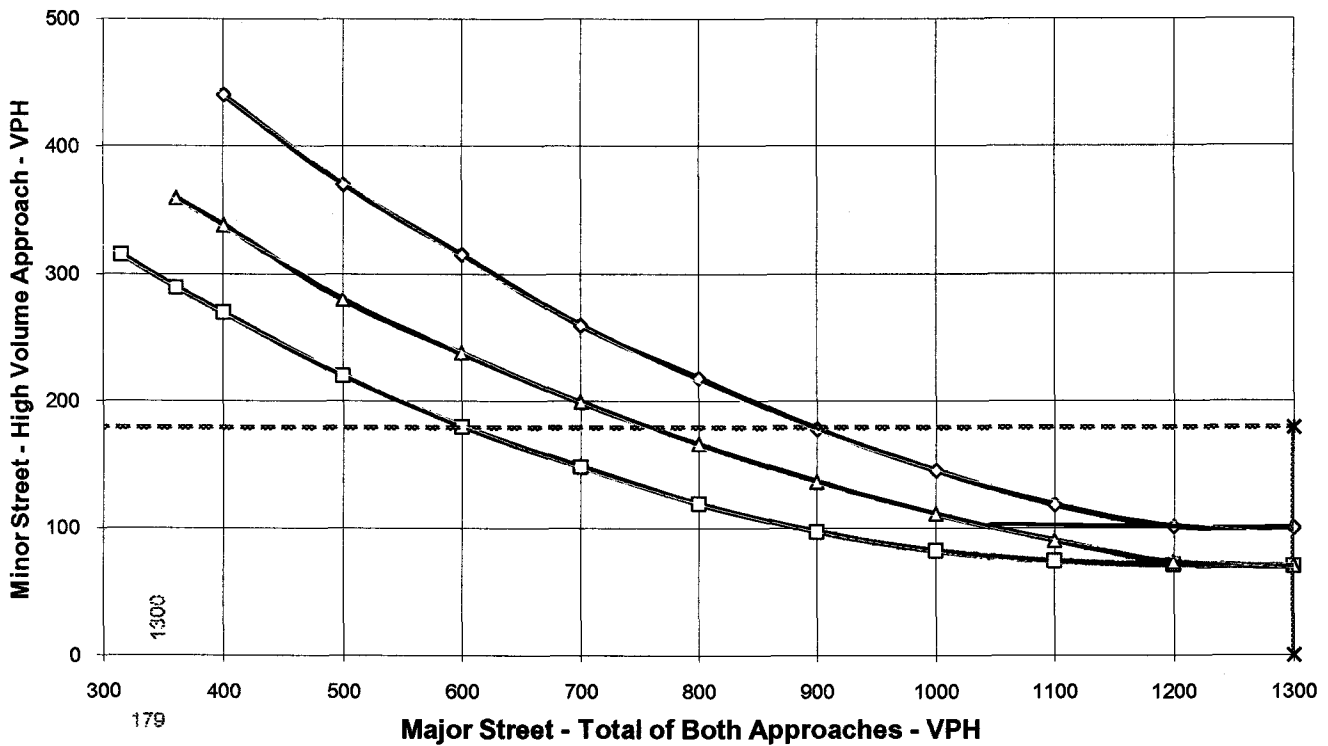
Number of Approach Lanes Major Street = **1**

Minor Street Name = **I-215 Freeway NB Ramps**

High Volume Approach (VPH) = **179**

Number of Approach Lanes Minor Street = **1**

WARRANTED FOR A SIGNAL



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- ×— Major Street Approaches
- *--- Minor Street Approaches

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

PEAK HOUR VOLUME WARRANT (Rural Areas)

Opening Year (2010) Without Project - No Action

Major Street Name = **Mountain Avenue**

Total of Both Approaches (VPH) = **1082**

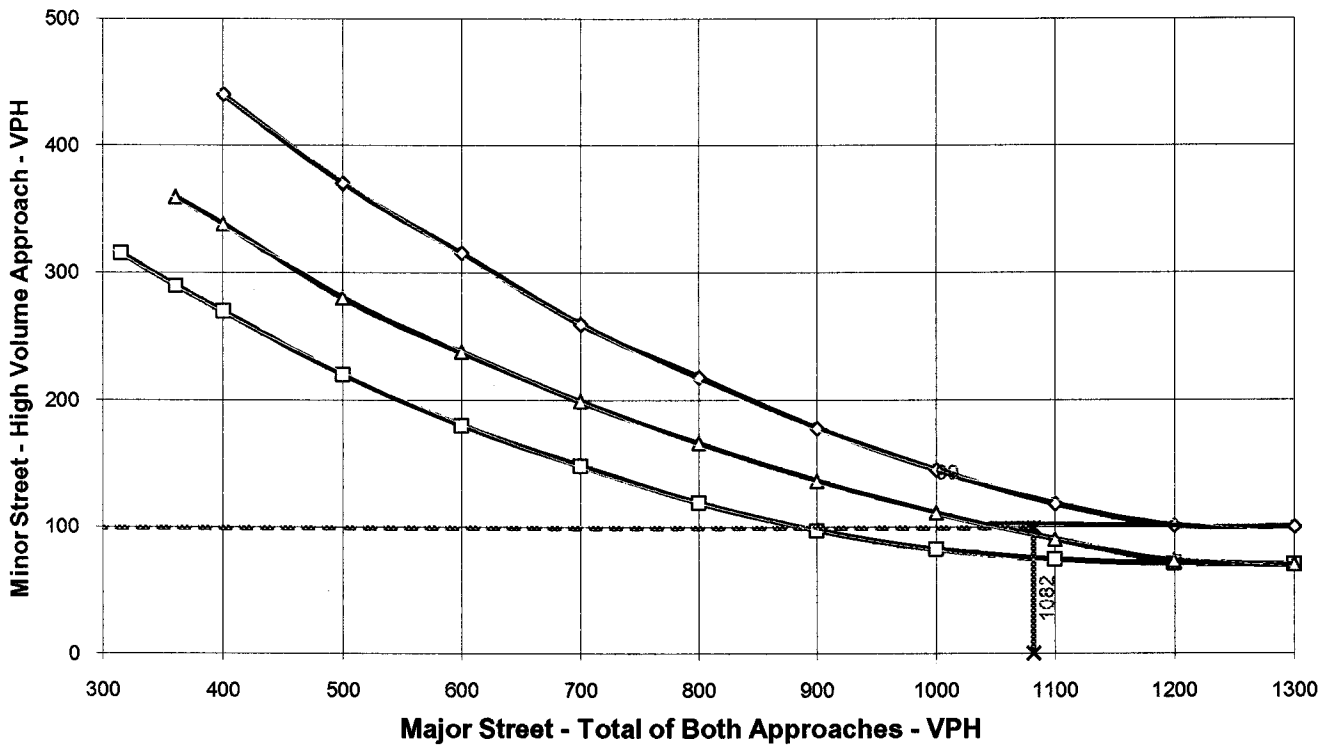
Number of Approach Lanes Major Street = **1**

Minor Street Name = **Soboba Street**

High Volume Approach (VPH) = **99**

Number of Approach Lanes Minor Street = **2**

WARRANTED FOR A SIGNAL



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- ×— Major Street Approaches
- *--- Minor Street Approaches

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

PEAK HOUR VOLUME WARRANT (Rural Areas)

Opening Year (2010) With Project - Proposed Action "A"

Major Street Name = **Lake Park Drive**

Total of Both Approaches (VPH) = **2222**

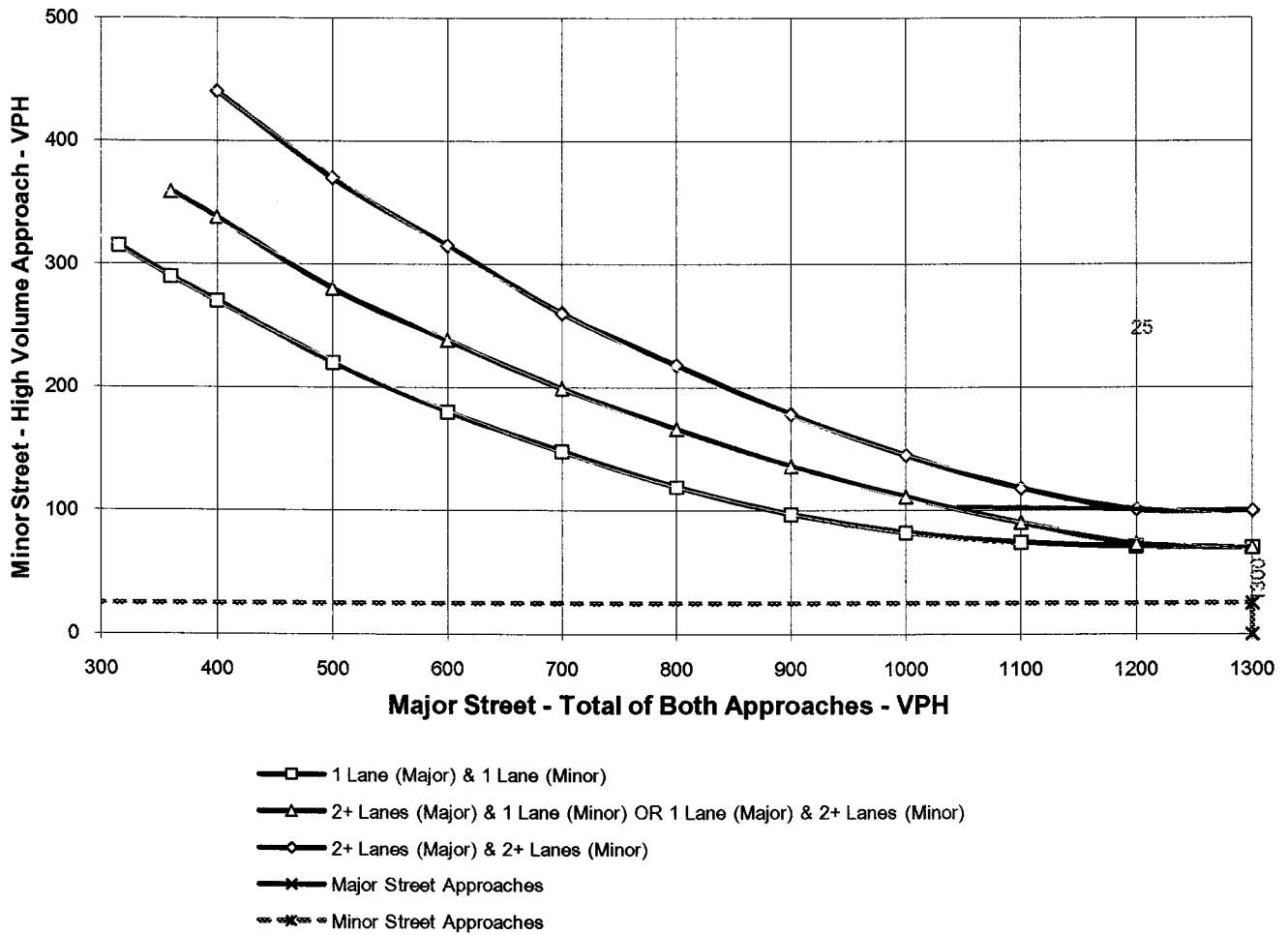
Number of Approach Lanes Major Street = **1**

Minor Street Name = **Soboba Springs Drive**

High Volume Approach (VPH) = **25**

Number of Approach Lanes Minor Street = **1**

SIGNAL WARRANT NOT SATISFIED



**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

PEAK HOUR VOLUME WARRANT (Rural Areas)

Opening Year (2010) With Project - Proposed Action "B"

Major Street Name = **Lake Park Drive**

Total of Both Approaches (VPH) = **2186**

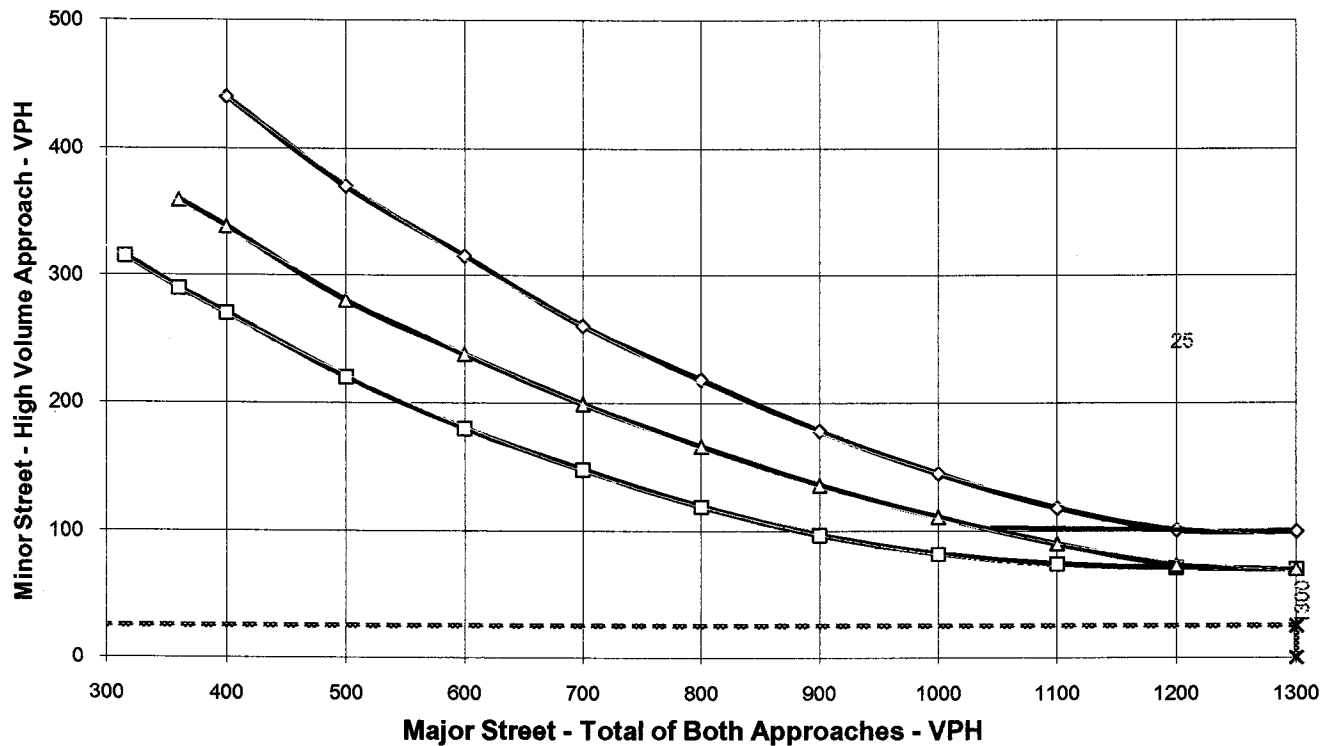
Number of Approach Lanes Major Street = **1**

Minor Street Name = **Soboba Springs Drive**

High Volume Approach (VPH) = **25**

Number of Approach Lanes Minor Street = **1**

SIGNAL WARRANT NOT SATISFIED



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- ×— Major Street Approaches
- *— Minor Street Approaches

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

PEAK HOUR VOLUME WARRANT (Rural Areas)

Opening Year (2010) With Project - Alternative 1

Major Street Name = **Lake Park Drive**

Total of Both Approaches (VPH) = **1904**

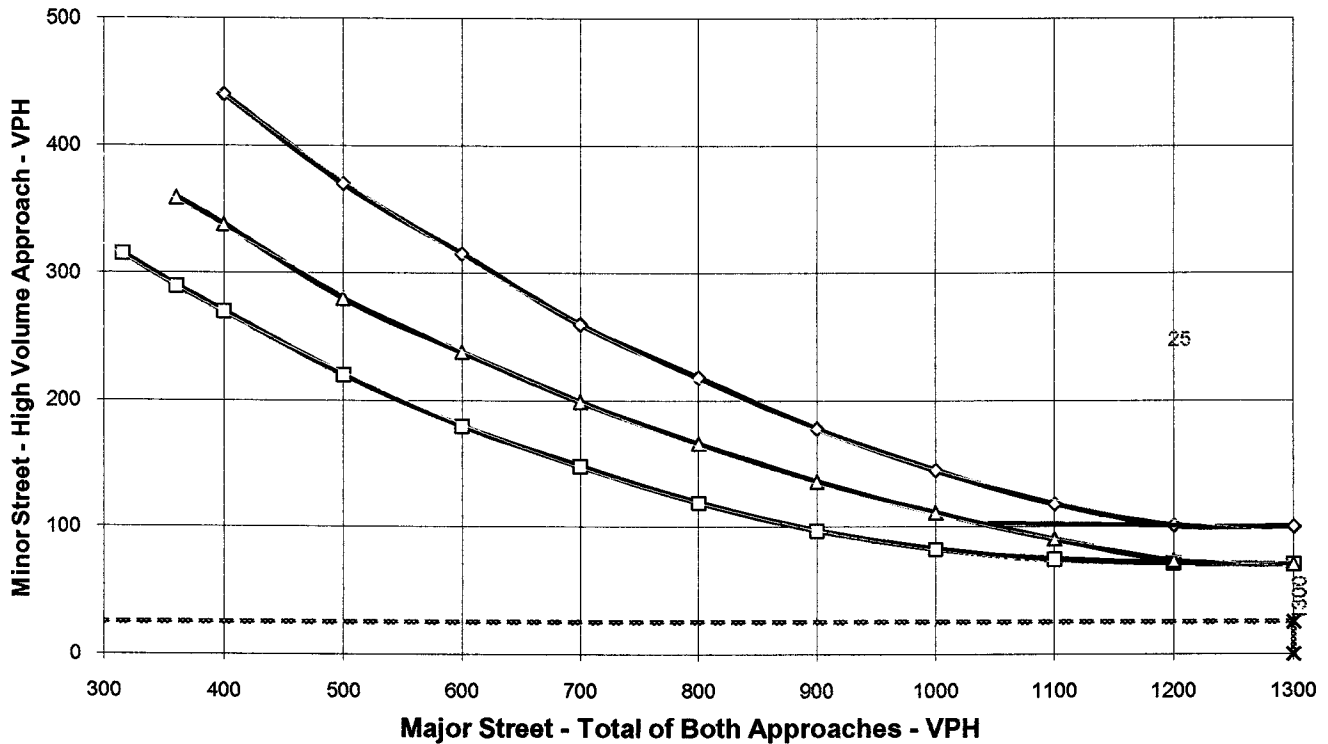
Number of Approach Lanes Major Street = **1**

Minor Street Name = **Soboba Springs Drive**

High Volume Approach (VPH) = **25**

Number of Approach Lanes Minor Street = **1**

SIGNAL WARRANT NOT SATISFIED



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- x— Major Street Approaches
- x— Minor Street Approaches

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

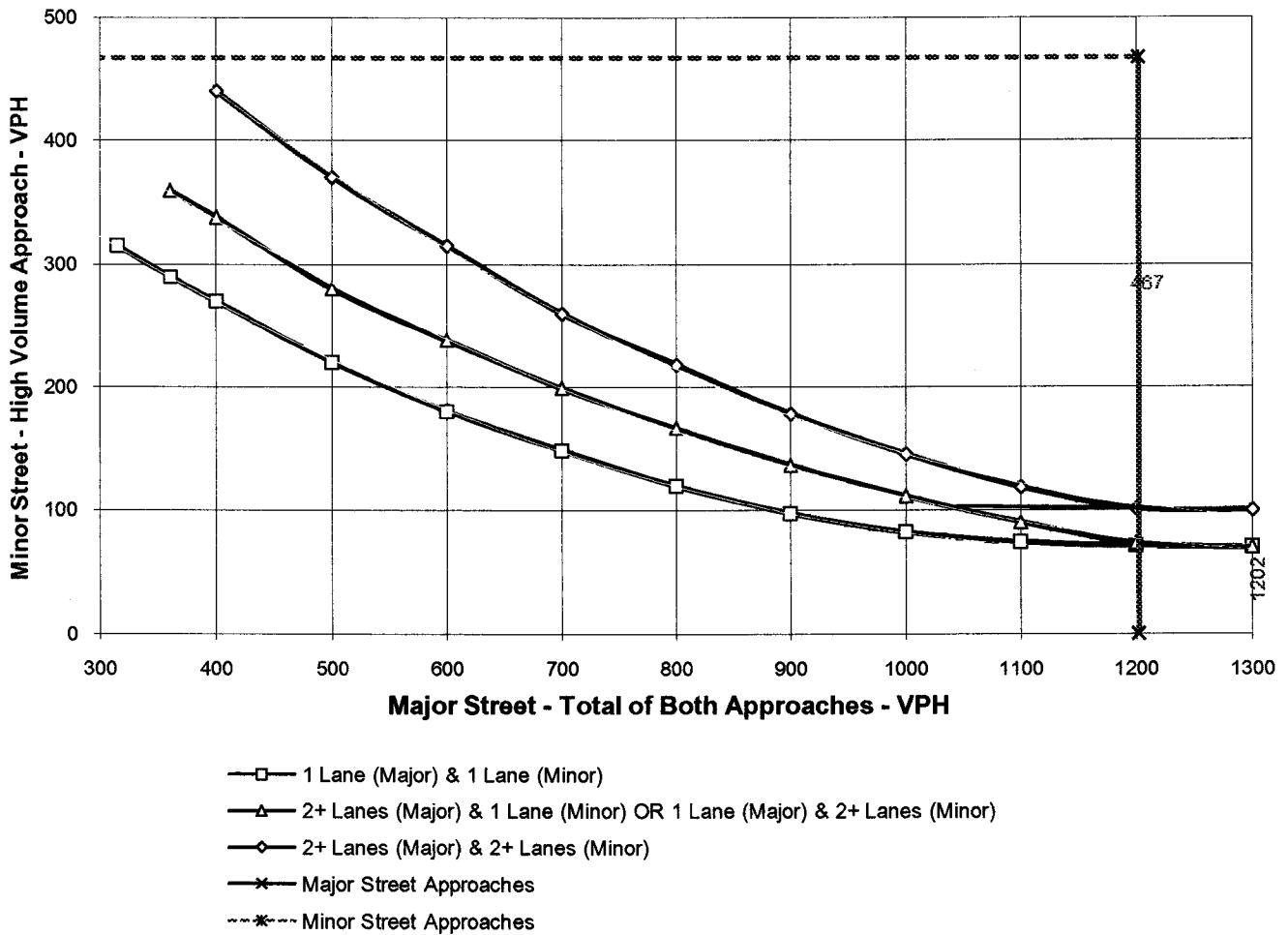
PEAK HOUR VOLUME WARRANT (Rural Areas)

Opening Year (2010) With Project - Proposed Action "A"

Major Street Name = **Project North Entrance** Total of Both Approaches (VPH) = **1202**
 Number of Approach Lanes Major Street = **1**

Minor Street Name = **Soboba Road** High Volume Approach (VPH) = **467**
 Number of Approach Lanes Minor Street = **1**

WARRANTED FOR A SIGNAL



**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

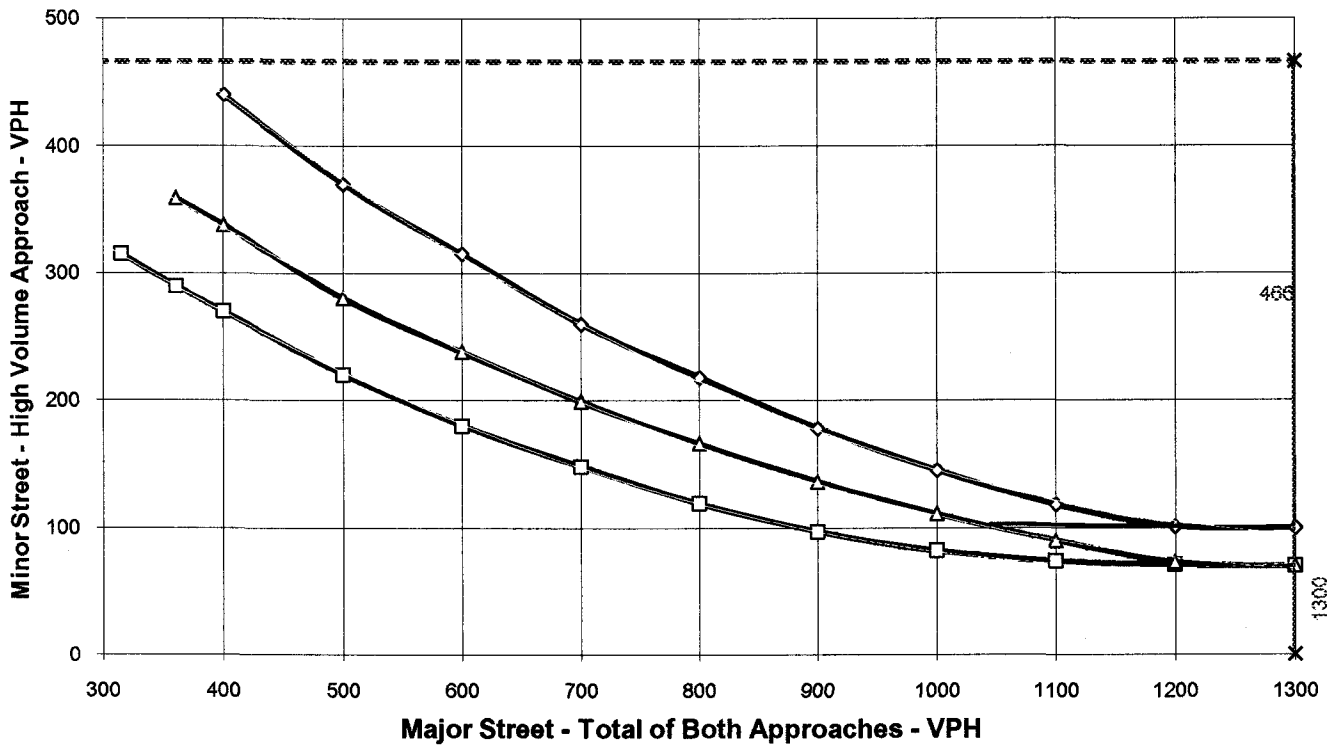
PEAK HOUR VOLUME WARRANT (Rural Areas)

Opening Year (2010) With Project - Proposed Action "A"

Major Street Name = **Project South Entrance** Total of Both Approaches (VPH) = **1601**
 Number of Approach Lanes Major Street = **1**

Minor Street Name = **Soboba Road** High Volume Approach (VPH) = **466**
 Number of Approach Lanes Minor Street = **1**

WARRANTED FOR A SIGNAL



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- ×— Major Street Approaches
- *--- Minor Street Approaches

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

PEAK HOUR VOLUME WARRANT (Rural Areas)

Opening Year (2010) With Project - Proposed Action "B"

Major Street Name = **Project North Access**

Total of Both Approaches (VPH) = **1338**

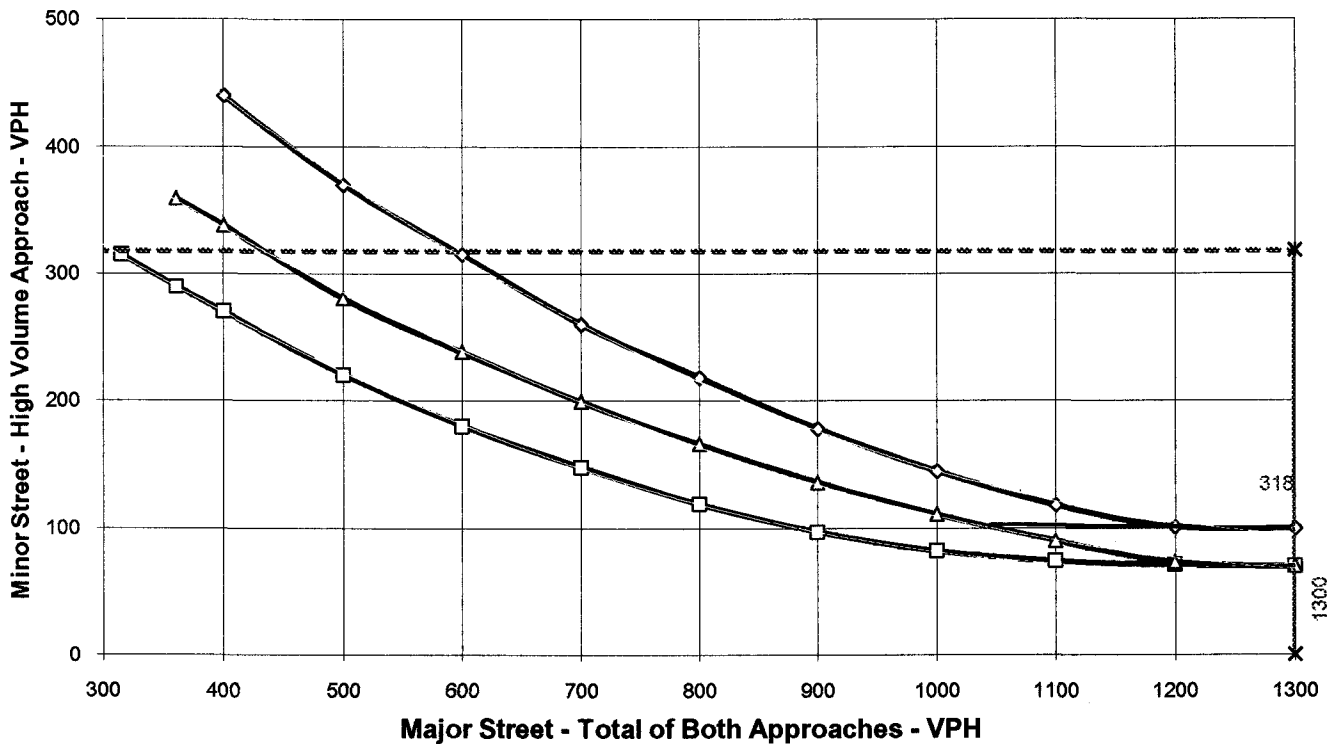
Number of Approach Lanes Major Street = **1**

Minor Street Name = **Soboba Road**

High Volume Approach (VPH) = **318**

Number of Approach Lanes Minor Street = **1**

WARRANTED FOR A SIGNAL



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- ×— Major Street Approaches
- *--- Minor Street Approaches

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

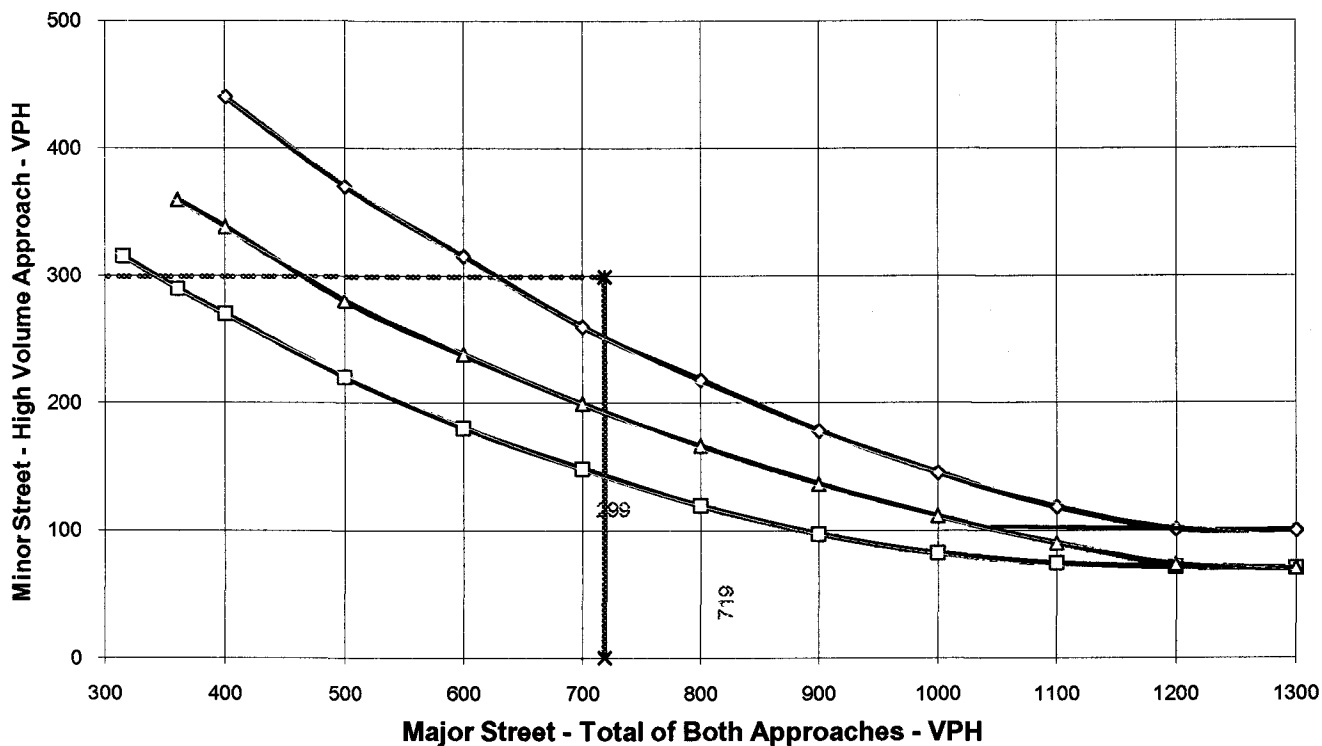
PEAK HOUR VOLUME WARRANT (Rural Areas)

Opening Year (2010) With Project - Proposed Action "B"

Major Street Name = **Project South Access** Total of Both Approaches (VPH) = **719**
 Number of Approach Lanes Major Street = **1**

Minor Street Name = **Soboba Road** High Volume Approach (VPH) = **299**
 Number of Approach Lanes Minor Street = **1**

WARRANTED FOR A SIGNAL



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- ×— Major Street Approaches
- *--- Minor Street Approaches

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

PEAK HOUR VOLUME WARRANT (Rural Areas)

Opening Year (2010) With Project - Alternative 1

Major Street Name = **Project North Access**

Total of Both Approaches (VPH) = **1035**

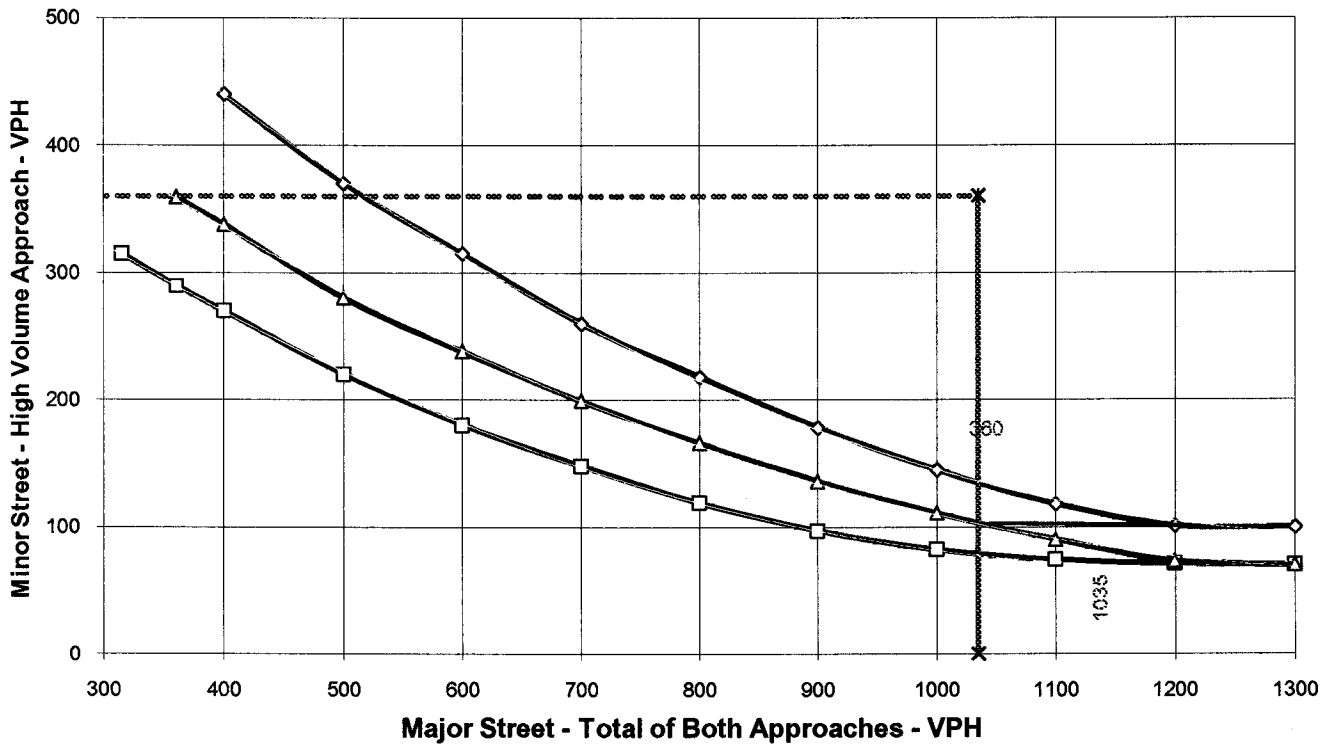
Number of Approach Lanes Major Street = **1**

Minor Street Name = **Soboba Road**

High Volume Approach (VPH) = **360**

Number of Approach Lanes Minor Street = **1**

WARRANTED FOR A SIGNAL



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- x— Major Street Approaches
- *--- Minor Street Approaches

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

PEAK HOUR VOLUME WARRANT (Rural Areas)

Opening Year (2010) With Project - Alternative 1

Major Street Name = **Project South Access**

Total of Both Approaches (VPH) = **1344**

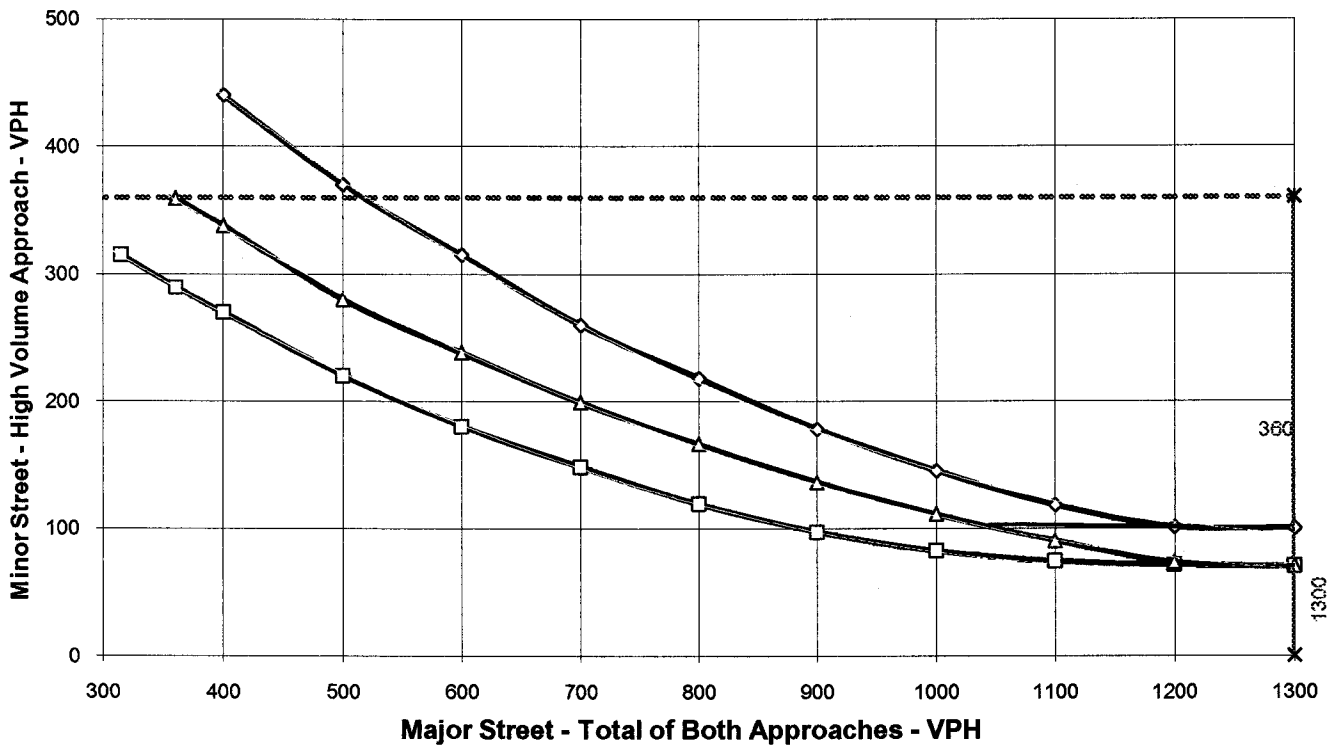
Number of Approach Lanes Major Street = **1**

Minor Street Name = **Soboba Road**

High Volume Approach (VPH) = **360**

Number of Approach Lanes Minor Street = **1**

WARRANTED FOR A SIGNAL



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- ×— Major Street Approaches
- *--- Minor Street Approaches

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

PEAK HOUR VOLUME WARRANT (Rural Areas)

Opening Year (2010) With Project - Alternative 3

Major Street Name = **Lake Park Drive**

Total of Both Approaches (VPH) = **1363**

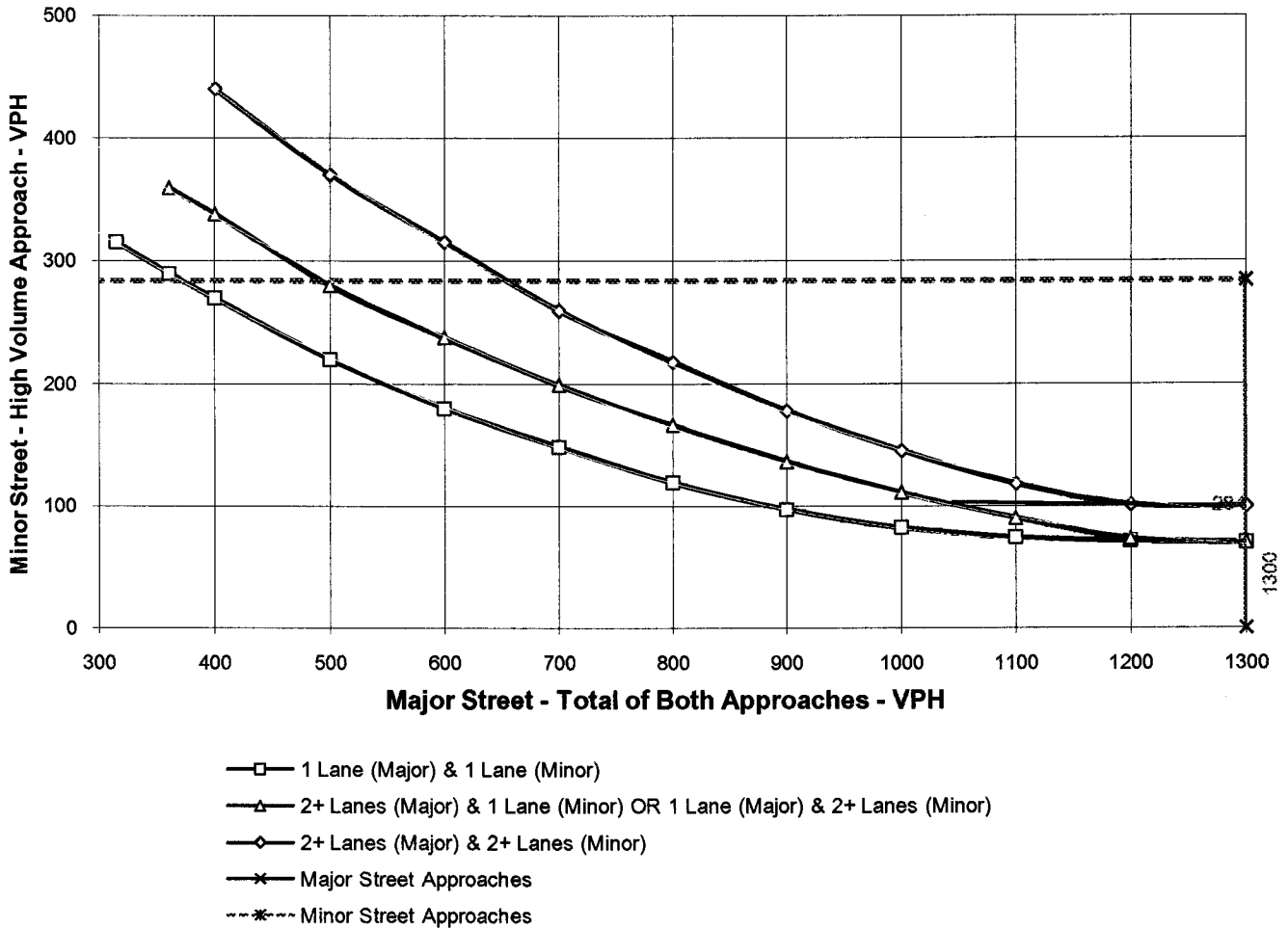
Number of Approach Lanes Major Street = **1**

Minor Street Name = **Project Access**

High Volume Approach (VPH) = **284**

Number of Approach Lanes Minor Street = **1**

WARRANTED FOR A SIGNAL



**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

PEAK HOUR VOLUME WARRANT (Rural Areas)

Opening Year (2010) With Project - Alternative 3

Major Street Name = **Soboba Road**

Total of Both Approaches (VPH) = **1081**

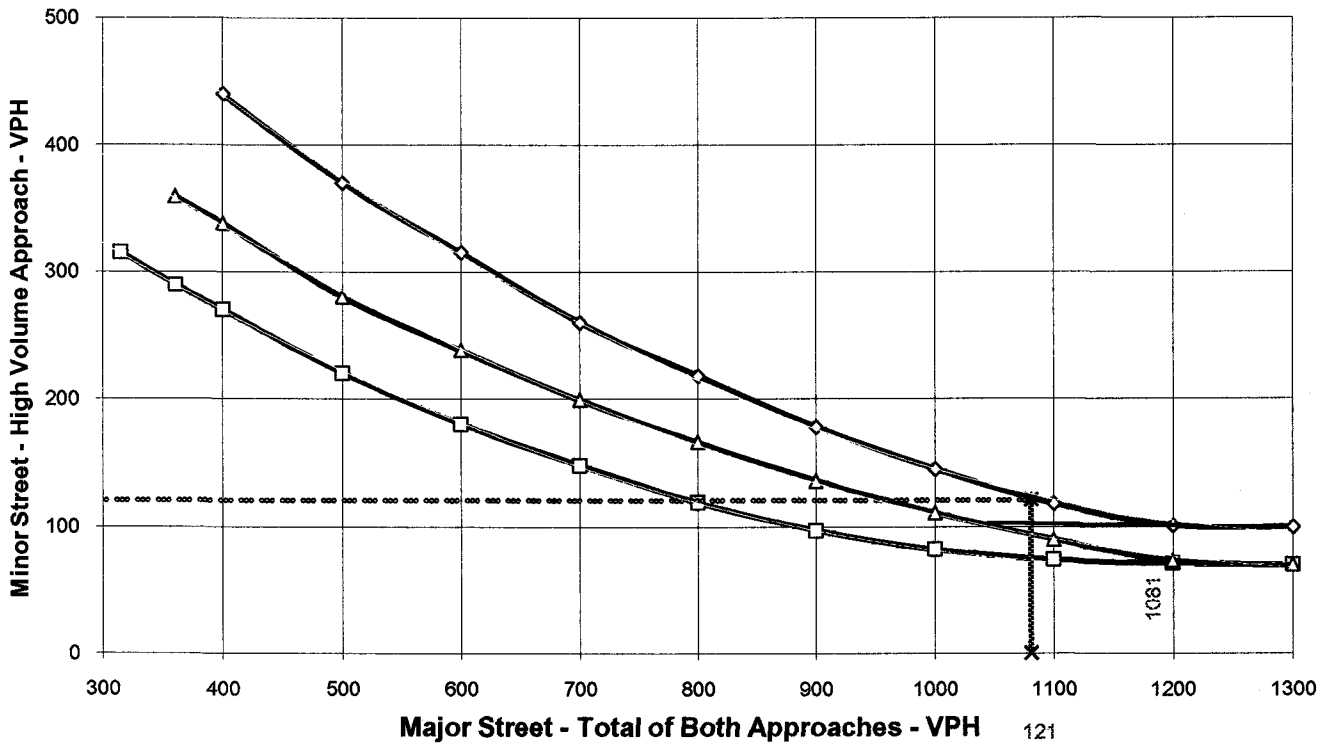
Number of Approach Lanes Major Street = **1**

Minor Street Name = **Project Access**

High Volume Approach (VPH) = **121**

Number of Approach Lanes Minor Street = **1**

WARRANTED FOR A SIGNAL



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- x— Major Street Approaches
- *--- Minor Street Approaches

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

PEAK HOUR VOLUME WARRANT (Rural Areas)

Year 2025 With Project - Proposed Action "A"

Major Street Name = **Lake Park Drive**

Total of Both Approaches (VPH) = **3012**

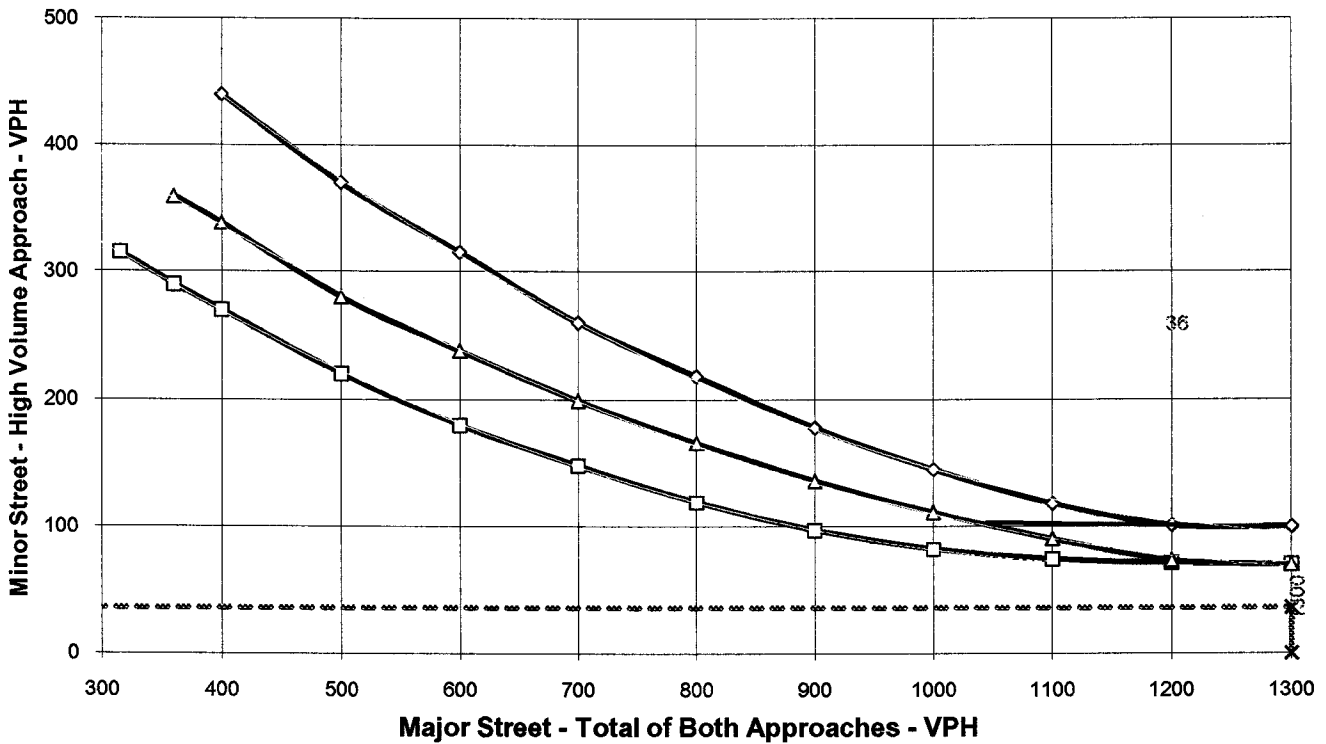
Number of Approach Lanes Major Street = **1**

Minor Street Name = **Soboba Springs Drive**

High Volume Approach (VPH) = **36**

Number of Approach Lanes Minor Street = **1**

SIGNAL WARRANT NOT SATISFIED



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- ×— Major Street Approaches
- *— Minor Street Approaches

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

PEAK HOUR VOLUME WARRANT (Rural Areas)

Year 2025 With Project - Proposed Action "B"

Major Street Name = **Lake Park Drive**

Total of Both Approaches (VPH) = **2976**

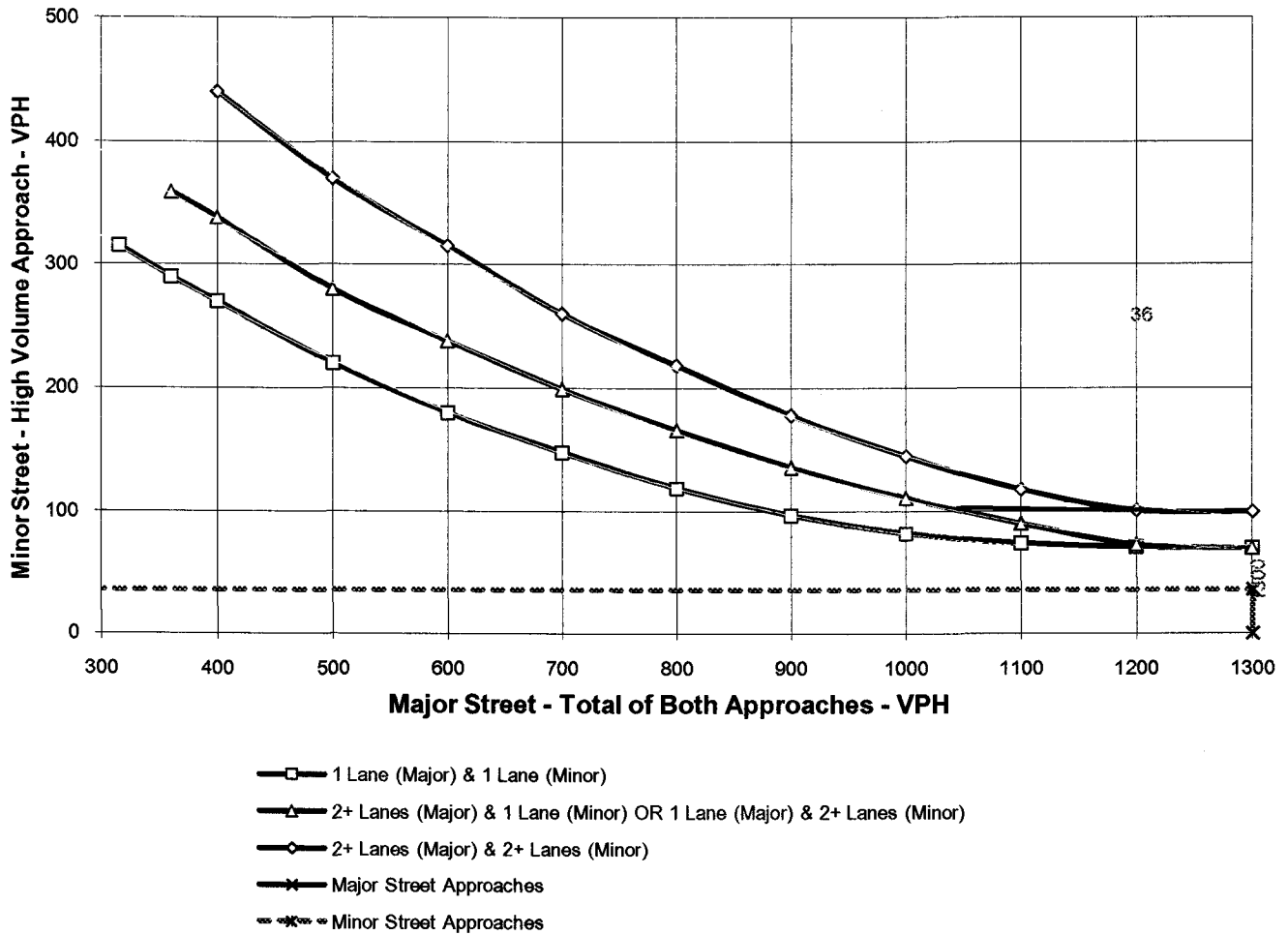
Number of Approach Lanes Major Street = **1**

Minor Street Name = **Soboba Springs Drive**

High Volume Approach (VPH) = **36**

Number of Approach Lanes Minor Street = **1**

SIGNAL WARRANT NOT SATISFIED



**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

PEAK HOUR VOLUME WARRANT (Rural Areas)

Year 2025 With Project - Alternative 1

Major Street Name = **Lake Park Drive**

Total of Both Approaches (VPH) = **2694**

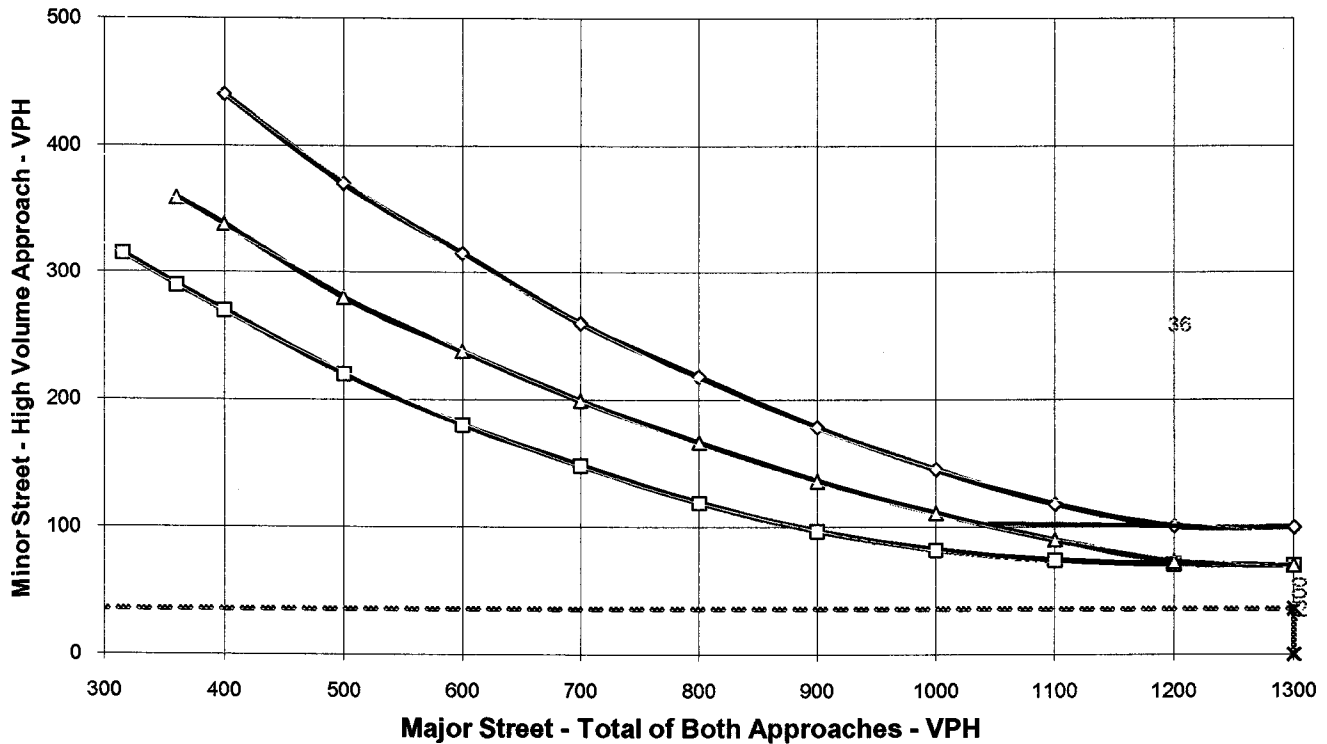
Number of Approach Lanes Major Street = **1**

Minor Street Name = **Soboba Springs Drive**

High Volume Approach (VPH) = **36**

Number of Approach Lanes Minor Street = **1**

SIGNAL WARRANT NOT SATISFIED



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- ×— Major Street Approaches
- *— Minor Street Approaches

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

PEAK HOUR VOLUME WARRANT (Rural Areas)

Year 2025 With Project - Alternative 2

Major Street Name = **Lake Park Drive**

Total of Both Approaches (VPH) = **2080**

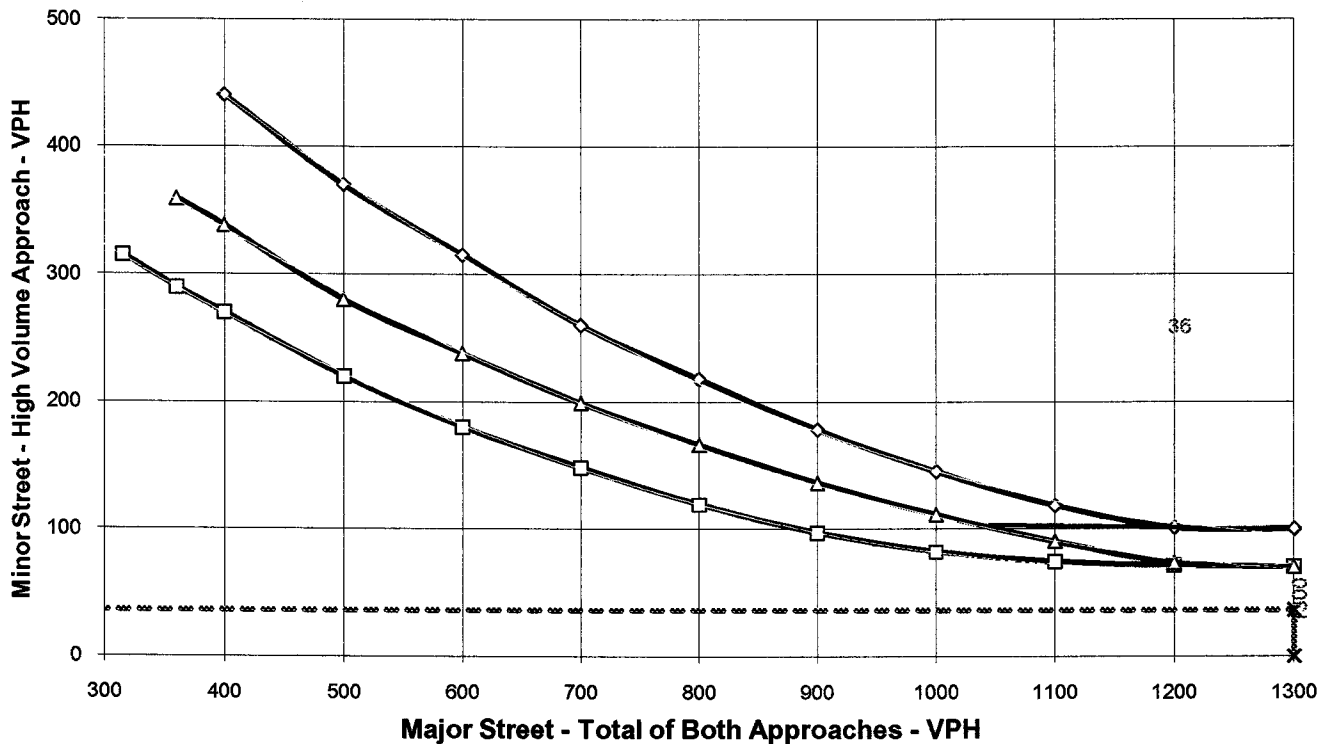
Number of Approach Lanes Major Street = **1**

Minor Street Name = **Soboba Springs Drive**

High Volume Approach (VPH) = **36**

Number of Approach Lanes Minor Street = **1**

SIGNAL WARRANT NOT SATISFIED



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- x— Major Street Approaches
- *— Minor Street Approaches

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

PEAK HOUR VOLUME WARRANT (Rural Areas)

Year 2025 With Project - Alternative 3

Major Street Name = **Lake Park Drive**

Total of Both Approaches (VPH) = **2339**

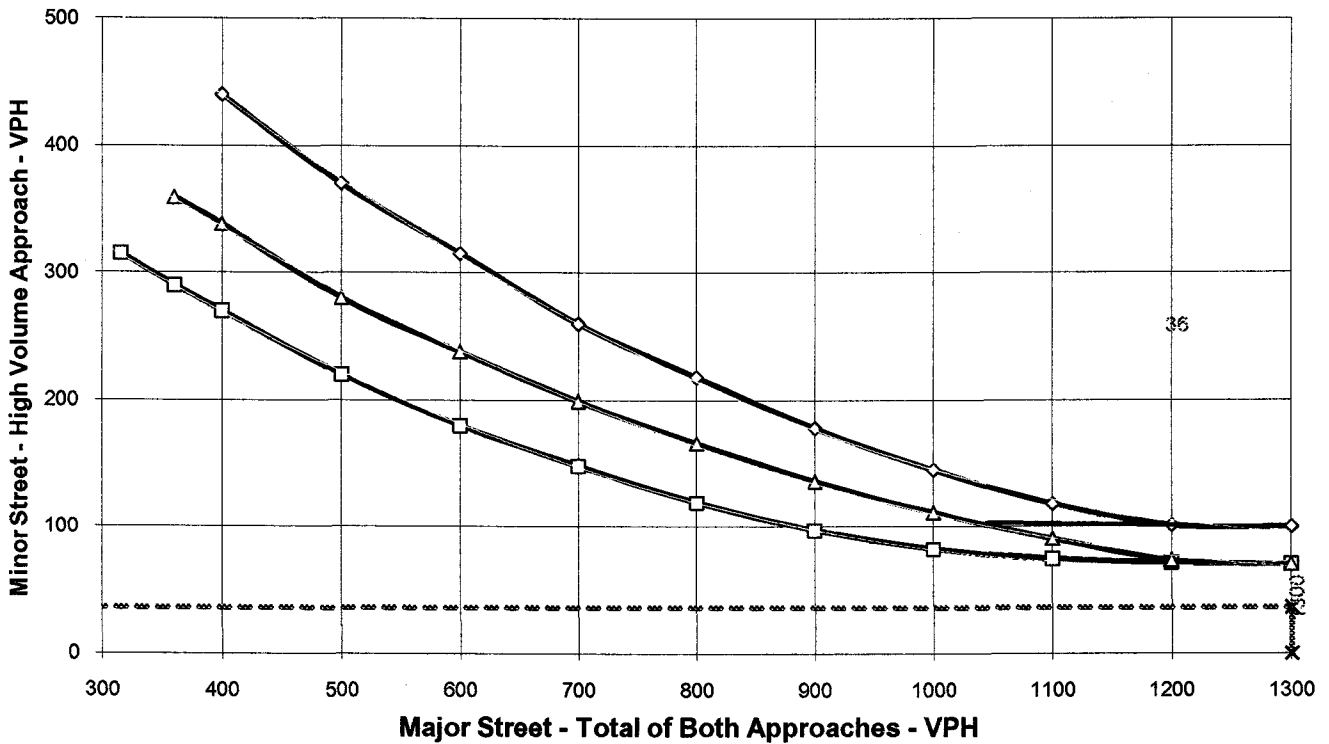
Number of Approach Lanes Major Street = **1**

Minor Street Name = **Soboba Springs Drive**

High Volume Approach (VPH) = **36**

Number of Approach Lanes Minor Street = **1**

SIGNAL WARRANT NOT SATISFIED



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- ×— Major Street Approaches
- Minor Street Approaches

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

PEAK HOUR VOLUME WARRANT (Rural Areas)

Year 2025 With Project - Alternative 2

Major Street Name = **Soboba Road**

Total of Both Approaches (VPH) = **1655**

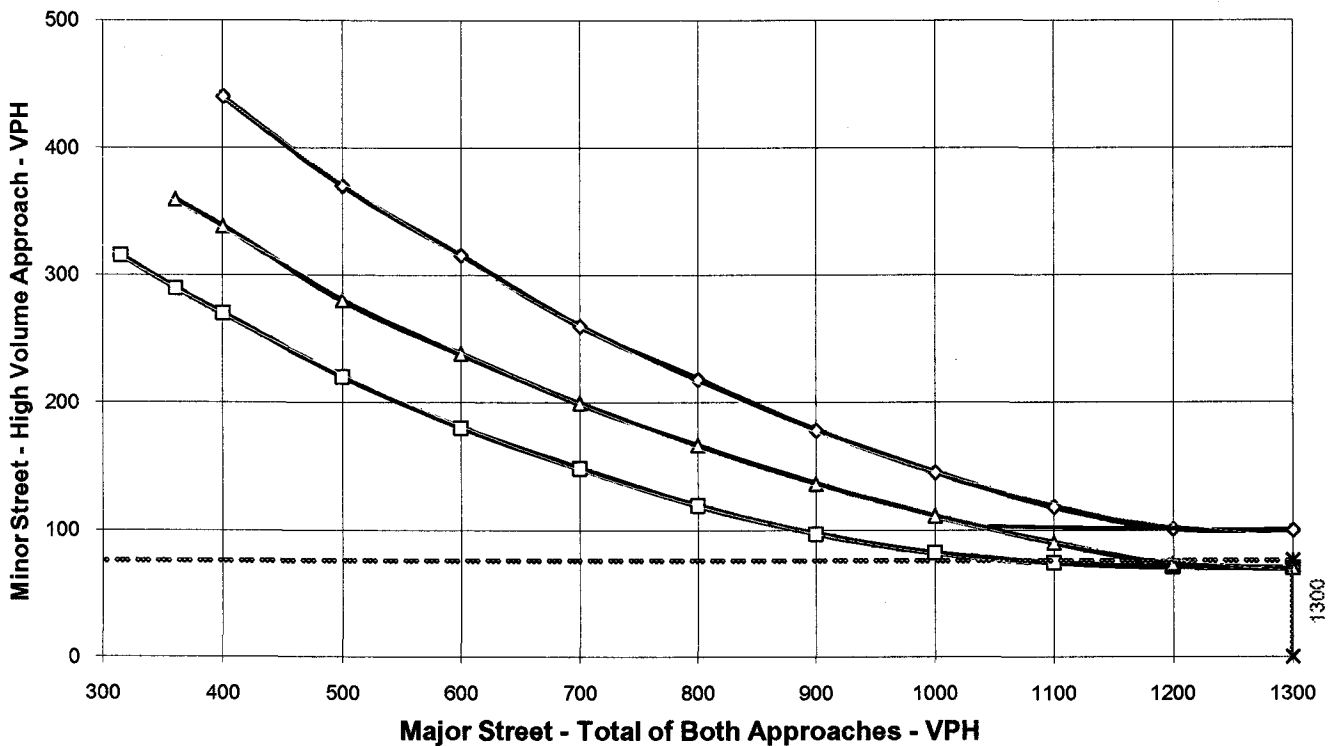
Number of Approach Lanes Major Street = **1**

Minor Street Name = **Project South Access**

High Volume Approach (VPH) = **76**

Number of Approach Lanes Minor Street = **1**

WARRANTED FOR A SIGNAL



- 1 Lane (Major) & 1 Lane (Minor)
- △— 2+ Lanes (Major) & 1 Lane (Minor) OR 1 Lane (Major) & 2+ Lanes (Minor)
- ◇— 2+ Lanes (Major) & 2+ Lanes (Minor)
- ×— Major Street Approaches
- *--- Minor Street Approaches

76

**** NOTE:**

100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACHING WITH ONE LANE.

APPENDIX E

**Shingle Springs Rancheria Interchange
Transportation/Circulation Report**

TRANSPORTATION/CIRCULATION

INTRODUCTION

This study was prepared to assess the traffic impacts and operations associated with the proposed Shingle Springs Rancheria Interchange, which is being proposed to provide access to the existing Shingle Springs Rancheria, and the newly proposed Shingle Springs Rancheria Casino. The analysis investigates existing operations in the vicinity of the proposed interchange, as well as operations in the vicinity of the proposed interchange for cumulative conditions, which for this study is assumed to be the year 2025. The analysis also investigates the traffic operations and impacts associated with the proposed interchange for both existing and cumulative conditions.

This study analyzes impacts for the following three alternatives:

- Alternative AA – No Project Alternative
- Alternative AB – “Flyover Alternative” - Modified trumpet design (Type L-11)
- Alternative AC – “Diamond Alternative” - Modified tight diamond w/ no right-of-way taking (Type L-1)

ENVIRONMENTAL SETTING

EXISTING SETTING

EXISTING ROADS

The following roadways are located in the vicinity of the proposed interchange:

US-50 is an east-west freeway which provides regional access between Sacramento and Placerville, and recreational areas within the southern Lake Tahoe area. In the vicinity of the proposed interchange, US-50 has two lanes in each direction, 10 ft. paved outside shoulders and 5 ft. paved inside shoulders, and a 70 ft. wide grassy median. At present, approximately 45,500 average daily trips occur along US-50 in the vicinity of the proposed interchange.

East Shingle Springs Drive is a 2-lane rural roadway which runs in a north-south direction immediately west of the proposed interchange. The roadway begins within the gated residential community immediately north of US-50 and Rock Barn Road, and continues south of US-50 approximately 1 mile to Buckeye Road, where the roadway terminates. At present, the average daily traffic along the roadway is less than 1,000 vehicles per day. E. Shingle Springs Drive is maintained by the County of El Dorado up to the private gate immediately north of US-50 and Rock Barn Road.

Greenstone Road is a 2-lane rural roadway which runs in a north-south direction immediately east of the proposed interchange. The roadway begins approximately 1 mile north of US-50 at Green Valley Road, and continues south of US-50 a few miles to Mother Lode Drive. At

present, the average daily traffic along the roadway is approximately 2,000 vehicles per day north of US-50, and less than 1,000 vehicles per day south of US-50. There are no sidewalks and minimal or non-existent shoulders along the roadway, and the roadway width is approximately 24 feet. Greenstone Road is maintained by the County of El Dorado.

Grassy Run Road, Rolling Rock Road, and Reservation Road comprise the route which is currently used to access the Shingle Springs Rancheria. The construction of the new interchange will result in the rerouting of a significant percentage of Rancheria traffic currently using these roadways to the new interchange. To access the Rancheria today, traffic turns left from Greenstone Road to Grassy Run Road. A few hundred feet west of Greenstone Road, Grassy Run Road transitions from a County road to a private residential roadway. Rancheria bound traffic continues along the private roads of Rolling Rock Road and Reservation Road. A count of traffic along the route currently used to access the site was conducted early 1999, and showed that during the PM peak hour approximately 15 inbound vehicles and 7 outbound vehicles were travelling to and from the Rancheria (and adjacent private development). All three of these roads are narrow 2-lane roadways with no sidewalk or shoulder, and contain extreme horizontal and vertical curvature.

EXISTING TRAFFIC VOLUMES

Peak Hour Volumes

Existing weekday AM and PM peak hour and Saturday peak hour counts along US-50 were established using the following resources:

- 1) Raw 1999 Caltrans Traffic counts from their count station on US-50 between the Cameron Park Drive and Ponderosa Road interchanges (Caltrans does not maintain a count station in the immediate vicinity of the proposed interchange).
- 2) *Caltrans' 1998 Traffic Volumes on California State Highways* (including "Peak Hour Volume Data by Direction" within Appendix).
- 3) Peak hour ramp counts at the US-50/East Shingle Springs Drive interchange.
- 4) 7-day/24-hour roadway tube counts along East Shingle Springs Drive south of US-50.

Although Caltrans publishes ADT volumes for US-50 between East Shingle Springs Drive and Greenstone Road, they are not based on counts at the exact location, but rather are based on counts collected at the nearest count station. Additionally, counts published by Caltrans do not provide detailed directional counts or peak hour counts. The Caltrans count station nearest to the area in question is located a few miles west of the project site between Cameron Park Drive and Ponderosa Road. Hourly counts from this count station were obtained from Caltrans for the entire calendar year 1999. Although the specific location of these Caltrans counts were a few interchanges towards the west, these detailed counts were used as a basis for establishing directional peak hour counts at the site of the proposed interchange. Weekday AM and PM peak hour volumes, as well as Saturday peak hour volumes, were established at the count station by averaging the peak hour volumes occurring throughout the year for each of the three specific peak hours. These counts were converted to volumes which would be expected along US-50

between East Shingle Springs Drive and Greenstone Road using the methodologies outlined in the following paragraphs.

Using *Caltrans' 1998 Traffic Volumes on California State Highways*, it was noted that the AADT (Annual Average Daily Traffic) east of Ponderosa Road (between Ponderosa Road and East Shingle Springs Drive) was 45,500 vehicles in 1998, which is 86% of the 1998 AADT of 53,000 reported west of Ponderosa Road. This same factor was applied to the peak hour counts west of Ponderosa Road to establish peak hour counts between Ponderosa Road and East Shingle Springs Drive.

To establish peak hour volumes along US-50 east of East Shingle Springs Drive, peak hour ramp volumes along US-50/East Shingle Springs Drive interchange ramps were added and subtracted to those volumes occurring west of the interchange. David Evans and Associates, Inc. collected weekday AM peak hour ramp volumes at the US-50/East Shingle Springs Drive interchange on Tuesday, March 28, 2000, and weekday PM peak hour volumes on Tuesday, November 9, 1999. Saturday peak hour ramp volumes were established by factoring, adjusting and balancing weekday PM peak counts. This factoring, adjustment and balancing was made based on 7-day/24-hour roadway counts collected March, 2000 along East Shingle Springs Drive immediately south of the interchange, and the hourly Caltrans US-50 counts previously described. The resulting Saturday peak hour counts were confirmed by a field sampling of Saturday counts. This procedure was used instead of an actual Saturday count since it is difficult to establish exactly when the peak hour occurs on a Saturday.

Table 1 provides a summary of existing freeway volumes along US-50 for all three peak hour scenarios.

**Table 1
Existing No Project
Peak Hour Volumes**

Freeway Segment	Peak Hour Volumes		
	AM Peak Hour	PM Peak Hour	Saturday Peak Hour
EB US-50 (btwn E. Shingle Springs & Greenstone)	1,229	2,407	1,872
WB US-50 (btwn Greenstone & E. Shingle Springs)	2,206	1,589	1,691
TOTAL	3,435	3,996	3,563

Daily Volumes

Existing daily volumes as reported for the local roads analysis is based on traffic counts as reported within El Dorado County Department of Transportation's "2000 Traffic Count Annual Summary." These counts were supplemented as necessary from additional traffic counts supplied by the El Dorado County Department of Transportation, the 1994 Regional Transportation Plan for El Dorado County. All older counts which were used were increased to reflect 2000 counts.

Volumes along Caltrans facilities were obtained from "2000 Caltrans Traffic Volume on California State Highways" data as included on Caltrans' web site.

LEVEL OF SERVICE CONCEPT

The operating conditions experienced by motorists are described as "levels of service" (LOS). Level of service is a qualitative measure of the effect of a number of factors, including speed and travel time, traffic interruptions, freedom to maneuver, driving comfort and convenience. Levels of service are designated "A" through "F" from best to worst, which cover the entire range of traffic operations that might occur. Levels of service "A" through "E" generally represent traffic volumes at less than roadway capacity, while LOS "F" represents over capacity and/or forced flow conditions.

EXISTING FREEWAY MAINLINE OPERATIONS

Traffic operations were evaluated for existing weekday AM peak hour, PM peak hour, and Saturday peak hour conditions.

Table 2 shows the current freeway mainline operations for all three peak hour scenarios.

**Table 2
Existing No Project
Freeway Mainline Level of Service**

Freeway Segment	Freeway Level of Service ⁽¹⁾⁽²⁾		
	AM Peak Hour	PM Peak Hour	Saturday Peak Hour
EB US-50 (btwn E. Shingle Springs & Greenstone)	B	D	C
WB US-50 (btwn Greenstone & E. Shingle Springs)	D	C	C

Notes:

- (1) Free Flow speed varies - "Ideal" Free Flow Speed of 65 mph adjusted to account for specific highway geometry.
- (2) The *Route 50 Transportation Concept Report* states that the concept level of service for US-50 between Sacramento and Placerville is LOS E.

As the above table shows, the freeway currently operates acceptably at LOS D along the eastbound direction during the PM peak hour, and along the westbound direction during the AM peak hour. During the Saturday peak hour, and along the opposing non-peak direction of travel during weekday AM and PM peak hours, the freeway operates acceptably at LOS C or better.

Detailed level of service analysis data is provided in **Appendix A**.

ACCIDENT ANALYSIS

To establish potential safety issues and accident potential along US-50 in the vicinity of the new interchange, an analysis of accidents over the past 4 years was conducted along US-50 between the East Shingle Springs Drive and Greenstone Road interchanges, which are located approximately 1 mile west and east of the proposed interchange, respectively.

Table 3 compares the accident rate occurring along this stretch of highway with the statewide average for a similar roadway facilities.

Table 3
Accident Rate

Total Accidents (4 years) ⁽¹⁾	27 accidents
Total Fatalities (4 years)	0 fatalities
Distance (E Shingle Springs to Greenstone)	1.89 miles
Daily Average Daily Traffic Volume ⁽¹⁾	Year 1997 to 1999: 43,300 veh/day Year 2000: 43,000 veh/day
4 Year Volume	63.1085 million-vehicles
Actual Accident Rate	0.23 accidents/million-vehicle-miles
Average Accident Rate for Similar Facility⁽¹⁾	0.60 accidents/million-vehicle-miles

Notes: (1) Caltrans accident summary printout.

As the above table shows, the accident rate along US-50 in the vicinity of the proposed interchange is 0.23 accidents per million-vehicle-miles, which is less than half the average rate for similar facilities of 0.60 accidents per million-vehicle-miles.

The above table also shows that there were no fatalities along this stretch of US-50 during the 4 year period studied.

To summarize, the section of US-50 in the vicinity of the proposed interchange does not experience a high number of any particular type of accidents, a high number of accidents due to any particular roadway condition, or a higher rate of accidents than that which occurs along similar types of facilities.

Table 4 provides an annual and 4-year summary of accidents occurring along US-50 between East Shingle Springs Drive and Greenstone Road. The table shows a total of 27 accidents occurred along this freeway segment during the 4 year period between January 1, 1997 and December 31, 2000. The table also shows that no particular type of accident, or roadway conditions or factors contributing to accidents, are significantly higher than any of the other accident types, or contributing roadway conditions or factors.

Table 3

CUMULATIVE (2025) SETTING

CUMULATIVE ROADWAY NETWORK

The roadway network in the immediate vicinity of the project site is assumed to remain the same for Cumulative Conditions as that which currently exists for Existing Conditions.

Caltrans currently has no programmed improvement for US-50 for Cumulative Conditions, although there are currently discussions to either provide an HOV lane along the freeway, and/or 6 standard lanes along the freeway. Within this analysis, it is assumed that US-50 will remain a 4-lane facility.

The cumulative roadway network analyzed for the local roads analysis was established in coordination with El Dorado County traffic engineering personnel. Local roadways which were analyzed were those identified as "major roadways," which are defined as those roadways identified on both the "*El Dorado County General Plan Circulation Map*," and within the roadway network contained within the 1999 version of the "*El Dorado County Travel Demand Forecasting Model*" in MINUTP. Cumulative year roadway geometrics are based on the roadway network geometries included within the 2022 CIP (Capitol Improvement Program) data network files contained with the 1999 El Dorado County travel demand model.

CUMULATIVE BACKGROUND VOLUMES

Peak Hour Volumes

Cumulative traffic volumes were established based on the El Dorado County traffic model as established for the 1996 El Dorado County General Plan. The model volumes are based on weekday PM peak hour conditions, and utilize Year 2022 as the future horizon year. Caltrans has established that the Year 2025 should be utilized to analyze Cumulative Conditions for this study. Cumulative volumes were initially established for year 2022 conditions, then factored up to year 2025 conditions through a straight line extrapolation of volumes.

US-50 volumes from the El Dorado County traffic model for 2022 conditions were very suspect. An analysis of PM peak hour volumes for 2022 conditions for US-50 shows that there was a loss of 850 vehicles from US-50 west of East Shingle Springs Drive to east of East Shingle Springs Drive. Since the model shows only 383 vehicles along East Shingle Springs Drive south of US-50 (and volumes north of US-50 would continue to be very low and negligible), there was an obvious error in the model. El Dorado County has recognized this flaw and is in the process of correcting the model.

To establish the general integrity of volumes from the model, we compared existing AM and PM peak hour volumes which were collected by David Evans and Associates, Inc. for US-50, East Shingle Springs Drive, and the US-50/East Shingle Springs Drive freeway ramps to forecasted volumes from the model. Focusing on the PM peak hour, it is noted that US-50 west of the East Shingle Springs Drive interchange are projected to experience an annual growth rate between 1.7-4.3%. However, US-50 east of the interchange is projected to increase only 0.7-0.9% per

year. This helped to confirm that forecasted US-50 volumes east of the interchange are very circumspect, and likely too low. To address this error, we increased the volume obtained from the model for US-50 east of the interchange. This adjustment was made by establishing reasonable cumulative ramp volumes along US-50/East Shingle Springs Drive freeway ramps for cumulative conditions based on both existing peak hour volumes, and projected growth as shown in the model. These ramp volumes were then added and subtracted from US-50 volumes west of the US-50/East Shingle Springs Drive interchange to establish US-50 volumes east of the US-50/East Shingle Springs Drive interchange.

Table 5 provides a summary of cumulative (2025) freeway volumes along US-50 for all three peak hour scenarios.

**Table 5
Cumulative No Project
Peak Hour Volumes**

Freeway Segment	Peak Hour Volumes		
	AM Peak Hour	PM Peak Hour	Saturday Peak Hour
EB US-50 (btwn E. Shingle Springs & Greenstone)	2,150	3,441	2,681
WB US-50 (btwn Greenstone & E. Shingle Springs)	3,086	2,316	2,465
TOTAL	5,236	5,757	5,146

Daily Volumes

Cumulative daily volumes as reported for the local roads analysis are based primarily on traffic counts as reported within the 1999 version of the "El Dorado County Travel Demand Forecasting Model." Traffic volumes along Caltrans facilities were adjusted to reflect 2025 volumes already used within this analysis, as well as additional 2025 traffic volumes as supplied by Caltrans.

CUMULATIVE FREEWAY MAINLINE OPERATIONS

Table 6 shows the freeway mainline operations which are projected for all three peak hour scenarios for a 4-lane facility.

**Table 6
Cumulative No Project (4-lanes)
Freeway Mainline Level of Service**

Freeway Segment	Freeway Level of Service ⁽¹⁾⁽²⁾		
	AM Peak Hour	PM Peak Hour	Saturday Peak Hour
EB US-50 (btwn E. Shingle Springs & Greenstone)	D	E	E
WB US-50 (btwn Greenstone & E. Shingle Springs)	E	D	D

Notes:

- (1). Free Flow speed varies - "Ideal" Free Flow Speed of 65 mph adjusted to account for specific highway geometry.
- (2). The *Route 50 Transportation Concept Report* states that the concept level of service for US-50 between Sacramento and Placerville is LOS E.

As the above table shows, if US-50 remains a 4-lane facility without any auxiliary lanes, the freeway is projected to operate acceptably at LOS E along the eastbound direction during the weekday PM peak hour and Saturday peak hour, and along the westbound direction during the AM peak hour. The levels of service along the opposing direction during all three peak hours are also projected to operate acceptably at LOS D.

Detailed level of service analysis data is provided in **Appendix B**.

DIRECT IMPACTS AND MEASURES TO MINIMIZE

SIGNIFICANCE CRITERIA

CALTRANS ROUTE CONCEPT REPORT

The target level of service for this analysis is based primarily on *Caltrans' State Route 50 Transportation Concept Report*, issued in April. The report includes the following text which describes Caltrans' goals and objectives for US-50 within El Dorado County, specifically within the vicinity of the proposed interchange.

Segment 6 consists of a four-lane divided freeway from the Sacramento/El Dorado County Line to the West Placerville Undercrossing. The terrain is predominantly rolling (some steep grades).

This segment carries the greater share of commuter travel emanating from El Dorado County. Although this segment stretches to just west of Placerville, the primary focus for this segment is on the commuter travel shed which lies between the Sacramento/El Dorado County Line to Ponderosa, near Shingle Springs. The increases in commute travel volumes arise from the growing communities of El Dorado Hills, Bass Lake, Cameron Park, and Shingle Springs which act as bedroom communities to employment centers in Sacramento County, i.e., Folsom,

Rancho Cordova. The remainder of the segment carries relatively lower commute traffic volumes, and travel patterns turn mostly interregional and recreational in nature. This segment overall operates at an acceptable LOS E. However, the western portion of this segment often, during peak periods, falls to LOS F. The level of service for the entire segment is expected to drop to "F" by the year 2007. By the year 2017, it is estimated that demand will exceed the capacity of the facility by 1.63 times with two or more hours of delay.

** Implementation of the Concept Improvements, i.e., six-lane freeway with HOV, in conjunction with additional local parallel facilities, light-rail extensions, etc., will not provide this segment with LOS E the entire twenty-year period. It will be necessary, therefore, to examine the need to further expand this segment of SR 50 prior to the conclusion of the planning period rather than beyond the twenty-year period.*

SACOG U.S. 50 CORRIDOR STUDY:

On December 18, 1997, the SACOG Board adopted the strategies in the Investment Strategy for the U.S. Corridor Major Investment Study. The study evaluated long-term investment strategies including light-rail extensions, alternative phasing strategies for carpool lanes, and transportation management strategies within the SR 50 Corridor from downtown Sacramento to El Dorado Hills in El Dorado County.

Relative to Segment 6, the findings of the study identified the following Tier 1 strategy project (projects for early funding consideration from regional or discretionary sources – in priority order): Priority No. 8 – HOV lanes on U.S. 50 between Prairie City Road and El Dorado Hills Boulevard (\$9.4 million). Under Tier 2 (projects to be considered for Regional or Discretionary Funds), the study also recommended transportation management strategies and operational policies to be implemented or studied.

The *Route 50 Transportation Concept Report* states that the concept level of service for US-50 between Sacramento and Placerville is LOS E. Thus for this analysis, LOS below this level (LOS "F") would be considered an unacceptable condition.

STANDARDS OF SIGNIFICANCE - FREEWAY MAINLINE AND FREEWAY RAMPS

At the direction of Caltrans staff, the concept level of service for freeway mainline and freeway ramp merge analysis is LOS E, thus LOS "F" is considered unacceptable for freeway mainline and merge analyses.

The target level of service for freeway ramp diverge analysis is a reduced LOS D, thus LOS "E" or "F" is considered unacceptable for diverge analyses.

STANDARDS OF SIGNIFICANCE - STUDY INTERSECTIONS

In terms of the new intersections which may be a part of the proposed interchange, the applicable target level of service criteria for the new intersections would likely need to conform to Caltrans level of service standards. Using Caltrans concept level of service criteria as described above, LOS D would logically be used as the target level of service for new intersections.

However, intersection levels of service should also be checked against the target level of service criteria established for El Dorado County. The 1996 El Dorado County General Plan states the following in Policy 3.5.1.1:

The County shall adopt a roadway plan consistent with planned land use and shall maintain an operating Level of Service of 'E' or better on all roadways, consistent with Objective 3.5.1. In addition, all road segments projected in the roadway plan at the year 2015 to be operating at LOS A, B, or C shall not be allowed to fall below LOS C and all road segments at LOS D shall not fall below LOS D.

Therefore, LOS "C" is regarded as the target LOS for the newly created intersections. LOS below this level (LOS "D", "E" or "F") is considered an unacceptable condition.

STANDARDS OF SIGNIFICANCE - LOCAL ROADS

Potential impacts to local roads (including SR-49 SR-198) were analyzed using the following methodology, as established in coordination with El Dorado County traffic engineering personnel.

- Impacts to all "major roads" within El Dorado County were analyzed, as identified and included within both the "El Dorado County General Plan Circulation Map", and the 1999 version of the "El Dorado County Travel Demand Forecasting Model" in MINUTP.
- Impacts were analyzed for existing and cumulative daily conditions.
- Roadway capacities, and resulting levels of service, were established through use of spreadsheets associated with the El Dorado County model.
- Roadway geometries, and corresponding capacities, for cumulative conditions are based on the roadway network geometries included within the 2022 CIP (Capitol Improvement Program) data network files contained with the 1999 El Dorado County travel demand model.
- A roadway was assumed to be impacted, but not necessarily "significantly impacted" by the project, if it added more than 2% to the existing roadway volume.
- A roadway was assumed to be significantly impacted by the project if it also met any of the following criteria:
 - a) Degrading from acceptable LOS (A,B or C) without the project to an unacceptable LOS (D,E or F) with the project

- b) Degrading from unacceptable LOS D without project to unacceptable LOS E with the project, when the cumulative LOS is D or better (as defined within the 2015 CIP)
- c) Degrading from unacceptable LOS D without project to unacceptable LOS F with project
- d) Degrading from unacceptable LOS E without project to unacceptable LOS F with the project
- e) Unacceptable LOS F both without and with the project

Impacts along US-50 from the Sacramento/El Dorado County line to east of the project site are based on the concept level of service E, established within *Caltrans' State Route 50 Transportation Concept Report* and additional information provided by Caltrans. Thus LOS F is considered an unacceptable level of service for US-50.

METHODOLOGY

OVERVIEW

The traffic operations analysis included within this study evaluates the following:

- Freeway Mainline Operations
- Freeway Ramp Merge/Diverge Operations
- Freeway Auxiliary Lane/Weaving Analysis
- Interchange Intersection Operations
- Interchange Queuing
- Freeway Ramp Metering
- Local Roads
- Accidents

ANALYSIS SCENARIOS

Traffic operations associated with the proposed interchange are analyzed for the following three peak hour scenarios, as established through coordination with Caltrans:

- Weekday AM Peak Hour
- Weekday PM Peak Hour
- Saturday Peak Hour

The peak hour scenarios which are selected for analysis are based on the peaking characteristics of US-50, as well as the peaking characteristics of the casino. Although casinos generally do not generate a significant amount of traffic during weekday AM peak hours (7-9 AM), the AM peak hour was analyzed along with the weekday PM peak hour (4-6 PM) to determine operations during the peak hours of operation along US-50.

It should be noted that the peak hour of trip generation for a casino through the week is typically in the early evening hours, which include the 4-6 PM time period. The Saturday peak hour was also selected for analysis in recognition of the fact that casino trip generation is typically highest on weekends, and particularly on Saturday. For this analysis, the Saturday peak hour is assumed

to be an amalgamation of the peak hour of regional background traffic (i.e. the hour when traffic volumes on US-50, etc. are heaviest) and the peak hour of trip generation by the casino. Since traffic volumes are relatively uniform on a Saturday, no specific peak hour is designated. Similarly, no specific peak hour is designated for casinos on a Saturday, although casino generated traffic typically tends to peak in the early evening hours. For these reasons, Saturday peak hour volumes are established by combining background traffic occurring during the busiest hour on a Saturday (i.e. when traffic volumes are heaviest on US-50) with the traffic generated by the casino during the casino's busiest hour. This combination creates a worst case scenario for analysis. Saturday peak hour traffic is described in more detail in subsequent sections.

Traffic operations are also analyzed for both of the following scenarios:

- Existing Conditions
- Cumulative (Year 2025) Conditions

For the local roads analysis, impacts are analyzed for daily conditions.

TRAFFIC OPERATIONS ANALYSIS METHODOLOGY

Freeway Mainline Analysis

Methodologies described in Chapter 3 of the *1997 Highway Capacity Manual* were used to analyze level of service operations for basic freeway segments. Highway Capacity Software (HCS) was used to perform freeway mainline analysis. The level of service for a freeway section is based on the density of the traffic.

In analyzing freeway mainline operations, the free flow speed along the freeway mainline was established by adjusting the assumed "ideal" free flow speed of 65 mph, which equals the freeway's posted speed limit. This adjustment takes into account specific highway geometry such as lane width, right-shoulder lateral clearance, interchange density and number of lanes. This methodology was used instead of using a "measured" free flow speed which could be obtained through field surveys.

Freeway mainline analysis within this report assumes that 6% of traffic volumes along US-50 in the vicinity of the proposed interchange are trucks, which reflects truck percentages as reported in Caltrans' *1997 Annual Average Daily Truck Traffic on the California State Highway System*. In addition, it was assumed that an additional 3% of traffic volumes are recreational vehicles, and that an additional 1% of traffic volumes are buses. These percentages were used for all peak hour scenarios for both existing and cumulative conditions.

It is expected that driver characteristics will differ between weekday peak hour conditions and Saturday conditions along US-50. During the weekday peak hours, it is assumed that a significant percentage of drivers are commuter oriented, travel the same route daily, and thus are more familiar with the route. However on Saturdays, it is assumed a much larger percentage of the drivers along the route are more recreation oriented, don't travel the route as frequently, and thus are less familiar with the route. This difference in travel characteristics results in a lessened capacity during the weekend. To account for this characteristic, freeway mainline analysis includes a "Driver Population Adjustment Factor" which allows for an adjustment to the freeway's capacity. For this analysis, the default Driver Population Adjustment Factor of 1.0 is used for weekday peak hour

conditions, whereas a factor of 0.9 is used for Saturday conditions. This reduced factor results in a more conservative analysis for weekend conditions. These same factors are also used for ramp merge/diverge analysis and weaving zone analysis described below.

Table 7 provides a LOS breakdown for freeway volumes. Different values are provided for weekday peak hour conditions (where a factor of 1.0 was used) and Saturday conditions (when a factor of 0.9 was used).

Table 7
Freeway Level of Service Breakdown

Scenario	A	B	C	D	E
Threshold Volume					
Weekday AM & PM Peak Hour Conditions (Driver Population Adjustment Factor = 1.0)	884	1,414	2,121	2,768	3,503
Threshold Volume					
Saturday Peak Hour Conditions (Driver Population Adjustment Factor = 0.9)	796	1,274	1,910	2,493	3,155

Note: As calculated using Highway Capacity Software

Freeway Ramp/Merge Diverge Analysis

Methodologies described in Chapter 5 of the *1997 Highway Capacity Manual* were used to analyze level of service operations for freeway merge/diverge areas. Highway Capacity Software (HCS) was used to perform freeway ramp merge/diverge analysis. The level of service for freeway merge/diverge areas is based primarily on the density of traffic in the "influence area" of the ramp, which is defined as the 1,500 ft. section of freeway immediately following an on-ramp or preceding an off-ramp, and includes the acceleration/deceleration lane and two adjacent freeway mainline lanes. A secondary measure of level of influence for merge/diverge areas is the minimum speed experienced in the influence area. When the mainline freeway volumes, combined with oncoming traffic from an on-ramp, exceeds the discharge capacity of the freeway downstream from the ramp, the system is defined as unstable, or level of service F. Similarly, level of service F is experienced when demand along an off-ramp exceeds the ramp's capacity resulting in backup onto the freeway itself.

Freeway ramp merge/diverge analysis were analyzed two different ways to provide for a comparative analysis, and to account for the various alternatives which are currently being proposed. The first method of analysis calculates ramp merge/diverge operations for rolling terrain, and does not take into account the specific grades and grade lengths; whereas the second method uses actual specific grades and grade lengths of the freeway leading up to the ramp, and along the ramp itself.

For detailed analysis which takes into account the specific grade and grade lengths of the freeway and ramp in establishing ramp merge/diverge operations, the grade length is calculated based on the following methodology, which was outlined in the *1997 Highway Capacity Manual*. The total grade length is established by taking the length of the constant grade portion of the highway leading to (and perhaps continuing slightly past) the ramp juncture point, and adding to that ¼ of the length

of the vertical curve at each end of the constant grade portion. The specific grade is taken to be the grade along the constant grade portion. By calculating the specific grade and grade length along the freeway in this manner, minor differences in the location of the ramp juncture points for various alternatives do not change the length of the grade, since the ramp juncture point location is not directly used to determine the length of the grade. Therefore, the same specific grade and grade length along the freeway are applicable to analyses for both project alternatives.

Although level of service analysis as included within HCS also provides for the input of specific ramp grade and length, this data is negligible in terms of the analysis unless the volume along the ramp approaches the capacity of the ramp. Within the analyses included within this report, it was found that the specific grade and length of the ramps itself were negligible in terms of the level of service of the ramp merge/diverge. To establish variances in merge/diverge levels of service for both project alternatives, a detailed analysis was performed for Alternative AB, and a trial and error input of the other possible input values for the Alternative AC. This trial and error input included the ramp lengths and grades for Alternative AC, as well as extreme ramp lengths and grades outside the range of current design values for Alternative AC. This trial and error analysis confirmed that the merge/diverge levels of service for both alternatives and peak hour scenarios correspond to those calculated for Alternative AB. Therefore, for analyses which provide for the input of specific grades and grade lengths, the same values are used for both alternatives since the input for each result in exactly the same level of service results.

Additionally, a free flow speed of 48 kph (30 mph) was used for both alternatives. This default value was used to approximate the following design speeds along the ramps:

- Alternative AB – Design speed of 80 kph (50 mph) at inlet and exit noses, and 45 kph (28 mph) along remainder of ramp.
- Alternative AC – Design speed of 80 kph (50 mph) at inlet and exit noses, and 40 kph (25 mph) along remainder of ramp.

Ramp merge/diverge analyses are performed only along the peak direction of travel for weekday peak hour conditions, which is along the westbound direction during the AM peak hour, and the eastbound direction during the PM peak hour. Ramp merge/diverge analyses are performed along both directions of travel for the Saturday peak hour.

Freeway ramp analysis within this report assumes that 2% of traffic volumes along the new ramps will be trucks. Although 6% of traffic volumes along the US-50 mainline are assumed to be trucks as described above, the percentage of trucks along the ramps would not be as high due to the Rancheria land uses. In addition, it was assumed that 3% of traffic volumes along the ramps would be recreational vehicles, and that 1% of traffic volumes would be buses. These percentages were used for all peak hour scenarios for both existing and cumulative conditions.

Freeway ramp merge/diverge analysis also provides for the consideration of interference to traffic flow and capacity based on the location of adjacent freeway ramps, and the traffic volumes to and from them. For 4-lane freeway merge/diverge analysis, the length to the adjacent ramp and the volume on the adjacent ramp does not impact the result in anyway since the equation used to calculate level of service does not include this variable.

In the case of an off-ramp, the adjacent ramp can be taken to be either the on-ramp within the nearest interchange upstream of the off-ramp being analyzed, or the on-ramp immediately downstream within the same interchange. In the case of an on-ramp, the adjacent ramp can be taken to be either the off-ramp within the nearest interchange downstream of the on-ramp being analyzed, or the off-ramp immediately upstream within the same interchange. The Highway Capacity Manual recommends that both analyses be performed. However, separate analysis was not performed for this (4-lane freeway) analysis since the distance between ramps is not relevant for merge/diverge analysis as discussed above.

Auxiliary Lane/Weaving Analysis

For scenarios where an auxiliary lane is proposed to provide acceptable levels of service, a weaving analysis was performed. Weaving analysis describes operations when traffic flows conflict with and cross each other to and from the freeway and ramps. If auxiliary lanes are proposed, weaving becomes the controlling factor in establishing the level of service for the ramp merge/diverge area due to the unique and often severe turbulence which is experienced within sections including auxiliary lanes.

Two different methodologies were utilized to analyze level of service operations for the weaving zone created by a new auxiliary lane: (1) methodologies as included within Caltrans' *Highway Design Manual*; and (2) methodologies described in the *1997 Highway Capacity Manual*. The first methodology, which is the methodology preferred by Caltrans, is described within Section 504.12 – Weaving Sections of the Caltrans' *Highway Design Manual*. The Highway Design Manual states that "a rough rule of thumb for length of a weaving section is one foot of length per weaving vehicle." A more detailed weaving analysis methodology is also included within the Caltrans' *Highway Design Manual* utilizing the nomograph depicted in Figure 504.7A. Although the nomograph can be used to determine the necessary weaving section length, it is intended more for design purposes. The output of the nomograph describes necessary weaving section length and number of lanes for given traffic volumes and a desired level of service. However, the nomograph can also be used to test proposed design and traffic volume characteristics, and determine if these result in an acceptable level of service, which was done for this analysis.

Caltrans weaving analysis methodologies were supplemented by methodologies described in Chapter 4 of the *1997 Highway Capacity Manual*, which were analyzed using Highway Capacity Software (HCS). This methodology establishes that the level of service associated with the weaving sections of the freeway is based on the average density of both weaving and non-weaving vehicles within the weaving lanes. The average density is calculated by first establishing the average (space mean) speed of all vehicles in the weaving section. The average density is then calculated by dividing total volumes by the average (space mean) speed, and converting results into passenger cars per mile per lane. The breakdown of operation within the weaving area of a freeway, or Level of Service F, is defined as conditions resulting in densities greater than 43 vehicles per mile per lane.

The *1997 Highway Capacity Manual* specifies that for weaving sections less than 2,500 ft. in length, weaving methodologies as described in Chapter 4 of the *Highway Capacity Manual* should be used, whereas for weaving sections greater than 2,500 ft. in length, ramp merge/diverge methodologies as described in Chapter 5 of the *Highway Capacity Manual* (and

above) should be used. However, for this study, Chapter 4 methodologies were used to analyze weaving along the eastbound lanes preceding the new interchange's eastbound off-ramp for both alternatives even though weaving section lengths may technically be slightly greater than 2,500 ft.

Unsignalized Intersection Analysis

Stop-controlled intersections were analyzed using the methodology outlined in the Transportation Research Board's Special Report 209, *Highway Capacity Manual, 1994*. This methodology calculates an average total delay per vehicle for each controlled movement and for the intersection as a whole. A level of service designation is assigned based upon the delay. Table 8 presents the relationship of total delay to level of service for unsignalized intersections. Intersection levels of service reported in this analysis are presented for the overall weighted level of service for the intersection as a whole, as well as for the worse case movement.

Table 8
Level of Service Criteria
Unsignalized Intersections

Level of Service	Total Delay per Vehicle (Seconds)	Description
A	0 - 5.0	Little or no delay
B	5.1 - 10.0	Short traffic delay
C	10.1 - 20.0	Average traffic delays
D	20.1 - 30.0	Long traffic delays
E	30.1 - 45.0	Very long traffic delays
F	> 45.0	Extreme delays potentially affecting other traffic movements in the intersection

Source: *Highway Capacity Manual*, Transportation Research Board, Special Report No. 209, Washington, D.C., 1994.

Signalized Intersection Analysis

Signalized intersection analyses were conducted using a methodology outlined in the Transportation Research Board's Special Report 209, *Highway Capacity Manual, 1994*. The methodology utilized is known as "operations analysis". This procedure calculates an average stopped delay per vehicle at a signalized intersection, and assigns a level of service designation based upon the delay. The method also provides a calculation of the volume-to-capacity (v/c) ratio of the critical movements at the intersection.

Table 9
Level of Service Criteria
Signalized Intersections

Level of Service	Stopped Delay per Vehicle (sec)	Description
A	0 - 5.0	Very low delay. Occurs when progression is extremely favorable and most vehicles arrive during the green phase. Most vehicles do not stop at all. Short cycle lengths may also contribute to low delay.
B	5.1 - 15.0	Generally occurs with good progression, short cycle lengths, or both. More vehicles stop than with LOS "A," causing higher levels of average delay.
C	15.1 - 25.0	These higher delays may result from fair progression, longer cycle lengths, or both. Individual cycle failures may begin to appear at this level. The number of vehicles stopping is significant at this level, though may still pass through the intersection without stopping.
D	25.1 - 40.0	The influence of congestion becomes more noticeable. Longer delays may result from some combination of unfavorable progression, long cycle lengths, or high v/c ratios. Many vehicles stop, and the proportion of vehicles not stopping declines. Individual cycle failures are noticeable.
E	40.1 - 60.0	These high delay values generally indicate poor progression, long cycle lengths, and high v/c ratios. Individual cycle failures are frequent occurrences.
F	> 60.0	This level, considered to be unacceptable to most drivers, often occurs with oversaturation, that is, when arrival flow rates exceed the capacity of the intersection. It may also occur at high v/c ratios below 1.0 with many individual cycle failures. Poor progression and long cycle lengths may also be major contributing causes to such delay levels.

Source: *Highway Capacity Manual*, Transportation Research Board, Special Report No. 209, Washington, D.C., 1994.

Queuing and Signal Coordination Analysis

Closely spaced intersections, such as those proposed for the new interchange, do not operate the same as isolated intersections with similar volumes. The HCM methodologies described above, and corresponding level of service grades, do not take into account closely spaced intersections, the effects of queuing on upstream intersections, coordinated signal systems, etc.

More advanced analysis methodologies such as SYNCHRO computer software provide the capability to simulate and analyze a coordinated signal system, analyze queues, and determine the intersection level of service. This process allows for a secondary analysis of intersection operations, and the analysis of the roadway corridor itself. Thus, a SYNCHRO analysis was performed to assure recommended intersection and roadway geometry will accommodate projected traffic queues.

Signal Warrant Analysis

For intersections where signalization is recommended, a Caltrans signal warrant analysis for rural conditions was performed for all three peak hour scenarios.

Ramp Metering Analysis

Ramp metering analysis was performed for 2010 conditions along the proposed eastbound and westbound on-ramps using analysis methodologies as described within Caltrans' *Ramp Meter Design Manual, 2000*. The analysis was performed for 2010 conditions, and 2010 conditions only, at the direction of Caltrans staff. Caltrans staff have indicated that ramp metering analysis is typically not done for long range (i.e. 20 year) planning, but rather is performed for near term conditions (typically about 5 years after the planned completion of construction). Since there is not a near term plan for ramp metering within this section of US-50, Caltrans requested that ramp metering be investigated as a potential interim solution.

Ramp metering analysis assumes that the proposed on-ramps would be metered in the cumulative condition. The purpose of the analysis is to determine if the minimum metering rate permitted by Caltrans can accommodate maximum vehicle queues within the available storage length of the on-ramps.

In performing this analysis, metered single-lane entrance ramps were considered for the on-ramps.

The peak hour volume for the Saturday conditions was used as a worst case condition since the volumes along the ramps are highest compared to other peak hour periods. The ramp storage length was determined by measuring the length directly from the design layout of the new interchange. Also, minimum vehicle spacing of 9 meters (29.5 ft) was used in considering storage length on metered ramps.

Local Roads Analysis

To perform an analysis of potential impacts to local roads, the methodology and study approach outlined in the standards of significance section, as established in coordination with El Dorado County traffic engineering personnel, was used.

Accident Analysis

An analysis of accidents over the past 4 years was conducted along US-50 between the East Shingle Springs Drive and Greenstone Road interchanges for Existing No Project conditions only. The analysis compares the accident rate occurring along this stretch of highway with the statewide average for a similar roadway facilities.

INTERCHANGE DESIGN

There are two "project" alternatives under consideration for the interchange (not including the "no project" Alternative AA):

- Alternative AB – "Flyover Alternative" - Modified trumpet design (Type L-11)
- Alternative AC – "Diamond Alternative" - Modified tight diamond w/ no right-of-way taking (Type L-1)

Both of these project alternatives are "modified" due to the absence of the southside leg.

For purposes of the analysis contained within this study, the relative positions of the ramp juncture points are relatively close to each other. Table 10 lists the metric station locations of these ramp juncture points, as well as the centerline of the new bridge structure, where applicable, for each alternative.

**Table 10
New Ramp Stations**

Alternative	Metric Stations ⁽¹⁾				
	Eastbound Off-Ramp	Eastbound On-Ramp	Westbound Off-Ramp	Westbound On-Ramp	Bridge Centerline
Alternative AB	31+85	38+00	37+70	32+45	na
Alternative AC	32+43	38+00	38+13	32+20	34+90

Notes:

- (1) Station identifying beginning of on-ramps located 100 meters (328 ft) upstream of gore point, and station identifying beginning of off-ramps located at gore point.

Table 11 provides the freeway segment length (or weaving length) between new interchange ramps and the adjacent ramps at the East Shingle Springs Drive and Greenstone Road interchanges, as defined within Caltrans' *Highway Design Manual*.

**Table 11
US-50 Freeway Segment Lengths**

US-50 Freeway Segment	Distance between Ramps ⁽¹⁾	
	Meters	Feet
Alternative AB – “Flyover Alternative” –		
<u>Modified trumpet design (Type L-11)</u>		
EB US-50 btwn E. Shingle Springs on-ramp & Rancheria off-ramp	791.5	2,597
EB US-50 btwn Rancheria off-ramp & on-ramp	615.0	2,018
EB US-50 btwn Rancheria on-ramp & Greenstone off-ramp	1,296.3	4,253
WB US-50 btwn Greenstone on-ramp & Rancheria off-ramp	696.3	2,285
WB US-50 btwn Rancheria off-ramp & on-ramp	552.0	1,723
WB US-50 btwn Rancheria on-ramp & E. Shingle Springs off-ramp	867.1	2,845
Alternative AC – “Diamond Alternative” –		
<u>Modified tight diamond w/ no right-of-way taking (Type L-1)</u>		
EB US-50 btwn E. Shingle Springs on-ramp & Rancheria off-ramp	849.5	2,787
EB US-50 btwn Rancheria off-ramp & on-ramp	557.0	1,828
EB US-50 btwn Rancheria on-ramp & Greenstone off-ramp	1,296.3	4,253
WB US-50 btwn Greenstone on-ramp & Rancheria off-ramp	653.3	2,143
WB US-50 btwn Rancheria off-ramp & on-ramp	593.0	1,946
WB US-50 btwn Rancheria on-ramp & E. Shingle Springs off-ramp	842.1	2,763

Notes:

- (1) Freeway segment lengths measured from 100 meters (328 ft) upstream of on-ramp gore pts to gore points of off-ramps. Slightly different values used for ramp merge/diverge analysis since 1997 Highway Capacity Manual methodologies use distance between ramps measured from gore point to gore point for merge/diverge analysis.

The proposed diamond interchange would consist of only a “T” intersection on the southside of the interchange, since no access is proposed south of US-50. This new “T” intersection would consist of the eastbound on and off ramps, and the new access road which would connect the eastbound ramps with the Rancheria. The new intersection along the northside of the interchange would be a standard 4-legged freeway ramp intersection consisting of the westbound on and off ramps, and the new access road.

Each of the intersections is currently designed to include single lane approaches, with the exception of the southbound approach along the 4-lane access road connecting the Rancheria with the westbound ramps, where a separate through lane and right turn lane would exist. **Figure 1** depicts the proposed lane geometrics.

BASELINE RANCHERIA TRAFFIC

Existing Baseline Rancheria Traffic

The provision of a new freeway interchange which will provide access to the existing Rancheria will result in a slight adjustment of traffic on roadways surrounding the project site. Vehicles currently access the Rancheria site from Greenstone Road via the private roadways of Grassy Run Road, Rolling Rock Road, and Reservation Road. Following the construction of the new interchange, these vehicles would likely shift over to the interchange. A count of traffic along

Figure 1

the route currently used to access the site was conducted early 1999, and showed that during the PM peak hour approximately 15 inbound vehicles and 7 outbound vehicles were travelling to and from the Rancheria (and adjacent private development). These volumes were reassigned to the new interchange and US-50 to create an adjusted baseline weekday PM peak hour scenario. AM peak hour and Saturday peak hour volumes throughout the study area were similarly adjusted to reflect rerouted trips during these peak hour periods. Newly generated trips from the proposed casino/hotel were added to these baseline Rancheria volumes.

Cumulative Baseline Rancheria Traffic

Baseline Rancheria traffic is expected to grow along with background traffic within the rest of the El Dorado County area. For this analysis, an annual growth rate of 1% per year was assumed and added to existing baseline Rancheria peak hour traffic volumes to establish cumulative baseline Rancheria volumes. The 1% annual growth rate was established through consultation with officials of the tribal council who provided information regarding historical growth, and projected growth within the Rancheria. Rather than increase each of the individual turning movements, the inbound/outbound volumes were grown by a compounded growth rate of 1% per year. These were then proportioned out among the intersection turning movements at each of the ramp intersections. Newly generated trips from the proposed casino/hotel were added to these baseline Rancheria volumes.

CASINO TRAFFIC VOLUMES

This section establishes the trips which would be generated by the proposed casino/hotel development. This is necessary since the casino development will comprise nearly all of the traffic volumes for the interchange. To establish total volumes for the proposed interchange, the casino/hotel volumes established within this section are added to baseline Rancheria traffic volumes which are generated by the remainder of the Rancheria.

Casino Project Description

The proposed casino project, as currently proposed, consists of development of a hotel/casino within the southernmost portion of the existing Rancheria, located a few hundred feet north of the newly proposed US-50 interchange.

The proposed project would include a 238,500 sq. ft. casino complex, a 250 room (142,750 sq. ft.) hotel, and 37,400 sq. ft. of convention/event center development. Included within the 238,500 sq. ft. casino complex would be 82,800 sq. ft. of casino gaming floor area, 43,300 sq. ft. of food and beverage facilities (with total seating capacity of 1,425 seats), banking and administration facilities, child care/family fun room, and retail. The gaming floor area is proposed to contain 3,000 gaming positions.

The analysis contained within this study is a follow up to previous traffic studies which were performed for the development of the casino/hotel complex. These earlier studies analyzed traffic impacts for a variety of alternatives which included a larger casino development, and a location other than that currently proposed (including the south side of US-50 at the southeast

corner of the US-50/E. Shingle Springs Drive interchange). Some of the analysis included within this report is based on information obtained for earlier versions of the proposed casino project.

Casino Project Trip Generation Methodology

Trip generation relates land uses to the number of persons or vehicles entering or exiting the site. To estimate the amount of traffic which would be added to roadways surrounding and serving the casino project site, the trip generation (in terms of vehicles) is established by multiplying some measurable aspect of the site (i.e. square footage, employees) by corresponding trip generation rates established for similar land uses. The directional split of traffic between inbound and outbound traffic is also quantified.

Although there is significant information available regarding trip generation for casinos, most of this information is for more traditional casinos such as those found in Reno, Las Vegas, or Atlantic City. The best reference from which to determine trip generation, The Institute of Transportation Engineers (ITE) *Trip Generation Manual*, does include trip generation information for casinos, however they are based on only a few locations, and casinos significantly different in nature than the proposed project.

Trip generation information for Indian gaming style casinos are not readily available due to their unique trip generation characteristics compared to those of more traditional casinos. These differences are due to the type of gaming, isolated locations, etc. Although trip generation characteristics for non Indian gaming casinos were not used directly to establish trip generation for the proposed project, information from these sources were utilized to verify trip generation assumptions.

Two very different approaches were used to establish the trip generation for the proposed casino. The first approach was to review the marketing study performed for an earlier, and larger, version of the project by Urban Systems. The marketing study established potential trips to the originally proposed casino to provide a basis from which potential casino revenues could be generated. These potential trips were factored down within this analysis to account for the reduced square footage currently proposed. Although it would be unwise to use these estimates alone from a traffic impact analysis perspective, they are useful for providing a baseline from which trip generation numbers from other means of analysis can be compared. The second approach for establishing trip generation rates for the casino is to investigate trip generation characteristics at other casinos, including both information within traffic studies for other Casinos, and the results of surveys conducted at two northern California Indian gaming casinos for this project.

To establish a trip generation rate at existing casinos, vehicular traffic both entering and exiting the sites are counted during the peak time periods for which a peak hour rate is desired. To account for the seasonal variations experienced by casinos, these rates require adjustment to establish trip generation for peak month conditions. The March 1999 ITE Journal article titled "*Gaming Casino Traffic*" (authored by Paul C. Box and William Bunte) summarizes the results of year long traffic counts at St Louis, Missouri area casinos. This article provides for the factoring of traffic counts or trip generation rates collected during an off-peak month to a peak month condition. Although some variations may occur from region to region, they do seem to reasonably reflect seasonal variations which would be experienced at northern California casinos.

According to these factors, the peak months for a casino are generally the summer months of May through August. Counts collected during the remaining months are factored up by 1.1-1.3 to convert values to peak month conditions. Casino patronage is particularly low during the winter months between October and February when a 1.2-1.3 factor is required to convert to peak month conditions.

Available traffic studies which provided useful trip generation survey results for existing casinos were:

- 1) *Mississippi Gulf Coast Transportation Management Plan for Waterfront Development*, Gulf Regional Planning Commission, Gulfport, MS, June, 1993.
- 2) *Auburn Rancheria Gaming Facility Traffic Study*, Fehr and Peers, June 1, 1999.

Independent Variable

To establish trip generation rates for any type of development, it is necessary to establish the variable against which actual trip volumes will be compared to establish trip generation rates. For casinos, trip generation is typically calculated based on one of the following variables:

- Entire Casino square footage
- Casino Gaming Floor square footage
- Gaming positions
- Employees

Available trip generation information from available sources use primarily the square footage of the entire casino or the square footage of the gaming floor area itself. Unfortunately, some trip generation information is not clear regarding whether trip generation rates were derived using the square footage for the entire casino, only the gaming portion of the casino, or other fractional parts of the casino or casino-hotel complex. Therefore, great care must be used when reviewing other studies. The traffic studies for the Mississippi Casino calculated trip generation based on only the gaming floor area. Although trip generation from the Auburn Rancheria Gaming Facility Traffic Study Information might erroneously be construed as using only the square footage of the gaming floor area to establish casino trip generation rates, in reality it established rates using the assumed entire square footage of the casino. Unfortunately, it is difficult to establish with complete certainty the square footage of other casinos due to the reluctance of casinos to divulge this information, and the lack of public information such as building plans since the casinos are on sovereign rancherias. A best estimate of the square footages of the casinos surveyed were established based on all available information and contacts with the respective casinos.

Casino Project Trip Generation Rates

As described earlier, two very different approaches were used to establish trip generation rates for the project.

Marketing Study

The first approach used to establish trip generation rates was a review of data within the Urban Systems Marketing Study. Although the market study established potential trips to the casino to provide a basis from which potential casino revenues could be generated, these rates provide a baseline for comparison with other trip generation rates established through actual traffic counts at other casinos.

As previously described, the marketing study was conducted for a slightly different project consisting of a slightly larger casino complex at a different location. The following details the trip generation established for the earlier version of the project, and the adjustments to convert trip generation to the smaller facility currently proposed.

Table 12a provides a summary of the trips which were projected by the marketing study for the previously proposed casino development during a weekday, weekday PM peak hour, Saturday, and Saturday peak hour during both an average month and the peak month. Trip generation rates are based on the total size of the previously proposed casino (282,600 square feet) which excludes the 250 room hotel and convention/event center development. The trip generation associated with these ancillary uses are described later in this section.

Table 12a
Trip Generation from Marketing Study
(for previously proposed 282,600 sq. ft. Casino)

Trip Type	Weekday		Saturday	
	Average	Peak Month	Average	Peak Month
<u>Daily Vehicle Trips</u>				
Visitors & Employees	8,326	11,102	13,878	16,652
Buses & Deliveries	40	40	40	40
TOTAL TRIPS	8,366	11,142	13,918	16,692
DAILY	29.60	39.43	49.25	59.07
TRIP GENERATION RATE	trips/ksf	trips/ksf	trips/ksf	trips/ksf
<u>PM Peak Hour Trips</u>				
Visitors & Employees	830 - 970	1,110 - 1,390	1,390 - 1,530	1,660 - 1,940
Buses & Deliveries	10	10	10	10
TOTAL TRIPS (Maximum)	980	1,400	1,540	1,950
PM PEAK HOUR	3.47	4.95	5.45	6.90
TRIP GENERATION RATE	trips/ksf	trips/ksf	trips/ksf	trips/ksf

Notes: ksf = 1,000 square feet

Source: *Urban Systems Marketing Study*

It was assumed that a total of 20 buses and deliveries would be generated by the site on an average day, which accounts for an additional 40 trips per day (1 trip inbound plus 1 trip outbound for each bus and delivery). The Urban Systems Marketing Study assumed that 9 buses

per day would bring visitors to the site, and information provided by the shipping and receiving department of a similar northern California Indian gaming casino revealed that they experience an average of 9 deliveries a day. A conservative estimate of 5 buses and deliveries a day (25% of the daily total) was assumed for peak hour conditions.

Based on the marketing study, it was projected that a 282,600 sq. ft. casino would generate 29.60 trips per 1,000 square feet (ksf) of casino during an average weekday, and 3.47 trips/ksf during the PM peak hour of an average weekday. During the peak month, these rates increase to 39.43 trips/ksf and 4.95 trips/ksf, respectively. On an average Saturday, it is projected that a 282,600 sq. ft. casino would generate 49.25 trips/ksf, and 5.45 trips/ksf during the average Saturday peak hour. During the peak month, these rates increase to 59.07 trips/ksf and 6.90 trips/ksf, respectively

These same trip generation rates were applied to the newly proposed, and slightly smaller, casino. The casino as currently proposed is 238,500 sq. ft., which is approximately 16% smaller than the 282,600 sq. ft. casino previously proposed. Since the same trip generation rates were used, the number of trips which would be associated with the smaller casino were also assumed to be approximately 16% less than those reported in the marketing study. The trip generation for the currently proposed 238,500 sq. ft. casino are summarized in Table 12b.

Table 12b
Trip Generation based on Marketing Study
(for currently proposed 238,500 sq. ft. Casino)

Trip Type	Weekday		Saturday	
	Average	Peak Month	Average	Peak Month
<u>Daily Vehicle Trips</u>				
DAILY TRIP GENERATION RATE	29.60 trips/ksf	39.43 trips/ksf	49.25 trips/ksf	59.07 trips/ksf
TOTAL TRIPS	7,060	9,403	11,746	14,088
<u>PM Peak Hour Trips</u>				
PM PEAK HOUR TRIP GENERATION RATE	3.47 trips/ksf	4.95 trips/ksf	5.45 trips/ksf	6.90 trips/ksf
TOTAL TRIPS (Maximum)	827	1,181	1,300	1,646

Notes:
ksf = 1,000 square feet
Source: *Urban Systems Marketing Study*

Existing Casino Surveys

The second approach used for establishing trip generation rates for the casino was to investigate trip generation characteristics at other casinos, including both information within traffic studies for other casinos, and the results of surveys conducted at two northern California Indian gaming casinos specifically for this project.

David Evans and Associates, Inc. conducted surveys of two Indian gaming casinos in the vicinity of the proposed project during both the PM peak period of a weekday, and the assumed peak hour period for a Saturday (late afternoon/early evening). One of the two casinos was also surveyed during a weekday AM peak period. At the request of the casinos surveyed, their identities are not divulged in this study. Traffic counts were collected at one northern California casino (Survey location A in **Table 13**) during the months of December and April. Utilizing monthly variation factors from the March 1999 ITE Journal article "*Gaming Casino Traffic*", the December weekday PM peak hour counts were factored by 1.2, and the April Saturday peak hour counts were factored by 1.1 to establish peak month rates. Traffic counts were collected at a second northern California casino (Survey location B in **Table 13**) during the month March for all three peak periods. Counts at this casino were factored by 1.1 to establish peak month rates.

Trip generation rates for three other northern California casinos were also used as reported within the traffic study performed for the proposed Auburn Rancheria Gaming Facility in Placer County. This traffic study, which was issued by Fehr and Peers on June 1, 1999, surveyed a total of four northern California Indian gaming casinos, one of which was also surveyed by David Evans and Associates, Inc. These surveys only analyzed trip generation during the PM peak hour of a weekday. Trip generation rates reported within this traffic study were also for peak month conditions following the factoring of raw counts utilizing the "*Gaming Casino Traffic*" article monthly variation factors.

Table 13 provides a summary of the trip generation rates and inbound/outbound directional splits found for the two casinos surveyed by David Evans and Associates, Inc., and the three additional casinos surveyed by Fehr and Peers. **Table 13** also provides a summary of the trip rates established from the marketing study for comparison. Due to the confidential nature of the surveyed casinos, their identities are designated simply as survey locations A through E.

**Table 13
Casino Trip Generation Rates**

Survey Location	Estimated Size (sq. ft.)	Trip Generation Rates (vehicle trips per 1,000 square feet of Casino)					
		Weekday AM Peak Hour		Weekday PM Peak Hour		Saturday Peak Hour	
		in / out	Total	In / out	Total	in / out	total
		Split	Rate	Split	Rate	Split	rate
Market Study⁽¹⁾							
Average	282,600 ⁽¹⁾	---	---	---	3.47	---	5.45
Peak	282,600 ⁽¹⁾	---	---	---	4.95	---	6.90
Casino Surveys							
A ⁽²⁾	78,000	---	---	45% / 55%	2.67	36% / 64%	5.86
B ⁽³⁾	50,000	70% / 30%	3.02	55% / 45%	5.06	56% / 44%	8.08
C ⁽⁴⁾	32,400	---	---	44% / 56%	9.26	---	---
D ⁽⁴⁾	20,000	---	---	67% / 33%	8.80	---	---
E ⁽⁴⁾	17,300	---	---	56% / 44%	13.83	---	---
Average in/out Split		70% / 30%		53% / 47%		46% / 54%	
Weighted Average Rate		3.02		5.95		6.73	
Final Trip Rate (trips/ksf of Casino)		70% / 30%		2.95 ⁽⁵⁾		53% / 47%	
				4.95		46% / 54%	
						6.90 ⁽⁶⁾	

Notes:

- (1) = Casino size from market study based on previously proposed casino square footage of 282,600 sq. ft.
- (2) = Weekday PM peak hour count conducted at Site A in December, factored by 1.2 to peak month.
Saturday peak hour count conducted at site A in April, factored by 1.1 to peak month.
- (3) = Peak hour counts conducted at site B in March, factored by 1.1 to peak month.
- (4) = Information from Auburn Rancheria Gaming Facility traffic study, Fehr & Peers, June 1, 1999.
PM Peak hour count conducted at site C, D & E in October, factored by 1.2 to peak month.
- (5) = AM peak hour trip rate assumed to be 60% of PM peak hour rate based on comparison of AM and PM rates at site B (60% of 5.06 = 3.02)(60% of 4.95 = 2.95).
- (6) = Saturday peak hour trip rate based on Market Study.

The final trip rate for each peak hour scenario was established separately using available information and methodologies. Inbound/Outbound directional splits were established for each peak hour by averaging the directional splits at surveyed casinos for each respective peak hour.

Use of the weighted average to establish trip rates is justified based on the following precedents:

- (1) A similar methodology using a "weighted average" was used by Fehr and Peers within their traffic study for the Auburn Rancheria. A PM peak hour rate was established within that report by using a "weighted average" of PM peak hour rates at four northern California casinos.
- (2) The member of the technical advisory committee which helped to put together the report prepared for the Institute of Transportation Engineers titled "Casino Trip Generation"

verified that this methodology was valid for determining reasonable trip generation for the project.

- (3) Page 7 of the Institute of Transportation Engineer's "Trip Generation Handbook" published October, 1998 states:

The data presented in Trip Generation allow for several trips of analyses of trip generation data for each combination of land use type, independent variable, and time period.

Weighted Average Trip Generation Rate: This rate is defined as the number of weighted trip ends per unit of the independent variable. The rate simply assumes a linear relationship between trip ends and the independent variable, having a slope equal to the rate and with the straight line passing through the origin (i.e., with a value of zero for the independent variable, the number of trips generated in zero). The averages (weighted by the units of the independent variable).

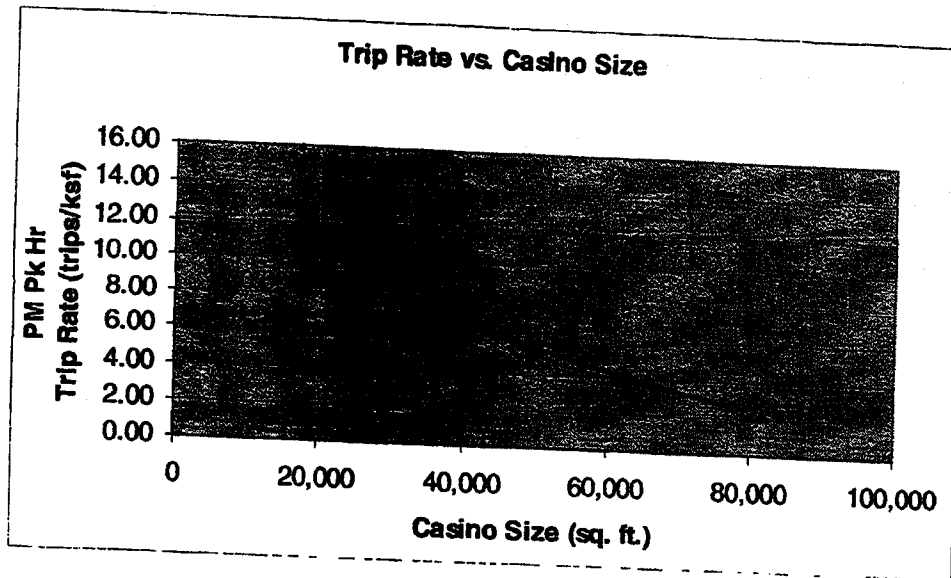
Weekday PM Peak Hour Trip Generation Rates

The weekday PM peak hour rate was established prior to rates for weekday AM peak hour and Saturday peak hour conditions due to the large amount of information available, and to create a baseline from which peak hour rates for the remaining scenarios could be compared and established.

A review of the weekday PM peak hour trip rates at the five northern California casinos shows that the rates range from 2.67 trips/ksf at site A (which has the largest square footage of the five locations) to 13.83 trips/ksf at site E (which has the smallest square footage of the five locations). The graph within Graph 1 below illustrates how smaller casinos experience high trip rates, whereas larger casinos experience smaller trip rates, and that trip rates begin to level off as the casino becomes increasingly large. This decrease, which is best exemplified by the fitted curve, reveals that trip rates decrease dramatically as the square footage of the smaller casinos increase, and continue to decrease (although at a steadily decreasing rate) as the casino size increases to large casinos such as the proposed project.

As Table 13 shows, the weighted average of the PM peak hour trip rates established for the five surveyed casinos was calculated as 5.95 trips/ksf. A weighted average was used rather than a straight average (which would have resulted in 7.92 trips/ksf) to give more weight to the larger casinos.

**Graph 1
Casino Trip Generation Rates vs. Casino Size Graph**



Using the above graph, an argument could be made that the trip generation for the proposed project would only be approximately 2-4 trips/ksf. As the graph shows, the fitted curve line begins to level off in the 2-4 trips/ksf range for casinos approaching 100,000 sq. ft. Since the proposed casino would be 282,600 sq. ft. in size, and thus well to the right of the point in the line where the rate levels off, it is not unreasonable to expect that only 3-4 trips/ksf would be generated by the project during the PM peak hour during a peak month.

Trip rates derived from the market study projected 3.47 trips/ksf for an average PM peak hour, and 4.95 for a peak month PM peak hour. These rates lie between the lower potential trip generation rates of 3-4 trips/ksf from the fitted curve, and the higher weighted average of 5.95 trips/ksf.

For this study, the PM peak hour rate of 4.95 trips/ksf from the market study was used for the weekday PM peak hour trip generation rate for peak month conditions. Based on the large size of the proposed casino, the same relative trip rate per 1,000 square feet is assumed to generally be the same for casinos over 200,000 sq. ft. It is believed that 4.95 trips/ksf is a reasonable rate for the proposed project, since it lies half way between the higher weighted average of 5.95, and the lower potential rates of 3-4 trips/ksf which the fitted curve in the above graph indicates could reasonably represent the rate for a large casino such as the proposed project.

Weekday AM Peak Hour Trip Generation Rates

Neither the marketing study, or any other trip generation information reviewed, provided information regarding AM peak hour trip generation for casinos.

Very few trips are generated by a casino during AM peak periods when compared to the PM peak hour and weekends, therefore a survey of a single existing casino during the AM peak hour was considered sufficient for purposes of this study. The AM peak hour trip generation rate for the proposed casino was established based on the assumption that casinos would likely experience similar proportional amounts of traffic during the AM peak hour as would be generated during the PM peak hour. For example, since traffic to and from casino B during the AM peak hour was found to be 60% of the trips the casino experienced during the PM peak hour, it is expected that other casinos (including the proposed project) would similarly experience the same proportional differences. Therefore, 60% of the trip rate assumed for weekday PM peak hour conditions is assumed for the weekday AM peak hour rate, resulting in a weekday AM peak hour trip generation rate of 2.95 trips/ksf for peak month conditions.

Saturday Peak Hour Trip Generation Rates

The weekends, and particularly Saturday, are typically the busiest time periods for a casino. Two northern California casinos were surveyed during the late afternoon/early evening hours of a Saturday to establish Saturday peak hour trip generation rates.

As Table 13 shows, the weighted average of the two Saturday peak hour trip rates established for the two surveyed casinos was calculated as 6.73 trips/ksf.

This rate of 6.73 trips/ksf lies between the average and maximum trip rates derived from the market study, lying well above the projected 5.45 trips/ksf during an average Saturday peak hour, and just below the 6.90 trips/ksf for the Saturday peak hour during the peak month.

For this study, the more conservative rate of 6.90 trips/ksf derived from the marketing study for Saturday peak hour/peak month conditions was used. Although there would likely only be a dozen or so days per year which would experience Saturday peak hour/peak month conditions, this rate was used to provide a worst case scenario for the project. In addition, it is important to note that peak hour spikes are not as pronounced on a Saturday as they are through the week. Instead high traffic volumes are experienced for long periods of time on a Saturday. Although there would only be one hour on any given Saturday which would constitute the "peak hour" for that day, observations show that the peak hour is only the absolute highest of a number of peak hours during which traffic volumes to and from the casino are high. Therefore, care must be taken with regards to the number of hours during which Saturday peak hour conditions are expected to occur. In reality, Saturday peak hour conditions can be assumed to generally exist for perhaps 6 or more hours. Typically, it is reasonable and justifiable to mitigate project impacts for the 25th-30th worst hour of a year. This is because there will always be special events during which excessively high traffic volumes can occur which can not be fully mitigated to acceptable levels of service. Additionally, it is prohibitively costly to mitigate for the absolute worst hour or day of the year. Care must be taken to differentiate between the 25th-30th worst hour vs. the 25th-30th worst day. Since volumes very near those generated during the peak hour may occur for up to 6 or so hours of a Saturday during the peak month, the worst 25-30 peak hours might actually occur during a total of only a half dozen or so days.

Based on these considerations, a trip rate of 6.90 trips/ksf is assumed for Saturday peak hour conditions during the peak month.

Hotel Trip Generation Rates

The proposed casino project includes a 250 room hotel. The existence of the hotel will not necessarily result in a significant increase in trip generation from that which the casino would generate if a hotel did not exist. In fact, a review of trip generation at the 50,000 square foot northern California casino designated as Site B, which includes a hotel, did not show a relative increase in trip generation than was calculated at the other four surveyed casinos (none of which included hotels). In fact, a review of the graph on the preceding page shows that the rate is less than the rates which would lie along the theoretical trip generation rate fitted curve.

This characteristic is due to the fact that the existence of the hotel will result in a significant level of internal trips. The marketing study confirmed that nearly all of the hotel guests are expected to also visit the casino, hence they are accounted for within the trip generation characteristics of the casino itself. Adding trip generation for them based on the hotel would result in a double counting of trips. Therefore, trips associated with casino visitors whom also are staying in the hotel should be deducted from any hotel trip generation calculations. Additionally, trip generation for casinos without hotels would include a number of trips to account for visitors arriving from or departing for their hotels, thus trip generation for the proposed casino could theoretically be reduced to account for this reduction in trips. This reduction in trips could in theory result in up to a 100% internal capture for the hotel.

These assumptions are validated by surveys of Mississippi Coast casinos included within the traffic study titled *Mississippi Gulf Coast Transportation Management Plan for Waterfront Development*. Of the eight casinos surveyed within this report, half had casinos and half did not. **Table 14** summarizes the trip generation rates for the eight casinos, as well as the weighted average for all of the casinos, those casinos with hotels, and those without hotels.

As the below table shows, the weighted Saturday peak hour average rate for hotels with casinos is actually lower than the rates for hotels without casinos (as well as the rate for all casinos either with or without hotels).

Although it is not unreasonable to conclude that the existence of the hotel would not add to the trip generation which would be expected if the casino stood alone, to be conservative, this study assumes that the hotel would generate 25% of the trips which would be generated by the hotel if it stood alone. Trip generation rates, and directional splits for inbound/outbound traffic for the hotel, were obtained from the ITE Trip Generation Manual, 6th Edition (Land Use Code 310 = Hotel).

Table 14
Mississippi Coasts Casino Trip Generation Rates

Casino	Casino Gaming Area Size (ksf)	Saturday Peak Hour Rate⁽¹⁾ (trips/ksf)
<i>w/ Hotels</i>		
Grand (Gulfport)	65,000	10.11
Grand	60,000	10.75
Casino One	19,350	12.56
Isle of Capri	35,000	13.34
<i>w/out Hotels</i>		
Copa Casino	30,000	10.77
Gold Coast	20,000	12.55
Biloxi Belle	18,000	14.17
Casino Magic	57,000	12.16
Weighted Average (all)		11.61
Weighted Average (w/ HOTEL)		11.22
Weighted Average (w/out HOTEL)		12.18

Notes:

(1) = Rate is trips/1,000 square feet of casino gaming floor area only, and not square footage of entire casino as rates within remainder of study reference.

Source: *Mississippi Gulf Coast Transportation Management Plan for Waterfront Development*, Gulf Regional Planning Commission, Gulfport, MS, June, 1993.

Casino Project Trip Generation

Using the rates and inbound/outbound directional splits as shown in Table 13 and described above for a casino, the number of vehicular trips to and from the proposed casino project were calculated for all three peak hour scenarios. Trips associated with the hotel portion of the casino project were also calculated. Table 15a provides a summary of trips which are projected to be generated from the casino project as originally proposed.

Trip generation for the casino for the 24 hour weekday and Saturday periods reflects the trip generation from the marketing study during the peak month for the project.

As the below table shows, it is projected that the proposed hotel/casino would generate a total of 9,918 trips during a typical weekday of the peak month, 739 of which would occur during the AM peak hour, and 1,219 of which would occur during the PM peak hour. On a Saturday during the peak month of the project, it is projected that the proposed project would generate 14,600 trips, 1,691 of which would occur during the peak hour. Trip generation for the proposed casino-

hotel are not specific to any year, and thus are assumed to be applicable for both existing and cumulative conditions.

Table 15a
Casino-Hotel Project Trip Generation

Time Period	Size	Rate	in / out Split	Trip Generation		
				In	Out	total
<u>Casino Trip Generation</u>						
Weekday	238.5 Ksf	39.43		---	---	9,404
Saturday	238.5 Ksf	59.07		---	---	14,088
Weekday AM Pk Hr	238.5 Ksf	2.95	70% / 30%	493	211	704
Weekday PM Pk Hr	238.5 Ksf	4.95	53% / 47%	626	555	1,181
Saturday Peak Hour	238.5 Ksf	6.90	46% / 54%	757	889	1,646
<u>Hotel Trip Generation⁽¹⁾</u>						
Weekday	250 Rooms	2.06		---	---	514
Saturday	250 Rooms	2.05		---	---	512
Weekday AM Pk Hr	250 Rooms	0.14	61% / 39%	21	14	35
Weekday PM Pk Hr	250 Rooms	0.15	53% / 47%	20	18	38
Saturday Peak Hour	250 Rooms	0.18	56% / 44%	25	20	45
<u>TOTAL TRIP GENERATION</u>						
Weekday				---	---	9,918
Saturday				---	---	14,600
Weekday AM Peak Hour				514	225	739
Weekday PM Peak Hour				646	573	1,219
Saturday Peak Hour				782	909	1,691

Notes:

ksf = 1,000 square feet

(1) = Trip rates based on ITE Trip Generation - Hotel (Land Use 310).
Rate reduced by 75% to account for internal capture to/from casino.

Casino Project Trip Generation Validation

Subsequent to the initial establishment of trip generation for this study, additional research was conducted to validate trip generation assumptions. Some parties have publicly stated that the proposed hotel and casino would generate over 17,000 trips per day instead of the of 9,918 weekday trips assumed within this analysis.

The trip generation established in the previous section was established using separate trip generation rates for casinos and hotels, not a combined Shingle Springs casino-hotel facility. Table 15b establishes trip generation rates based on the TOTAL number of trips that were established for the entire casino-hotel facility. Rates for a combined facility are necessary to help validate trip generation assumptions used within this analysis against research described below.

**Table 15b
Casino-Hotel Project Trip Generation**

	Trips Generated by Shingle Springs Casino			Trips Generated by Shingle Springs Hotel (25% of ITE Trip Gen)			Trips Generated by Shingle Springs Casino-Hotel		
	Size (GFA ksf)	Trip Rate	Trips	Size (rooms)	Trip Rate	Trips	Size (GFA ksf)	Adj Trip Rate	TOTAL TRIPS
Weekday Daily	238.5	39.43	9,404	250	2.06	514	238.5	41.58	9,918
Wkdy AM Pk Hr	238.5	2.95	704	250	0.14	35	238.5	3.10	739
Wkdy PM Pk Hr	238.5	4.95	1,181	250	0.15	38	238.5	5.11	1,219
Saturday Daily	238.5	59.07	14,088	250	2.05	512	238.5	61.22	14,600
Saturday Pk Hr	238.5	6.90	1,646	250	0.18	45	238.5	7.09	1,691

Notes: Trip Rates for casino and casino-hotel based on square footage of "Casino Gaming Floor Area" (GFA).

The following research and analysis helps to verify that the trip generation assumptions used within this report are reasonable and conservative, and helps illuminate how erroneous conclusions might be mistakenly drawn by others from similar research.

San Diego Casino Study

San Diego County Department of Public Works prepared a study of casino trip generation titled "Report on the Potential Impacts of Tribal Gaming on Northern and Eastern San Diego County." The traffic study portion of this report, which was included as an appendix, was titled "Preliminary Traffic Assessment of Indian Gaming Projects in the San Diego Region" dated October 17, 2000. Due to confusion regarding the specific criteria used in preparing this study, David Evans & Associates contacted the licensed traffic engineer serving as the project manager for this study. The project manager stated that the November 1, 2000 report which has been referenced within comments was only a preliminary report, and that the assumptions used regarding trips rates have since been revised. Additionally, it is important to note that the preliminary San Diego report did not specifically differentiate between the square footage of the ENTIRE casino facility vs. the square footage of ONLY the gaming floor area. This distinction is crucial when comparing trip generation rates. The project manager stated that since the submittal of the preliminary report, they have established that the 130 trips/1,000 sq. ft. of casino they used previously was with respect to the square footage of ONLY the gaming floor area, and not the square footage of the ENTIRE casino. Trip generation rates associated with the ENTIRE square footage of the casino would logically be significantly smaller than rates associated with the square footage of ONLY the gaming floor area due to the inclusion of square footage associated with ancillary uses such as restaurants, banking facilities, day care, offices, rest rooms, lobby areas, retail, etc. San Diego County is in the process of revising their earlier report with a more detailed report using more refined numbers, which will specify that the trip generation rates used are relative to the square footage of ONLY the gaming floor area. The project manager stated that they will be revising their trip generation rate down to 100 trips/1,000 square feet of gaming floor area.

The ENTIRE Shingle Springs casino, including all ancillary uses (but excluding the hotel) is proposed to be 238,500 sq. ft., whereas the gaming floor area is proposed to include only 82,800

sq. ft. The following shows how the total trip generation was initially calculated within the Shingle Springs traffic study using trip rates corresponding to the 238,500 sq. ft. of the ENTIRE Casino square footage:

Shingle Springs Trip Generation
(using ENTIRE Casino square footage as originally used within traffic study)

Trip Rate used within		39.43 trips per 1,000 sq. ft.
Shingle Springs Traffic Study =		of ENTIRE Casino
Size of ENTIRE Shingle Springs Casino =	x	238.5 ksf (thousand sq. ft.)
<hr style="border: 0.5px solid black;"/>		
Shingle Springs Trip Generation (used in Traffic Study) =		9,404 trips

If instead, the 82,800 sq. ft. of ONLY the gaming floor area of Shingle Springs was used, the trip rate (assuming the total trip generation were held constant) would be 113.57, as shown within the following calculation:

Shingle Springs Trip Generation
(using sq. ft. of ONLY Casino Gaming area as used within San Diego traffic study)

Shingle Springs Trip Generation (used in Traffic Study)		=	<u>9,404 trips</u>		=	113.57 trips per 1,000 sq. ft.
Trip Rate used within			82.8 ksf			of ONLY Casino
Revised San Diego Traffic Study						Gaming Floor Area

As can be seen, the trip generation rate which would correspond to the trip generation used for the project would be almost 14% higher than the trip rate which is being used within the revised San Diego report. If the rates which will be used within the Revised San Diego study were to be used for the Shingle Springs Traffic Study, the total number of trips which the casino would generate would be as follow:

Shingle Springs Trip Generation
(using trip rate used within Revised San Diego Traffic Study)

Trip Rate used within		100 trips per 1,000 sq. ft.
Revised San Diego Traffic Study =		of ONLY Casino
Size of Shingle Springs		Gaming Floor Area
Casino Gaming Floor Area =	x	82.8 ksf (thousand sq. ft.)
<hr style="border: 0.5px solid black;"/>		
Alternative Shingle Springs Trip Generation =		8,280 trips

The use of this rate would result in a reduction of 1,184 daily trips (a 12.6% reduction).

The project manager also stated that the revised San Diego study will also assume an internal capture for a mixed hotel/casino, although a slightly more conservative rate of 3.0 trips/room for an average weekday will be added to casino hotel trip generation. Based on the Institute of Transportation Engineers', "Trip Generation, 6th Edition," a hotel generates 8.23 trips per room on an average weekday. The Shingle Springs report assumed that 25% of the trips which the hotel would generate if standing alone would be added onto the trips generated by the hotel, which resulted in an average daily rate of 2.06 trips per room (8.23 x 0.25 = 2.06). The revised

San Diego Study rate of 3.0 trips/room assumes that 36.5% of the trips which the hotel would generate if standing alone would be added onto the trips generated by the casino (3.0/8.23 = 36.5%). If the Shingle Springs report used a similar assumption of 3 trips/room, it would result in an addition of only 236 trips during an average day, 16 additional trips during the AM peak hour, and 17 additional trips during the PM peak hour. With respect to assumptions used with the revised San Diego traffic study, these differences would easily have been absorbed within the extra 1,184 daily trips which would have derived were the 100 trips/ksf of gaming floor space per the revised San Diego study. Ignoring that, however, these additional trips would have resulted in only negligible changes in the calculated levels of service within the report, and would have created no additional impacts or changes in any of the conclusions of the traffic study.

Mystic Lake Casino

David Evans and Associates located trip generation calculation research for Mystic Lake Casino-Hotel, a large stand-alone Indian gaming casino facility in southwestern Minnesota. This research was included within the "St. Croix Meadows Racing Park Proposed Casino Traffic Impact Study; Hudson, Wisconsin" (also called the Hudson Casino) prepared by BRW within the past 2 years. The Mystic Lake Casino-Hotel is also a very large complex, and very similar in nature to the proposed Shingle Springs casino, as shown by the comparison in Table 16a.

**Table 16a
Shingle Springs vs. Mystic Lake Size Comparison**

Variable	Mystic Lake	Shingle Springs
Size of Casino (All – without Hotel)	447.6 ksf	238.5
Size of Casino Gaming Floor Area	101.5 ksf	82.8
Number of Hotel Rooms	416 rooms	250 rooms
Number of Gaming Positions	3,916	3,000

Source: "St. Croix Meadows Racing Park Proposed Casino Traffic Impact Study; Hudson, Wisconsin;" BRW.

Trip rates for the Mystic Lake Casino-Hotel were established based on surveys of existing weekday PM peak hour, and Saturday peak hour trips which are currently visiting the facility. Because this facility is large, it is assumed that trip rates experienced at the facility would provide a reasonable check of peak hour trip rates used for the Shingle Springs analysis. A comparison of weekday PM peak hour and Saturday peak hour rates is provided in Table 16b.

**Table 16b
Shingle Springs vs. Mystic Lake Trip Generation Rate Comparison**

Casino-Hotel	Size of Entire Casino (without Hotel) (ksf)	Trip Generation Rate (ksf)	
		Weekday PM Pk Hr	Saturday Pk Hr
Mystic Lake	447.6	4.08	4.51
Shingle Springs	238.5	5.11	7.09
% Difference		+25%	+57%

Source: Mystic Lake data from "St. Croix Meadows Racing Park Proposed Casino Traffic Impact Study; Hudson Wisconsin;" BRW.

As can be seen, the trip rates assumed for the Shingle Springs Casino are 25% higher for weekday PM peak hour conditions, and 57% higher than Saturday peak hour conditions than actually occur at the Mystic Lake Casino-Hotel. Thus, from this perspective, the peak hour trip rates used for the Shingle Springs facility are considered to be conservative.

Trip rates for the Shingle Springs facility were established using 25% of ITE trip rates for stand-alone hotels. Since these result in trip generation 25%-57% less than trip rates established for the Mystic Lake Casino-Hotel, this validates that the assumption in which only 25% of hotel trips as established in the ITE Trip Generation Manual were used to establish Shingle Springs trip generation are reasonable and conservative.

ITE Article

An article published within the May, 1992 Institute of Transportation Engineers Journal titled "Trip Generation Rates for Las Vegas Area Hotel-Casinos" was referenced to see how trip generation rates for the proposed Shingle Springs Casino corresponded to the findings within the article. Trip characteristics for "all hotel-casinos" analyzed as part of the study, rather than "strip hotel-casinos," were used for comparisons since they included rural casinos in outlying areas, and off-strip casinos with 200-300 rooms, as well as strip casinos. Inclusion of these other casino types would provide results closer to those which would be expected for the Shingle Springs facility than would results due to strictly strip casinos. As discussed elsewhere, the use of trip generation rates for Las Vegas area casinos, particularly those lying along the strip, are not considered to be directly applicable to the trip rates for the Shingle Springs Casino, but are provided here to provide an additional check on the peak hour trip rates used. Within this study, trip generation rates for entire hotel-casino complexes were established using three separate variables (1) number of hotel rooms, (2) employees, (3) thousand square feet of casino gaming floor area. Using the fitted curve equations provided, and quantities as assumed for the Shingle Springs facility, the Shingle Springs facility would generate the trips shown in Table 17.

Table 17
Shingle Springs vs. Las Vegas Casino-Hotels Peak Hour Trip Rate Comparison

	250 Rooms	1,500 Employees	82.8 ksf Casino Gaming Floor Area	Average Trips
<u>AM Peak Hour</u>				
Shingle Springs Trips (Per Traffic Study)	739	739	739	739
Trips per ITE Article Fitted Curve Equation	328	503	851	561
Difference in Volumes (Traffic Study vs. Article)	-411	-236	+112	-178
% Difference (Article Rates vs. Traffic Study Rates)	-125%	-47%	+13%	-32%
Coefficient of Determination (R2)	0.568	0.927	0.748	---
<u>PM Peak Hour</u>				
Shingle Springs Trips (Per Traffic Study)	1,219	1,219	1,219	1,219
Trips per ITE Article Fitted Curve Equation	501	827	1,386	905
Difference in Volumes (Traffic Study vs. Article)	-718	-392	+167	-314
% Difference (Article Rates vs. Traffic Study Rates)	-143%	-47%	+12%	-35%
Coefficient of Determination (R2)	0.445	0.799	0.631	---

Source: "Trip Generation Rates for Las Vegas Area Hotel-Casinos" May, 1992 Institute of Transportation Engineers Journal.

As the table shows, use of the fitted curve equations for Shingle Springs quantities for two of the three variables result in peak hour volumes between 47% and 143% less than those which are assumed for the Shingle Springs study. The remaining variable (casino gaming floor area) results in slightly higher peak hour rates (12-13% higher). When average together, it can be seen that fitted curve equations within the article result in an average number of trips which are 32-35% lower than those which were actually used. Thus, from this perspective, the peak hour trip rates used for the Shingle Springs facility are considered to be conservative.

Because hotels are included within these studies, these results also validates that the assumption in which only 25% of hotel trips as established in the ITE Trip Generation Manual were used to establish Shingle Springs trip generation are reasonable and conservative.

ITE Casino Trip Generation Report

A write up regarding trip generation rates for the Shingle Springs study was submitted to a member of the technical advisory committee which helped to put together a report prepared for the Institute of Transportation Engineers titled "*Casino Trip Generation.*" This report is currently undergoing final review and is expected to be published in the near future. This individual reviewed the trip generation assumptions used within the Shingle Springs study, which included trip rates for total square footage (238,500 sq. ft.), gaming floor area square footage (82,800 sq. ft.), trips per gaming position (3,000), and trips per employee (1,500). He verified that the rates and methodology were fully consistent with this report, and that in his opinion, the rates used, and trips generation volumes calculated, were very conservative for a facility this size.

He also verified that an assumption of approximately 2 trip/room (25% of the trip generation vs. a stand alone hotel) is reasonable.

Casino Project Trip Generation Validation Summary

As the research and analysis above indicates, the trip generation assumptions used within this report are reasonable and conservative.

Additionally, the discussion above shows that great care must be taken when comparing assumptions used within this analysis against those drawn from other sources. As shown, it is important that data be used correctly. For example, it is important that trip generation based on rates established for the square footage of the gaming floor area be carefully distinguished from those established for the square footage of an entire casino. Similarly, it is important that allowances be made for the mixed nature of the project, and recognized that a combined casino-hotel will draw significantly less traffic than the combined traffic associated with a similar sized stand alone casino and stand alone hotel.

Water Supply Trucks

It is anticipated that an average of 25 trucks may deliver water to the casino-hotel per day, which when accounting for the inbound and outbound leg of each delivery, would result in a total of 50 trips per day on average. Although not specifically identified within the trip generation breakdown described elsewhere in this report, they could conceivably be included within the

total. This is especially true when it is remembered that the trip generation utilized within this study is considered to be extremely conservative, well more than the site will likely experience, and was established without regards to individual trip types. Trip generation breakdowns described elsewhere in this report were established by delineating the total number of trips into all of the various potential trip types, and were performed prior to the decision to include water delivery trucks. Nonetheless, the addition of water trucks would likely be negligible for the following reasons: (1) they would comprise only ½% of the 9,918 daily trips; (2) deliveries would be spread out over the entire 24 hour period each day, and to avoid peak hour congestion, it is likely that very few, if any, deliveries would be made during the AM or PM peak periods due to congestion.

However, impacts identified within this study were tested to determine if the addition of water delivery trucks could potentially cause additional impacts beyond those identified. At most, it is assumed that 10% of deliveries would occur during the AM, PM and/or Saturday peak hour, since peak hour traffic volumes typically represent approximately 10% of daily volumes along any traffic facility. It was found that the addition of 5 trips during any peak hour (10% of 50 trips), or 50 daily trips, would not create any additional impacts beyond those identified within this study, including intersections, ramps, US-50 freeway mainline, and local roads.

CASINO PROJECT TRIP DISTRIBUTION AND ASSIGNMENT

Different approaches were taken to distribute peak hour and daily trips. The initial phase of this study investigated peak hour trip distribution and considered only trips in the immediate vicinity of the project. To facilitate the more detailed phase of this study which considered impacts to local streets throughout El Dorado County, a more detailed trip distribution scenario was established. However, the local street analysis requiring a more detailed trip distribution scenario considered trip distribution on a daily basis rather than a peak hour basis.

Peak Hour Trip Distribution and Assignment

Peak Hour Trip Distribution

Peak hour trip distribution of casino project generated traffic is based on information in the Urban Systems Marketing Study, and the geographical location of population centers from which the casino is expected to draw both customers and employees. Based on this criteria, a significant percentage of the casino's traffic is expected to originate from the Sacramento/San Francisco Bay area.

Based on this criteria, peak hour trips to and from the proposed casino project (for analysis of impacts within the immediate vicinity of the project) were distributed as follows:

- 80% to/from the west (Sacramento/San Francisco Bay area)
- 20% to/from the east (Placerville/South Lake Tahoe area)

Casino project trips as assigned to the two proposed freeway interchange intersections for weekday AM and PM peak hours, and the Saturday peak hour, are depicted in **Figure 2**.

Figure 2

Peak Hour Passer-By Diversion

Not all of the traffic to and from the proposed casino project would be newly generated trips. A significant percentage of the through traffic on Highway 50 consists of vehicles travelling to and from Lake Tahoe, and a large percentage of these trips have a known propensity to gamble. Also, Shingle Springs will be an attractive stop for vehicles travelling a significant distance to and from locations such as the Bay Area, Stockton, etc. Thus, many of the people visiting the casino will be people who would have already been on the freeway en route to other existing casinos or recreational activities, particularly east of the project site including in large part the Tahoe area. Therefore, the trip generation calculated for the proposed casino must be adjusted before assigning the trips to the freeway. In other words, although 100% of the trips generated by the casino/hotel would be assigned as new trips to the ramps, intersections, and roadways of the new interchange, only a percentage of these trips should be assigned to US-50 as new trips since a significant percentage are assumed to already exist on the freeway. Without the new interchange and casino, these trips would have continued past the new interchange along US-50.

With the new interchange and casino, it is assumed some of these trips will be intercepted or diverted to the new casino.

For this traffic analysis, it was conservatively assumed that 40% of peak hour trips generated by the proposed casino project would be trips which are already assumed to exist within existing (and future projected baseline) US-50 traffic volumes during peak hours. Following the completion of the casino and interchange, this 40% is assumed to stop at the site rather than continuing past the site, which they would do in the absence of the development. This assumed passer-by capture rate was established in coordination with Caltrans traffic engineering personnel.

The assumed passer-by capture rate of 40% is based on and/or validated by the following:

(1). Mississippi Coast Casinos - License plate data included within the "*Mississippi Gulf Coast Transportation Management Plan for Waterfront Development*" traffic study suggests a 40% capture from the local area.

(2a). Urban Systems Marketing Study - The Urban Systems Marketing Study provided research regarding capture rates of I-15 traffic to facilities in Primm, Nevada. Primm, Nevada is located just inside the California/Nevada border, approximately 25 miles southwest of Las Vegas. Thus Primm is also located along a heavily used corridor in which a large percentage of trips have a known propensity to gamble. Studies show that 20% to 24% of "non-commuter, non-truck" traffic passing traffic chooses to exit I-15 and visit a facility located in Primm. In addition to multiple casino facilities, Primm offers hotels, dining and fuel, thus the capture rate is not due exclusively to gaming.

Using this information as a basis, the Urban Systems Marketing Study concluded that 8% of "through" vehicular traffic passing along US-50 would be captured. Again, through traffic excluded both commuter traffic and truck traffic. Based on research conducted by Urban Systems, it is assumed that 65% of traffic along US-50 in the vicinity of the project is commuter

oriented. Additionally, Caltrans truck volume data shows that 6% of the volumes along US-50 in the vicinity of the project are trucks. Based on 2000 Caltrans counts, the average annual daily traffic volumes along US-50 between Shingle Springs is 43,000 vehicles. Thus, as the following calculation shows, there are 12,470 through vehicular (non-truck) trips on an average weekday.

2000 AADT on US-50 between Shingle Springs & Greenstone	43,000
Commuter Traffic = 65%	27,950
Through Traffic = 35%	15,050
Truck Traffic = 6%	2,580
Through Vehicular (Non-Truck) Traffic	12,470
Traffic which Casino will Intercept = 8%	1,000
Trips due to traffic intercept (2 trip ends/vehicle)	2,000
Total Shingle Springs Trip Generation (Average Weekday)	9,918
% of Average Daily Trip Generation	20.2%

If it is assumed that the project captures 8% of this traffic, then 20.2% of the trips generated by the project on an average weekday are due to the "capture" of passing through traffic.

The diversions outlined within the Urban System Marketing Study do not necessarily include other types of potential trip diversions, including the diversions of commuters, business travelers, truckers, vacationers, etc. traveling between SF Bay/Sacramento and the Placerville/South Lake Tahoe area who might decide to stop for an hour or two of leisure before continuing on. The exact number of visitors falling into these categories is unknown, but are obvious components. As outlined in detail within the next section, these other potential trip diversions potentially include the following trip types:

- Interception of gamers who would have been traveling to South Lake Tahoe casinos, but instead will visit Shingle Springs. Note that most of these gamers would have been arriving from the west (i.e. Bay Area, Sacramento, and Stockton), however some of the gamers would have been diverted from portions of El Dorado County, including locations east of the project site.
- Diversion of gamers from the west (i.e. Bay Area, Sacramento, and Stockton) who would have gone to South Lake Tahoe casinos in the absence of Shingle Springs, is visiting Shingle Springs as a primary destination instead, but will still continue to Tahoe as a secondary destination
- Diversion and interception of a portion of the following unique trip types, which it is assumed would have already been on US-50 with or without the casino-hotel:
 - gamers who are primarily tourists visiting family and friends in the area
 - gamers who are primarily in the area on business or attending conventions
 - gamers who are primarily in the area vacationing
 - employees who would have been on US-50 to/from other jobs in the area
 - guests staying in casino's hotel who would have been participating in area recreation (and possibly staying in other area hotels)
 - other miscellaneous hotel related trips
 - buses
 - vehicles making deliveries

(2b). Urban Systems Marketing Study - An early draft of the Environmental Assessment for this study included the following assumptions regarding the potential interception and diversion of traffic to the casino. These assumptions were calculated based on information from the Urban Systems Marketing Study:

- A 20.9% interception of South Lake Tahoe trips (trips which would potentially travel to the proposed casino instead of going to South Lake Tahoe recreational activities)
- A 21.8% interception of Stateline casino trips (trips which would potentially travel to the proposed casino instead of going to Stateline casinos)
- A 15.0% diversion of South Lake Tahoe trips (trips which would still continue to/from Lake Tahoe after stopping temporarily at the new casino)

It is important to note that these percentages are not percentages of US-50 traffic volumes, but rather the percentage of trips generated by the proposed casino/hotel. Together, these potential diversions and intercepts add up to a 57.7% total potential reduction of casino project trip generation which would travel along US-50. Additionally, due to the actual interception of traffic which would have gone to Tahoe without the new casino, baseline volumes along US-50 east off the site could theoretically be reduced by a volume equal to 15% of the total project trip generation. However, neither existing or future baseline volumes are reduced within this analysis to account for this potential characteristic.

(3). Auburn Rancheria Environmental Assessment - The "*Environmental Assessment for the United Auburn Indian Community of the Auburn Rancheria*" (Environmental Science Associates, June, 2000) assumed that a minimum of 30% of the trips generated by the casino would be "pass-by" trips. Since this facility is located a few miles off from I-80, it is reasonable that the passer-by capture for a facility located immediately next to a major freeway such as Shingle Springs would attract a higher passer-by percentage, thus 40% seems reasonable in comparison.

(4). Sensitivity Analysis - A sensitivity analysis was performed to establish the passer-by capture threshold where additional potential impacts than those identified within this traffic study might be created. This sensitivity analysis shows that as long as the passer-by capture rate is no less than 29%, that no additional peak hour impacts than those identified within this traffic study would be created.

The establishment of a capture rate for any project is not an exact science, and is very difficult to establish with certainty. This is especially true for a complex project such as Shingle Springs where a broad spectrum of trips types relating to gamers, hotel guests, employees, recreation related traffic, etc. will exist in unknown quantities. Although a reasonable estimate of the total trip generation can will be established based on comparisons with other facilities, the exact nature of various trip types is more troublesome. Additionally, the breakdown between new trips, trips which would have existed without the project, diverted passer-by trips, intercepted passer-by trips, etc. are also troublesome to breakdown. There are other factors which will influence passer-by capture.

It is important to note that trip generation characteristics will likely be affected during time periods when US-50 is experiencing peak congestion. Although US-50 in the vicinity of the project does not currently operate in a significantly congested manner, freeway congestion will continue to become increasingly problematic in the future until the freeway is widened to 6-lanes. During times when the freeway is highly congested, persons desiring to travel to or from the casino would potentially be discouraged from travelling during peak hour conditions when US-50 volumes are approaching or exceeding capacity. This would be especially true for traffic to and from the Sacramento/SF Bay area, which would encounter significant traffic problems along long stretches of the route. For these reasons, it is probable that during peak hours when US-50 is congested that less traffic would travel to/from the Sacramento/SF Bay area. Additionally, the facility would have an added attractiveness when the highway is congested, at which time the casino would provide an even higher degree of attraction as a means of waiting out congestion.

Similarly, since the facility is located in the vicinity of the Sierra snowline, and at a point where the highway is frequently shut down, or where chain control begins, the facility would have an added attractiveness during times when continued travel up into the mountains is questionable.

Trip generation as described above is based on peak hour characteristics at similar facilities, which also experienced congestion along access roadways. Thus, the trip generation characteristics described above are considered to be a fair representation of the trips for the proposed casino project, even when US-50 is congested. However, to account for the reduced number of longer trips during these congested peak hour periods, trips generated by the casino would be expected to be more locally oriented, or the result of trip diversions. Although it is recognized that the diversions might in reality be lower during uncongested peak hours (and newly generated trips might constitute a higher percentage of all generated trips), this aspect would be more than offset by the lower traffic volumes along an uncongested freeway. Thus, the same 40% reduction is assumed for all three peak hour scenarios.

Daily Trip Distribution and Assignment

To establish daily trip distribution to local roads, it was necessary to disaggregate the total number of project trips into various trip types to differentiate between newly generated trips and those which are assumed to already be on US-50 as passer-by trips. To do this, the total daily project traffic volumes for an average weekday during the peak month were used:

$$9,404 \text{ casino trips} + 514 \text{ hotel trips} = 9,918 \text{ TOTAL trips}$$

A variety of materials were used to separate these trips into trip types, particularly the November, 1999 Marketing Study performed by Urban Systems. Urban System's personnel were also contacted to establish additional information such as casino employee characteristics, typical vehicle occupancy rates, etc. which were helpful in converting information within the Marketing Study to vehicular trips. Substantial additional research and analysis was also performed to refine the assumptions, and distribute the traffic.

Trips generated by Single Springs were broken down into categories and assigned to highways and local roadways within El Dorado County as shown in Table 18.

Table 18
Shingle Springs Average Daily Trip Generation

Trip Type	Avg Wkdy Trips
Employees (Assume Hotel = 10%)	1,375
Gamers - Local Residents (El Dorado Co. & vicinity)	965
Gamers - Regional Residents (SF, Sacramento, etc.)	
(West along US-50)	4,565
(North along SR-49)	199
(South along SR-49)	135
Gamers - Tourists	
Visiting Friends & Family	48
Business	169
Conventioneers	14
Vacationers	37
Recreation by Hotel Guests	304
Other Hotel (Non Employee/Recreation)	72
Buses	20
Deliveries	20
Traffic Diversion of "Passing Through" Traffic	1,995
Total	9,918

The specific types of trips which are assumed to be generated by the proposed casino/hotel, and methodologies used to distribute the trips, are described below:

Employee Trips to both the casino and hotel were established assuming a total of 1,500 casino employees (including full time administrative staff) and an additional 200 hotel employees. It was assumed that on an average weekday that 550 casino employees, 200 administrative employees, and 90 hotel employees would travel to and from the site. It was assumed that each employee vehicle would include 1.2 people, and that each vehicle would make a total of 2.2 trips per day. Employee trips were distributed using the County's travel forecast model, which distributed traffic volumes to US-50 interchanges as shown in **Table 19**.

Local & Regional Gamer Trips are defined as trips where a gamer's trip originated generally from their residence. These trips were established through use of market forecasts as described within the Marketing Study performed by Urban Systems. This study carefully analyzed the potential gaming market and the likely locations where gamers would be drawn from. Factors such as distance from the proposed casino, the propensity for gaming by residents in certain locations, and the influence of competing casinos such as Jackson Rancheria and the proposed Auburn Rancheria were considered. The exact locations of gamers was delineated as much as was necessary to determine the exact roadways in which gamers would arrive at the project site. For example, all of the gamers from the San Francisco, Sacramento, and Stockton areas would arrive via US-50, whereas gamers in the Rocklin, Auburn area have a choice between SR-49 and US-50 (via Auburn-Folsom Rd, Sunrise Blvd, Hazel Ave, etc.). Local gamer trips were

separated from regional gamer trips and distributed onto the local roadway network using the County's travel forecast model, in the same manner in which employees were distributed. It is assumed that gamers and employees living in the immediate vicinity of the project will be distributed in a similar manner since each is a function of the location of local residential development.

Table 19
Local Trip Distribution (Employees & Local Gamers)

US-50 Interchange/Intersection	Cumulative %	% to/from South	Total % to/from Interchange	% to/from North
	<i>9.1% (Continuing West)</i>			
1. El Dorado Hills Blvd / Latrobe Rd	18.9%	← 2.0%	9.8%	7.8% →
2. Bass Lake Rd	20.3%	← 0.6%	1.4%	0.8% →
3. Cambridge Rd	23.3%	← 0.1%	3.0%	2.9% →
4. Cameron Pk Dr	35.3%	← 2.0%	12.0%	10.0% →
5. Ponderosa Rd / S. Shingle Rd	49.6%	← 5.5%	14.3%	8.8% →
6. E. Shingle Springs Dr		← 3.5%	7.0%	
	<i>53.1% (to/from west)</i>			
6.5. NEW SHINGLE SPRINGS RANCHERIA INTERCHANGE				
	<i>46.9% (to/from east)</i>			
7. Greenstone Rd	34.1%	← 7.9%	12.8%	4.9% →
8. El Dorado Rd	33.2%	← 0.6%	0.9%	0.3% →
9. Missouri Flat Rd	20.0%	← 9.3%	13.2%	3.9% →
10. Placerville Dr / Forni Rd	14.0%	← 1.3%	6.0%	4.7% →
11. Main St	13.0%	← 0.2%	1.0%	0.8% →
12. SR 49	9.6%	← 2.1%	3.4%	1.3% →
13. Bedford Ave	7.4%	← 0.8%	2.2%	1.4% →
14. Mosquito Rd	5.4%	← 0.9%	2.0%	1.1% →
15. Schnell School Rd	4.1%	← 0.8%	1.3%	0.5% →
16. Point View Dr	3.3%	← 0.6%	0.8%	0.2% →
17. Carson Rd	2.6%	← 0.4%	0.7%	0.3% →
18. Carson Rd (@ Barkely)	1.9%		0.7%	0.7% →
19. Carson Rd (@ Pony Express Tr)	1.5%		0.4%	0.4% →

20. Pony Express Tr		0.5%	0.5% -->
	1.0%		
21. Sly Park Rd		<-- 0.3%	0.6%
	0.4% (Continuing East)		0.3% -->

Source: 1999 El Dorado County travel forecast model

Tourist Gamer Trips are defined as gamers who are visiting the northern California area. These can be separated into people visiting family and friends in Northern California, people visiting the area on business or attending conventions, and people who are vacationing in the area. For purposes of this study, it is assumed that most of the business people and conventioners will be arriving at the site from locations west of El Dorado County (i.e. Sacramento). It is also assumed that 80% of the tourist gamers visiting family and friends would also be arriving from the west, with the remaining 20% assumed to be visiting family and friends within El Dorado County (and thus also distributed to local roadways using the County's travel forecast model). Finally, people who are vacationing in the area were assumed to arrive from local points of recreation (described below). Most of the trips associated with Tourist Gamers were assumed to be arriving at the site from locations west of El Dorado County, although a portion of the trips were assigned to local roadways within El Dorado County.

Recreation Trips by Hotel Guests are based on the assumption that some of those people who are staying at the hotel (whether they are gamers or not) will visit one or more of the many recreational spots in the area. Assuming that each hotel guest party (i.e. 1 occupied room) stays an average of 1.75 days, and assuming that each party makes 1.25 recreation oriented trips per stay, an average of 304 trips per day (inbound plus outbound) would be generated by the project. Extensive research was performed regarding the various recreational choices in the area, and the number of visitors to each. Using this information, as well as input from the El Dorado County Chamber of Commerce, recreational trips by guests were distributed to roadways to and from the destinations shown in Table 20.

Table 20
Hotel Recreation Trip Distribution

Recreation Choice	Percentage
Downtown Placerville (Antiques, Museums, Restaurants, Shopping, etc.)	25%
Apple Hill (+ North County Wineries)	13%
Coloma area North County Wineries	1%
South County Wineries	8%
Marshall Gold State Park	12%
Gold Bug Park	3%
White Water Rafting (Coloma Put In)	3%
White Water Rafting (Chili Bar Put In)	2%
Golf (El Dorado Hills Golf Course)	2%
Golf (Apple Mtn Golf Course)	3%
Other Misc. Attractions (in western & central El Dorado Co.)	5%

South Lake Tahoe & other attractions in East County (east along US-50)	8%
Sacramento & Folsom (west along US-50)	10%
Auburn (and other pts north along SR-49)	3%
Yosemite (& other pts south along SR-49)	2%
TOTAL	100%

Other Hotel Trips are trips associated with that portion of trips related specifically to the hotel, which are over and above project trips calculated specifically in connection with the casino. Other hotel trips are the remainder of hotel trips after deductions for hotel employee trips (assumed as 10% of the projected 1,500 employees), and recreation trips as described above. As discussed elsewhere, 75% of trips associated with the hotel are assumed to be trips which would have been generated by the casino with or without the hotel. Most of these trips were assumed to be arriving at the site from locations west of El Dorado County.

Bus Trips are assumed to number, on average, 20 per day based on information included within the Marketing Study performed by Urban Systems. It is assumed that these trips would be arriving at the site from locations west of El Dorado County.

Delivery Trips are assumed to number, on average, 20 per day based on information provided by the shipping and receiving department of a similar northern California casino. It is assumed that these trips would be arriving at the site from Placerville.

Traffic Intercept Trips are the most difficult component of the traffic to establish. Although the previous traffic study assumed a passer-by capture rate of 40% along US-50, a more refined calculation was established which took into account the assumed passer-by capture for each individual trip type generated by the project. The following details this breakdown.

As described within the introduction to this section, the Urban Systems Marketing Study concluded that 8% of "through" vehicular traffic passing along US-50 would be captured (excluding both commuter traffic and truck traffic), which converts to 20.2% of trips generated by the project on an average weekday being due to the "capture" of passing through traffic. These trips do not fall into any other category, and thus are assumed to be simply "passer-by capture of through vehicular volumes" trips.

It is important to note, however, that other trips generated by the project will also exist on US-50, and thus should not be added to trip generation volumes on US-50. Table 21 breaks down the various types of trips which are assumed would already exist along US-50, and which would be either diverted or intercepted by Shingle Springs. These trip type percentages are based in large part on information provided by Urban Systems both within the Marketing Study and verbally. However, additional assumptions are based on assumptions considered to be reasonable for the project.

Table 21
Passer-By Capture % by Trip Type

Passer-By Capture (Through Vehicular) includes gamers with South Lake Tahoe as primary destination	20.1%
"Intercepted Trips" associated with <u>Bay Area, Sacramento, and Stockton gamers</u> who would have been traveling to South Lake Tahoe casinos, but instead will visit Shingle Springs. (12% of trips as defined)	5.5%
"Diverted Trips" associated with <u>Bay Area, Sacramento, and Stockton gamers</u> who would have gone to South Lake Tahoe casinos in the absence of Shingle Springs, is visiting Shingle Springs as a primary destination instead, but will still continue to Tahoe as a secondary destination (4% of trips as defined)	1.8%
"Intercepted Trips" associated with <u>El Dorado County area gamers</u> who would have gone to South Lake Tahoe casinos in the absence of Shingle Springs, but instead will visit Shingle Springs. (63% of trips as defined) (Note: Capture rate varies along US-50 within El Dorado County)	6.1%
"Diverted Trips" associated with <u>Employees</u> who are assumed would have been on US-50 to/from other jobs in the area (25% of trips as defined)	3.5%
"Diverted & Intercepted Trips" associated with <u>Gamers who are Tourists</u> visiting family and friends (20%), in area on <u>Business</u> (10%), in area attending <u>Conventions</u> (10%), in area <u>Vacationing</u> (25%)	0.4%
Misc. "Diverted & Intercepted Trips" associated with <u>Hotel Guests</u> participating in area <u>Recreation</u> (25%), other <u>misc. Hotel</u> related trips (50%), <u>Buses</u> (50%), and <u>Deliveries</u> (100%)	1.4%
TOTAL PASSER-BY CAPTURE	38.8%

As the table shows, it is assumed that almost 39% of the trips which the project would generate on an average weekday would already be present on US-50. This is slightly less than the 40% passer-by capture assumed for peak hour conditions. It is expected that the passer-by capture during peak hours is higher than daily conditions due to increased traffic volumes and congestion during the peak hours, at which time the casino would provide an even higher degree of attraction as a means of waiting out congestion.

It is important to note that the total number of project trips was established through careful research of trip generation characteristics at other casino facilities, some of which includes hotels. Trip generation rates were established by conducting counts of traffic entering and exiting driveways at other casinos, which would thus include every type of trip possible including each of the various types of project trips described above such as the employee trips, recreation oriented trips, buses, deliveries, capture of passerby volumes, and obviously gamer trips. It is important to note that in reality the exact number of trips which would correspond to any specific trip type can never be known with certainty, and in fact many trips would be spread out over the various trip types. It would be nearly impossible for a study of this magnitude, with the

complexity and variety of trips generated, to be broken down perfectly into each exact trip type. However, the assumptions, trip allocations between trip types, and distribution and assignments to highways and local roadways represents the analyst's best faith effort to reasonably identify all trip types, and simulate traffic conditions with the project. Although close scrutiny may result in reasonable questions regarding the nature of each trip type, overlap between trip types, etc., it is believed the analysis reasonably represents the manner in which trips would be generated and distributed "as a whole." The breakdown into trip types simply represents the best analytical approach to reasonably distribute trips.

PEAK HOUR PROJECT VOLUMES

Existing Plus Project – Peak Hour Volumes

Peak hour turning movements at the new ramp intersections which would be expected without the proposed casino/hotel are depicted in Figure 3 for Existing Conditions.

Peak hour turning movements at the new ramp intersections following the completion and opening of the proposed casino/hotel are depicted in Figure 4 for Existing Conditions.

Table 22 provides a breakdown of the volumes along the freeway ramps and freeway mainline. This breakdown includes existing (no project) volumes, passer-by volumes, rerouted baseline Rancheria volumes, and newly generated casino traffic volumes.

**Table 22
Freeway & Freeway Ramp Volumes by Type
Existing Plus Project Conditions**

US-50 WB On-Ramp		AM Pk Hr	PM Pk Hr	SAT Pk Hr	US-50 WB Off-Ramp		AM Pk Hr	PM Pk Hr	SAT Pk Hr
PLUS PROJECT	Rancheria Only	5	3	3	PLUS PROJECT	Rancheria Only	4	10	7
	New Casino	108	275	436		New Casino	62	78	94
	Casino Passer-By	58	92	171		Casino Passer-By	131	103	147
	Total Volume	171	370	610		Total Volume	197	191	248
US-50 WB Mainline (West of New Interchange)		AM Pk Hr	PM Pk Hr	SAT Pk Hr	US-50 WB Mainline (East of New Interchange)		AM Pk Hr	PM Pk Hr	SAT Pk Hr
No Project Freeway Vol		2,206	1,589	1,691	No Project Freeway Vol**		2,206	1,589	1,691
- Non-Casino Off-Ramp Vol*		135	113	154	+ New Casino Vol		62	78	94
Adj No Project Fwy Vol		2,071	1,476	1,537	Plus Project Freeway Vol		2,268	1,667	1,785
+ On-Ramp Volume		171	370	610					
Plus Project Freeway Vol		2,242	1,846	2,147					

* "Non-Casino Off-Ramp Volume" subtracted from "No Project Freeway Volume" to account for cumulative volumes (Rancheria volumes and "Passer-by" Volumes) diverted to upstream off-ramp.

** "No Project Freeway Volume" includes existing Rancheria Traffic

US-50 EB Off-Ramp		AM Pk Hr	PM Pk Hr	SAT Pk Hr
PLUS PROJECT	Rancheria Only	3	5	4
	New Casino	246	310	375
	Casino Passer-By	75	155	166
	Total Volume	324	470	545

US-50 EB On-Ramp		AM Pk Hr	PM Pk Hr	SAT Pk Hr
PLUS PROJECT	Rancheria Only	10	4	3
	New Casino	27	69	109
	Casino Passer-By	32	137	193
	Total Volume	69	210	305

US-50 EB Mainline (West of New Interchange)		AM Pk Hr	PM Pk Hr	SAT Pk Hr
No Project Freeway Vol**		1,229	2,407	1,872
+ New Casino Vol		246	310	375
Plus Project Freeway Vol		1,475	2,717	2,247

** "No Project Freeway Volume" includes existing Rancheria Traffic

US-50 EB Mainline (East of New Interchange)		AM Pk Hr	PM Pk Hr	SAT Pk Hr
No Project Freeway Vol		1,229	2,407	1,872
- Non-Casino Off-Ramp Vol*		78	160	170
Adj No Project Fwy Vol		1,151	2,247	1,702
+ On-Ramp Volume		69	210	305
Plus Project Freeway Vol		1,220	2,457	2,007

* "Non-Casino Off-Ramp Volume" subtracted from No Project Freeway Volume" to account for cumulative volumes (Rancheria volumes and "Passer-by" Volumes) diverted to upstream off-ramp.

Figure 3

Figure 4

Cumulative Plus Project - Peak Hour Volumes

Peak hour turning movements at the new ramp intersections following the completion and opening of the proposed casino/hotel are depicted in Figure 5 for Cumulative Conditions.

Table 23 provides a breakdown of the volumes along the freeway ramps and freeway mainline. This breakdown includes cumulative (no project) volumes, passer-by volumes, rerouted baseline Rancheria volumes, and newly generated casino traffic volumes.

Table 23
Freeway & Freeway Ramp Volumes by Type Cumulative Plus Project Conditions

US-50 WB On-Ramp		AM Pk Hr	PM Pk Hr	SAT Pk Hr
PLUS PROJECT	Rancheria Only	7	4	4
	New Casino	108	275	436
	Casino Passer-By	57	92	175
	Total Volume	172	371	615

US-50 WB Off-Ramp		AM Pk Hr	PM Pk Hr	SAT Pk Hr
PLUS PROJECT	Rancheria Only	5	12	8
	New Casino	62	78	94
	Casino Passer-By	131	103	150
	Total Volume	198	193	252

US-50 WB Mainline (West of New Interchange)	AM Pk Hr	PM Pk Hr	SAT Pk Hr
No Project Freeway Vol	3,086	2,316	2,465
- Non-Casino Off-Ramp Vol*	136	115	158
Adj No Project Fwy Vol	2,950	2,201	2,307
+ On-Ramp Volume	172	371	615
Plus Project Freeway Vol	3,122	2,572	2,922

US-50 WB Mainline (East of New Interchange)	AM Pk Hr	PM Pk Hr	SAT Pk Hr
No Project Freeway Vol**	3,086	2,316	2,465
+ New Casino Vol	62	78	94
Plus Project Freeway Vol	3,148	2,394	2,559

** "No Project Freeway Volume" includes existing Rancheria Traffic

* "Non-Casino Off-Ramp Volume" subtracted from "No Project Freeway Volume" to account for cumulative volumes (Rancheria volumes and "Passer-by" Volumes) diverted to upstream off-ramp.

US-50 EB Off-Ramp		AM Pk Hr	PM Pk Hr	SAT Pk Hr
PLUS PROJECT	Rancheria Only	4	7	6
	New Casino	246	310	375
	Casino Passer-By	85	155	165
	Total Volume	335	472	546

US-50 EB On-Ramp		AM Pk Hr	PM Pk Hr	SAT Pk Hr
PLUS PROJECT	Rancheria Only	12	5	4
	New Casino	27	69	109
	Casino Passer-By	37	137	193
	Total Volume	76	211	306

US-50 EB Mainline (West of New Interchange)	AM Pk Hr	PM Pk Hr	SAT Pk Hr
No Project Freeway Vol**	2,150	3,441	2,681
+ New Casino Vol	246	310	375
Plus Project Freeway Vol	2,396	3,751	3,056

US-50 EB Mainline (East of New Interchange)	AM Pk Hr	PM Pk Hr	SAT Pk Hr
No Project Freeway Vol	2,150	3,441	2,681
- Non-Casino Off-Ramp Vol*	89	162	171
Adj No Project Fwy Vol	2,061	3,279	2,510
+ On-Ramp Volume	69	210	305
Plus Project Freeway Vol	2,137	3,490	2,816

** "No Project Freeway Volume" includes existing Rancheria Traffic

* "Non-Casino Off-Ramp Volume" subtracted from No Project Freeway Volume" to account for cumulative volumes (Rancheria volumes and "Passer-by" Volumes) diverted to upstream off-ramp.

Figure 5

DIRECT IMPACTS/MEASURES TO MINIMIZE

DIRECT IMPACT 1. EXISTING PLUS PROJECT- RAMP MERGE/DIVERGE OPERATIONS

Development of the project would create new merge/diverge lanes along US-50 at newly created interchange ramps, thereby increasing peak hour congestion along US-50 between East Shingle Springs Road and Greenstone Road.

AA No Project Alternative

There would be *no impact* associated with the No Project Alternative.

AB Flyover Alternative

As described in the methodology section, freeway ramp merge/diverge analysis were analyzed two different ways to provide for a comparative analysis, and to account for the various alternatives which are currently being proposed. The first method of analysis calculates ramp merge/diverge operations for rolling terrain, and does not take into account the specific grades and grade lengths; whereas the second method uses actual specific grades and grade lengths of the freeway leading up to the ramp, and along the ramp itself.

For detailed analysis which takes into account the specific grade and grade lengths of the freeway and ramp in establishing ramp merge/diverge operations, the grade length is calculated by taking the straight portion of the grade leading to (and perhaps continuing past) the ramp juncture point, and adding to this value $\frac{1}{4}$ of the vertical curves at both the beginning and end of the grade. By calculating the specific grade and grade length along the freeway in this manner, minor differences in the location of the ramp juncture points for each alternative do not change the length of the grade.

Although level of service analysis as included within HCS (Highway Capacity Software) also provides for the input of specific ramp grade and length, this data is negligible in terms of the analysis unless the volume along the ramp approaches the capacity of the ramp. Within the analyses included within this report, it was found that the specific grade and length of the ramps itself were negligible in terms of the level of service of the ramp merge/diverge.

As discussed in the methodology section, freeway ramp merge/diverge analysis also provides for the consideration of interference to traffic flow and capacity based on the location of adjacent freeway ramps, and the traffic volumes to and from them. For 4-lane freeway merge/diverge analysis, the length to the adjacent ramp and the volume on the adjacent ramp does not impact the result in anyway since the equation used to calculate level of service does not include this variable. Thus, although a distance to the adjacent ramp may be provided within 4-lane merge/diverge analysis included within the appendix, the value is irrelevant in terms of the level of service calculation.

A free flow speed of 48 kph (30 mph) was used for both project alternatives. This default value was used to approximate the design speed of 80 kph (50 mph) at inlet and exit noses of the ramps associated with Alternative AB, and 45 kph (28 mph) along the remainder of ramp.

Ramp merge/diverge analyses are performed only along the peak direction of travel for weekday peak hour conditions, which is along the westbound direction during the AM peak hour, and the eastbound direction during the PM peak hour. Ramp merge/diverge analyses are performed along both directions of travel for the Saturday peak hour. Analyses were not performed along the non-peak direction of travel following consultation and approval of Caltrans personnel.

Table 24a provides a summary of freeway merge/diverge operations for all three peak hour scenarios following the completion of the proposed interchange and proposed hotel/casino. The ramp merge/diverge level of service is applicable to all three scenarios, and uses "rolling terrain" to establish levels of service. The levels of service depicted are based on minimum acceleration/deceleration lengths of 250 ft. (76 meters).

Table 24a
Existing plus Project
Freeway Ramp Level of Service (Rolling Terrain)⁽¹⁾

	Level of Service		
	AM	PM	Saturday
	Peak Hour	Peak Hour	Peak Hour
EB off-ramp	-----	D	D
EB on-ramp	-----	D	C
WB off-ramp	C	-----	C
WB on-ramp	C	-----	C

Notes:

Length of deceleration and acceleration lanes assumed to be 250 ft. (76 meters)

(1) Level of service calculated using generalized "rolling" terrain instead of exact specific grade/length.

As the above table shows, all of the ramps are projected to operate acceptably at LOS D or better during all three peak hour scenarios for existing conditions with the new interchange and casino/hotel.

Table 24b provides a similar summary of freeway merge/diverge operations, but is based on the use of the specific grade and grade length along the freeway. As previously described, it has been determined that the levels of service reported are the same for both alternatives. Again, the levels of service depicted are based on minimum acceleration/deceleration lengths of 250 ft. (76 meters).

Table 24b
Existing plus Project
Freeway Ramp Level of Service (Specific Grade/Length)⁽¹⁾

	Specific Grade	Specific Length (ft)	Level of Service		
			AM Peak Hour	PM Peak Hour	Saturday Peak Hour
EB off-ramp	+4.38	2,525	-----	D	D
EB on-ramp	+2.28	1,150	-----	C	C
WB off-ramp	-2.28	1,150	C	-----	C
WB on-ramp	-4.38	2,525	C	-----	C

Notes:

Length of deceleration and acceleration lanes assumed to be 250 ft. (76 meters)

(1) Level of service calculated using exact specific grade/length instead of generalized "rolling" terrain.

As both of the above table shows, the freeway ramp merge/diverge areas for the new interchange are projected to operate acceptably at LOS D or better during all three peak hour scenarios for existing conditions with the new interchange and casino/hotel, regardless of whether they are analyzed using rolling terrain or specific grade/length. Therefore, this is considered a *less-than-significant impact*.

Figure 6 provides a summary of Existing plus Project ramp volumes and merge/diverge levels of service as calculated using specific grades and lengths.

Detailed analysis data using both "rolling" terrain and specific grades and lengths are provided in Appendix C.

AC Diamond Alternative (tight diamond)

Impacts associated with Alternative AC are identical to those identified above for Alternative AB.

Because of the methodology used to calculate the specific grade and grade length along the freeway, minor differences in the location of the ramp juncture points for each alternative do not change the length of the grade. Therefore, the specific grade and length of the ramps itself were negligible in terms of the level of service of the ramp merge/diverge, and the same specific grade and grade length along the freeway are applicable to analyses for both alternatives. To verify, a detailed analysis was performed for Alternative AB, and a trial and error input of the other possible input values for the Alternative AC was performed. This trial and error input included the ramp lengths, grades and volumes for Alternative AC, as well as extreme ramp lengths, grades and volumes outside the range of possibilities for Alternative AC. This trial and error analysis confirmed that the merge/diverge levels of service for the all alternatives and peak hour scenarios correspond to those calculated for Alternative AB. Therefore, for analyses which provide for the input of specific grades and grade lengths, the same values are used for both alternatives since the input for each result in exactly the same level of service results.

Figure 6

A free flow speed of 48 kph (30 mph) was used for both project alternatives. This default value was used to approximate the design speed of 80 kph (50 mph) at inlet and exit noses of the ramps associated with both alternatives, and 40 kph (25 mph) along the remainder of the ramp associated specifically with Alternative AC.

As the above tables show, the freeway ramp merge/diverge areas for the new interchange are projected to still operate acceptably at LOS D or better during all three peak hour scenarios for existing conditions (assuming either rolling terrain or specific grade/length) with the new interchange and casino/hotel. Therefore, this is considered a *less-than-significant impact*.

DIRECT IMPACT 2. EXISTING PLUS PROJECT- PEAK HOUR FREEWAY MAINLINE OPERATIONS

Development of the project would increase peak hour congestion along US-50 between East Shingle Springs Road and Greenstone Road.

AA No Project Alternative

There would be *no impact* associated with the No Project Alternative.

AB Flyover Alternative

Table 25 shows the freeway mainline operations for all three peak hour scenarios following the completion of the proposed interchange and proposed hotel/casino.

**Table 25
Existing Plus Project
Freeway Mainline Level of Service**

Freeway Segment	Freeway Level of Service ⁽¹⁾					
	Existing (no project)			Existing plus Project		
	AM Pk Hr	PM Pk Hr	Sat Pk Hr	AM Pk Hr	PM Pk Hr	Sat Pk Hr
EB US-50 (btwn E. Shingle Springs & Rancheria)	B	D	C	C	D	D
EB US-50 (btwn Rancheria & Greenstone)	B	D	C	B	D	D
WB US-50 (btwn Greenstone & Rancheria)	D	C	C	D	C	C
WB US-50 (btwn Rancheria & E. Shingle Springs)	D	C	C	D	C	D

Notes:

(1) Free Flow speed varies - "Ideal" Free Flow Speed of 65 mph adjusted to account for specific hwy geometry.

As the above table shows, the freeway is projected to operate acceptably at LOS D or better during all three peak hour scenarios for existing conditions with the new interchange and casino/hotel. Therefore, this is considered a *less-than-significant-impact*.

Figure 6 provides a summary of Existing plus Project freeway volumes and levels of service. Detailed analysis data is provided in Appendix D.

AC Diamond Alternative (tight diamond)

Impacts associated with Alternative AC are identical to those identified above for Alternative AB. As the above table shows, the freeway is projected to operate acceptably at LOS D or better during all three peak hour scenarios for existing conditions with the new interchange and casino/hotel. Therefore, this is considered a *less-than-significant impact*.

DIRECT IMPACT 3. EXISTING PLUS PROJECT- INTERCHANGE INTERSECTION OPERATIONS

Development of the project would create two new intersections within the newly created interchange, each of which would experience newly created delays.

AA No Project Alternative

There would be *no impact* associated with the no project alternative

AB Flyover Alternative

Since there are no intersections associated with the Flyover Alternative, there would be *no impact* associated with the Flyover Alternative.


AC Diamond Alternative (tight diamond)

Interchange operations are analyzed only for Alternative AC, since the modified trumpet design of Alternative AB contains no intersections to analyze. If constructed as a diamond interchange as proposed for Alternative AC, the interchange would include two intersections.

Unsignalized Intersection Analysis

Table 26a provides a summary of intersection operations following the completion of the interchange and casino/hotel for unsignalized intersections.

**Table 26a
Existing plus Project
Unsignalized Level of Service**

Intersection	AM Peak Hour		PM Peak Hour		Saturday Peak Hour	
	LOS	delay (sec)	LOS	delay (sec)	LOS	delay (sec)
New Shingle Springs Rancheria Road / WB ramps	A	(1.3)	A	(1.0)	A	(1.2)
New Shingle Springs Rancheria Road / EB ramps	B	(5.3)	C	(11.8)		

Notes:

Applicable only for Alternative AC since Alternative AB includes no intersections.
Delay for unsignalized intersections based on overall average vehicle delay.



As the above table shows, the eastbound ramp intersection would not operate at an acceptable level of service as an unsignalized intersection.

Signalized Intersection Analysis

Both of the intersections would meet Caltrans signal warrants for rural conditions, although the westbound ramps are warranted only for weekday PM peak hour and Saturday peak hour conditions, and the eastbound ramps are warranted only for Saturday peak hour conditions. It is thus recommended that at a minimum the eastbound ramp intersection be signalized as part of the interchange construction. Although not necessary to provide acceptable level of service operation, it is also recommended that the westbound ramp intersection also be signalized to provide efficient operation.

Table 26b below shows the level of service for the two intersections following signalization.

The signalized analysis was conducted using SYNCHRO software, which considered the effects of each of the two intersections upon each other.

Detailed analysis data for both unsignalized and signalized intersections, as well as a signal Warrant worksheet, are provided in Appendix E.

**Table 26b
Existing plus Project
Signalized Level of Service**

Intersection	AM Peak Hour		PM Peak Hour		Saturday Peak Hour	
	LOS	delay (sec)	LOS	delay (sec)	LOS	delay (sec)
New Shingle Springs Rancheria Road / WB ramps	A	(3.4)	A	(3.6)	B	(6.8)
New Shingle Springs Rancheria Road / EB ramps	B	(8.1)	B	(6.7)	B	(8.4)

Notes:

Applicable only for Alternative AC since Alternative AB includes no intersections.

Interchange Queuing Analysis

Using the SYNCHRO analysis described previously, the queuing along each of the intersection approaches were analyzed. Table 27 summarizes the queuing which would occur for Alternative AC along each of the intersection approaches for existing conditions following the completion of the interchange project for signalized intersections.

**Table 27
Existing plus Project
Intersection Queuing for Signalized Intersections**

Movement	Available Queue Capacity (ft)	AM Peak Hour Queue (ft)		PM Peak Hour Queue (ft)		Saturday Peak Hour Queue (ft)	
		50 th Percentile	95 th Percentile	50 th Percentile	95 th Percentile	50 th Percentile	95 th Percentile

New Project Access / Westbound US-50 Ramps

NB	163	0	0	0	0	0	0
SB Thru	760+	8	26	25	65	45	91
SB Right	760+	20	59	52	133	131	336
WB	1,275	55	92	54	93	69	129

New Project Access / Eastbound US-50 Ramps

SB	163	16	42	49	117	87	
EB	983	89	135	125	171	134	215

Notes:

[Redacted text]

As the above table shows, the queuing capacity for each of the approaches is expected to easily accommodate the projected queues for each of the peak hour scenarios for Alternative AC, with the exception of the southbound approach to the eastbound ramp intersection during Saturday peak hour conditions. Although the queue capacity is adequate for more than 50% of the Saturday peak hours (as shown by the 50th percentile queue), it would exceed the link capacity between the westbound and eastbound ramps during at least 5% of the Saturday peak hours (as shown by the 95th percentile queue). Even then, the excess queue would amount only to a single vehicle. This excess queue would in effect stretch through the westbound ramp intersection to the section of the new roadway north of the interchange. If signalized and coordinated with the signal for the eastbound ramps, the signal timing coordination could ensure that the eastbound ramp intersection would provide additional or offset green time to clear the westbound intersection. Additionally, even in a worse case scenario where traffic did extend through the westbound ramp intersection, these vehicles would not block any of the predominate movements through the intersection, since the other movements are associated primarily with southbound rights and westbound off-ramp right turns to the Rancheria. The only movements which would potentially be blocked would be associated with vehicles which intended to use the interchange to reverse their direction of travel along US-50. If the interchange were congested to such a degree that this extreme queuing developed, it is doubtful anyone would negotiate this movement at this particular interchange at that time.

Detailed queuing analysis is included within **Appendix E**.

Recommended Interchange Intersection Design

To summarize, it is recommended that the two newly created intersections be signalized, and that the signals be coordinated to assure that queues would not develop which would block the westbound ramp intersection. Therefore, this is considered a *less-than-significant impact*.

DIRECT IMPACT 4. EXISTING PLUS PROJECT- LOCAL ROADS ANALYSIS

Development of the project would add traffic to local roadways within El Dorado County.

AA No Project Alternative

There would be *no impact* associated with the no project alternative

AB Flyover Alternative

Table 28a provides a summary of all of the local roadway and highway segments within El Dorado County along which the project is projected to increase existing traffic volumes by 2% or more. The table also shows the resulting level of service along the roadway using capacities as established within spreadsheets associated with the El Dorado County model.

Table 28a
Existing Local Roadway Impact Summary (Existing + Project Volumes 2% over Existing)

Road	Segment	Geometry/Classification	Existing		Project		Existing + Project	
			Daily Vol	LOS	Daily Vol	% Inc	Daily Vol	LOS
BARKLEY RD.	North of Carson Rd	2-Lane Rural Local Rd	1,009	B	21	2.1%	1,030	B
BIG CUT RD	Benham St to Quarry Rd	2-Lane Rural Local Rd	1,068	B	28	2.6%	1,096	B
BUCKEYE RD	E. Shingle Springs to Mother Lode Rd (W)	2-Lane Rural Local Rd	201	A	59	29.4%	260	A
E. SHINGLE SPRINGS RD	US-50 to Buckeye Rd	2-Lane Rural Local Rd	1,886	C	83	4.4%	1,969	C
FORNI RD	Ray Lawyer Ext to Placerville Dr	2-Lane Local Rural Rd	902	B	18	2.0%	920	B
GREENSTONE RD.	Mother Lode to US 50	2-Lane Rural Minor Collector	1,085	B	322	29.7%	1,407	B
	US 50 to Green Vly Rd	2-Lane Rural Minor Collector	2,659	D	268	10.1%	2,927	D
GREEN VALLEY RD.	Lotus Rd to Greenstone	2-Lane Urban Minor Arterial	3,981	D	238	6.0%	4,219	D
LARSEN RD	Barkley Rd to North Canyon Rd	2-Lane Rural Local Rd	460	B	21	4.6%	481	B
LOTUS RD.	Green Vly Rd to GoldHill Rd	2-Lane Rural Minor Arterial	7,760	D	224	2.9%	7,984	D
	GoldHill Rd to Thompson Hill	2-Lane Rural Minor Arterial	4,860	D	207	4.3%	5,067	D
	Thompson Hill to Bassi Rd	2-Lane Rural Minor Arterial	4,985	E	207	4.2%	5,192	E
	Bassi Rd to SR 49	2-Lane Rural Minor Arterial	4,985	E	207	4.2%	5,192	E
MOTHER LODE DR.	Greenstone to Pleasant Vly Rd	2-Lane Rural Major Collector	10,001	E	279	2.8%	10,280	E
NORTH SHINGLE RD.	US 50 to Ponderosa	2-Lane Urban Minor Arterial	7,440	E	208	2.8%	7,648	E
OLD FRENCHTOWN RD.	French Crk to Mother Lode Dr	2-Lane Rural Minor Collector	1,301	B	28	2.2%	1,329	B
OXFORD RD	Cambridge Rd to Cameron Park Dr.	2-Lane Urban Collector	3,609	D	85	2.4%	3,694	D
PLEASANT VALLEY RD.	Mother Lode Dr to El Dorado Rd	2-Lane Rural Minor Arterial	8,045	D	250	3.1%	8,295	D
	El Dorado Rd to SR 49	2-Lane Rural Minor Arterial	9,680	E	243	2.5%	9,923	E
SR 49	County Line to Sandridge	2-Lane Rural Highway	5,600	C	146	2.6%	5,746	C
	Sandridge to Pleasant Valley Rd.	2-Lane Rural Highway	7,900	E	189	2.4%	8,089	E
	US 50 to SR 193 (N)	2-Lane Rural Highway	5,000	E	152	3.0%	5,152	E
	SR 193 (N) to Gold Hill	2-Lane Rural Highway	3,900	D	108	2.8%	4,008	D
	Gold Hill Rd to Cold Springs	2-Lane Rural Highway	1,800	C	101	5.6%	1,901	C
	Cold Springs to Lotus Rd	2-Lane Rural Highway	1,800	C	112	6.2%	1,912	C
	Lotus Rd to Marshall Rd.	2-Lane Rural Highway	4,300	C	235	5.5%	4,535	C
	Marshall to Salmon Falls Rd	2-Lane Rural Highway	2,750	B	203	7.4%	2,953	B
	Salmon Falls Rd to SR 193	2-Lane Rural Highway	2,750	D	198	7.2%	2,948	D

Note: Existing daily volumes based primarily on El Dorado County Department of Transportation's "2000 Traffic Count Annual Summary." Exceptions include Buckeye Road and Larsen Road which were derived from the 1999 version of the "El Dorado County Travel Demand Forecasting Model," and SR-49 which were obtained from "2000 Caltrans Traffic Volume on California State Highways" data as included on Caltrans' web site.

A local roadway is assumed to be significantly impacted by the project if it meets any of the following criteria:

- a) Degrading from acceptable LOS (A,B or C) without the project to an unacceptable LOS (D,E or F) with the project
- b) Degrading from unacceptable LOS D without project to unacceptable LOS E with the project, when the cumulative LOS is D or better (as defined within the 2015 CIP)
- c) Degrading from unacceptable LOS D without project to unacceptable LOS F with project
- d) Degrading from unacceptable LOS E without project to unacceptable LOS F with the project
- e) Unacceptable LOS F both without and with the project

Table 28b provides a summary of the portion of US-50 within El Dorado County along which the project is projected to increase existing traffic volumes by 2% or more (with the exception of US-50 between East Shingle Springs Drive and Greenstone Road, which is analyzed elsewhere for peak hour conditions). Traffic volumes east of Greenstone Road are not expected to increase by 2% or more. The table also shows the resulting level of service along the roadway using capacities as established within spreadsheets associated with the El Dorado County model.

Impacts along US-50 from the Sacramento/El Dorado County line to east of the project site are based on the concept level of service "E", established within *Caltrans' State Route 50 Transportation Concept Report* and additional information provided by Caltrans. Thus LOS F is considered an unacceptable level of service for US-50.

Table 28b
Existing Local Roadway (US-50) Impact Summary
(Existing + Project Volumes 2% over Existing)

Road	Segment	Geometry/Classification	Existing		Project		Existing + Project	
			Daily Vol	LOS	Daily Vol	% Inc	Daily Vol	LOS
US HIGHWAY 50								
	County Line to EDHB/Latrobe	4-Lane Freeway	70,000	D	4,235	6.1%	74,235	D
	EDHB/Latrobe to Bass Lake Rd	4-Lane Freeway	62,000	D	4,377	7.1%	66,377	D
	Bass Lake Rd to Cambridge Rd	4-Lane Freeway	56,000	C	4,397	7.9%	60,397	C
	Cambridge Rd to Cameron Pk Dr	4-Lane Freeway	55,000	C	4,439	8.1%	59,439	C
	Cameron Pk Dr to Shingle Springs	4-Lane Freeway	57,000	C	4,607	8.1%	61,607	C
	Shingle Springs to E. Shingle Spr	4-Lane Freeway	43,000	B	4,807	11.2%	47,807	C
	E. Shingle Spr to New Interchange	4-Lane Freeway	43,000	B	4,856	11.3%	47,856	C

Note: Existing daily volumes for US-50 from "2000 Caltrans Traffic Volume on California State Highways" data as included on Caltrans' web site.

Based on the methodology and impact analysis criteria established above, the proposed project was found to not significantly impact any of the local roadways and highways (including US-50, SR-49, and SR-193) for existing conditions on an average weekday. Therefore, this is considered a *less-than-significant impact*.

AC Diamond Alternative (tight diamond)

Impacts associated with Alternative AC are identical to those identified above for Alternative AB. Based on the methodology and impact analysis criteria established above, the proposed project was found to not significantly impact any of the local roadways and highways (including US-50, SR-49, and SR-193) for existing conditions on an average weekday. Therefore, this is considered a *less-than-significant impact*.

DIRECT IMPACT 5. CUMULATIVE PLUS PROJECT- RAMP MERGE/DIVERGE OPERATIONS

Development of the project would create new merge/diverge lanes along US-50 at newly created interchange ramps, thereby increasing peak hour congestion along US-50 between East Shingle Springs Road and Greenstone Road.

AA No Project Alternative

There would be *no impact* associated with the No Project Alternative.

AB Flyover Alternative

As described in the methodology section, freeway ramp merge/diverge analysis were analyzed two different ways to provide for a comparative analysis, and to account for the various alternatives which are currently being proposed. The first method of analysis calculates ramp merge/diverge operations for rolling terrain, and does not take into account the specific grades and grade lengths; whereas the second method uses actual specific grades and grade lengths of the freeway leading up to the ramp, and along the ramp itself.

For detailed analysis which takes into account the specific grade and grade lengths of the freeway and ramp in establishing ramp merge/diverge operations, the grade length is calculated by taking the straight portion of the grade leading to (and perhaps continuing past) the ramp juncture point, and adding to this value $\frac{1}{4}$ of the vertical curves at both the beginning and end of the grade. By calculating the specific grade and grade length along the freeway in this manner, minor differences in the location of the ramp juncture points for each alternative do not change the length of the grade.

Although level of service analysis as included within HCS (Highway Capacity Software) also provides for the input of specific ramp grade and length, this data is negligible in terms of the analysis unless the volume along the ramp approaches the capacity of the ramp. Within the analyses included within this report, it was found that the specific grade and length of the ramps itself were negligible in terms of the level of service of the ramp merge/diverge.

As discussed in the methodology section, freeway ramp merge/diverge analysis also provides for the consideration of interference to traffic flow and capacity based on the location of adjacent freeway ramps, and the traffic volumes to and from them. For 4-lane freeway merge/diverge analysis, the length to the adjacent ramp and the volume on the adjacent ramp does not impact the result in anyway since the equation used to calculate level of service does not include this

variable. Thus, although a distance to the adjacent ramp may be provided within 4-lane merge/diverge analysis included within the appendix, the value is irrelevant in terms of the level of service calculation.

A free flow speed of 48 kph (30 mph) was used for both project alternatives. This default value was used to approximate the design speed of 80 kph (50 mph) at inlet and exit noses of the ramps associated with Alternative AB, and 45 kph (28 mph) along the remainder of ramp.

Ramp merge/diverge analyses are performed only along the peak direction of travel for weekday peak hour conditions, which is along the westbound direction during the AM peak hour, and the eastbound direction during the PM peak hour. Ramp merge/diverge analyses are performed along both directions of travel for the Saturday peak hour. Analyses were not performed along the non-peak direction of travel following consultation and approval of Caltrans personnel.

Table 29a provides a summary of cumulative freeway merge/diverge operations along the existing 4-lane freeway for all three peak hour scenarios following the completion of the proposed interchange and proposed hotel/casino. The ramp merge/diverge level of service is applicable to all three scenarios, and uses "rolling terrain" to establish levels of service. The levels of service depicted are based on minimum acceleration/deceleration lengths of 250 ft. (76 meters).

Table 29a
Cumulative plus Project
Freeway Ramp Level of Service (Rolling Terrain)⁽¹⁾

	Level of Service		
	AM Peak Hour	PM Peak Hour	Saturday Peak Hour
EB off-ramp	-----	[REDACTED]	[REDACTED]
EB on-ramp	-----	E	E
WB off-ramp	[REDACTED]	-----	D
WB on-ramp	E	-----	E

Notes:

Length of deceleration and acceleration lanes assumed to be 250 ft.(76 meters).

(1) Level of service calculated using generalized "rolling" terrain instead of exact specific grade/length.

(2) LOS D is the target level of service for freeway diverge analysis, thus LOS E is unacceptable for off-ramp operations. LOS E is the target level of service for freeway mainline and merge (on-ramp) analyses.



As the above table shows, both the eastbound and westbound on ramps are projected to operate acceptably along a 4-lane facility during all three peak hours. However, the eastbound off-ramp is projected to operate unacceptably at LOS F during the weekday PM peak hour and Saturday peak hour, and the westbound off-ramp is projected to operate unacceptably at LOS E during the AM peak hour. The principal reason for these deficient operations is the LOS F operation along the freeway mainline which results in unacceptable levels of service at the ramp merge/diverge region.

Table 29b provides a similar summary of cumulative freeway merge/diverge operations, but is based on the use of the specific grade and grade length along the freeway. As previously described, it has been determined that the levels of service reported are the same for both alternatives. Again, the levels of service depicted are based on minimum acceleration/deceleration lengths of 250 ft.(76 meters).

Table 29b
Cumulative plus Project
Freeway Ramp Level of Service (Specific Grade/Length)⁽¹⁾

	Specific Grade	Specific Length (ft)	Level of Service		
			AM Peak Hour	PM Peak Hour	Saturday Peak Hour
EB off-ramp	+4.38	2,525	-----	[REDACTED]	[REDACTED]
EB on-ramp	+2.28	1,150	-----	E	D
WB off-ramp	-2.28	1,150	D	-----	D
WB on-ramp	-4.38	2,525	D	-----	D

Notes:

Length of deceleration and acceleration lanes assumed to be 250 ft.(76 meters).

(1) Level of service calculated using exact specific grade/length instead of generalized "rolling" terrain.

As the above table shows, some levels of services are shown to be slightly improved when compared to levels of service calculated when using "rolling" terrain instead of actual specific grades and grade lengths. Specifically, the westbound off-ramp is found to operate acceptably at LOS D for all peak hour scenarios, whereas both on-ramps are projected to operate acceptably at LOS E or better. Therefore, these are considered as *less-than-significant impacts*.

Using minimum acceleration/deceleration lengths of 250 ft. (76 meters) along a 4-lane facility, the eastbound off-ramp would continue to operate unacceptably at LOS F for cumulative conditions during both the weekday PM peak hour and Saturday peak hour.

The possibility of obtaining improved LOS by extending the deceleration lane leading from the eastbound off-ramp was investigated. The ramp will continue to operate unacceptably regardless of the length of the deceleration lane due to the high volumes and weaving along the freeway which exceeds capacity. This is considered a *significant impact*.

New Eastbound Auxiliary Lane

To provide acceptable level of service for the ramp diverge area of the eastbound off-ramp, it would be necessary to provide an eastbound auxiliary lane between the eastbound East Shingle Springs Drive on-ramp and the eastbound off-ramp to the Rancheria. The provision of this auxiliary lane would result in acceptable LOS D or better operation for the eastbound off-ramp during all three peak hour scenarios during the cumulative year.

Two different methodologies were utilized to analyze level of service operations for the weaving zone created by a new auxiliary lane; (1) methodologies as included within Caltrans' *Highway Design Manual*; and (2) methodologies described in the *1997 Highway Capacity Manual*. The first

methodology, which is the methodology preferred by Caltrans, is described within Section 504.12 – Weaving Sections of the Caltrans' *Highway Design Manual*. The Highway Design Manual states that “a rough rule of thumb for length of a weaving section is one foot of length per weaving vehicle.” For Cumulative plus Project conditions it is projected that eastbound US-50 would experience approximately 900 weaving vehicles during the Saturday peak hour, and 742 weaving vehicles during the weekday PM peak hour. In both cases, the weaving volumes would indicate the need for a weaving section of less than 1000 ft using the rule-of-thumb methodology. The weaving section length of approximately 2,500 ft. far exceed this.

A more detailed weaving analysis methodology is also included within the Caltrans' *Highway Design Manual* utilizing the nomograph depicted in Figure 504.7A. Although the nomograph can be used to determine the necessary weaving section length, it is intended more for design purposes. The output of the nomograph describes necessary weaving section length and number of lanes for given traffic volumes and a desired level of service. However, the nomograph can also be used to test proposed design and traffic volume characteristics, and determine if these result in an acceptable level of service, which was done for this analysis.

Using the nomograph, it was determined that the weaving section would operate at LOS D or better for all peak hour scenarios provided the weaving section was at least 175 meters long (approximately 575 ft.) and included three lanes.

Caltrans weaving analysis methodologies were supplemented by methodologies described in Chapter 4 of the *1997 Highway Capacity Manual*, which were analyzed using Highway Capacity Software (HCS). As discussed earlier, Chapter 4 methodologies are appropriate for weaving sections less than 2,500 ft. in length; whereas Chapter 5 ramp merge/diverge methodologies are considered more appropriate for weaving sections greater than 2,500 ft. in length. However, for this study, Chapter 4 methodologies were used to analyze weaving along the eastbound lanes preceding the new interchange's eastbound off-ramp for both alternatives.

For Alternative AB, the exact weaving length between the eastbound East Shingle Springs Drive on-ramp and the eastbound off-ramp to the Rancheria, as defined by Caltrans' *Highway Design Manual*, is 2,597 ft (791.5 meters)

For *1997 Highway Capacity Manual* weaving analysis, the weaving zone lengths are slightly longer due to a different definition of where the zone is said to begin and end relative to the freeway ramp gore points. Although the weaving length of the eastbound freeway segment upstream of the new off-ramp is technically within a few hundred feet over the 2,500 ft. threshold, the methodologies were still used to provide a worst case analysis.

Using HCM methodologies it was determined the 3-lane weaving section (which includes an eastbound auxiliary lane between the eastbound East Shingle Springs Drive on-ramp and the eastbound off-ramp to the Rancheria) would operate at an acceptable LOS D or better operation during all three peak hour scenarios during the cumulative year.

Figure 7 provides a summary of Cumulative plus Project ramp volumes and necessary improvements to provide acceptable ramp merge/diverge levels of service for a 4-lane freeway.

Figure 7

Ramp merge/diverge levels of service are calculated using specific freeway grade and grade length.

Detailed ramp merge/diverge analysis using both "rolling" terrain and specific grades and lengths for a 4-lane facility are provided in **Appendix F**; while analysis for the weaving section created by the provision of an eastbound auxiliary lane are provided in **Appendix G**.

AC Diamond Alternative (tight diamond)

Impacts associated with Alternative AC are identical to those identified above for Alternative AB.

Because of the methodology used to calculate the specific grade and grade length along the freeway, minor differences in the location of the ramp juncture points for each alternative do not change the length of the grade. Therefore, the specific grade and length of the ramps itself were negligible in terms of the level of service of the ramp merge/diverge, and the same specific grade and grade length along the freeway are applicable to analyses for both alternatives. To verify, a detailed analysis was performed for Alternative AB, and a trial and error input of the other possible input values for the Alternative AC was performed. This trial and error input included the ramp lengths, grades and volumes for Alternative AC, as well as extreme ramp lengths, grades and volumes outside the range of possibilities for Alternative AC. This trial and error analysis confirmed that the merge/diverge levels of service for the all alternatives and peak hour scenarios correspond to those calculated for Alternative AB. Therefore, for analyses which provide for the input of specific grades and grade lengths, the same values are used for both alternatives since the input for each result in exactly the same level of service results.

A free flow speed of 48 kph (30 mph) was used for both project alternatives. This default value was used to approximate the design speed of 80 kph (50 mph) at inlet and exit noses of the ramps associated with both alternatives, and 40 kph (25 mph) along the remainder of the ramp associated specifically with Alternative AC.

As the applicable tables for Alternative AB show, the westbound off-ramp is found to operate acceptably at LOS D for all peak hour scenarios, whereas both on-ramps are projected to operate acceptably at LOS E or better. Therefore, these are considered as *less-than-significant impacts*.

Using minimum acceleration/deceleration lengths of 250 ft. (76 meters) along a 4-lane facility, the eastbound off-ramp would operate unacceptably at LOS F for cumulative conditions during both the weekday PM peak hour and Saturday peak hour. The ramp will continue to operate unacceptably regardless of the length of the deceleration lane due to the high volumes and weaving along the freeway which exceeds capacity. This is considered a *significant impact*.

New Eastbound Auxiliary Lane

As was recommended for Alternative AB, to provide acceptable level of service for the ramp diverge area of the eastbound off-ramp, it would be necessary to provide an eastbound auxiliary lane between the eastbound East Shingle Springs Drive on-ramp and the eastbound off-ramp to the Rancheria. The provision of this auxiliary lane would result in acceptable LOS D or better

operation for the eastbound off-ramp during all three peak hour scenarios during the cumulative year.

For Alternative AC, the exact weaving length between the eastbound East Shingle Springs Drive on-ramp and the eastbound off-ramp to the Rancheria, as defined by Caltrans' *Highway Design Manual*, is 2,787 ft (849.5 meters)

Using HCM methodologies it was determined the 3-lane weaving section (which includes an eastbound auxiliary lane between the eastbound East Shingle Springs Drive on-ramp and the eastbound off-ramp to the Rancheria) would operate at an acceptable LOS D or better operation during all three peak hour scenarios during the cumulative year.

MEASURES TO MINIMIZE IMPACT

AB Flyover Alternative

Provide an eastbound auxiliary lane between the eastbound East Shingle Springs Drive on-ramp and the eastbound off-ramp to the Rancheria. The provision of this auxiliary lane would result in acceptable LOS D or better operation for the eastbound off-ramp during all three peak hour scenarios during the cumulative year.

AC Diamond Alternative (tight diamond)

Provide an eastbound auxiliary lane between the eastbound East Shingle Springs Drive on-ramp and the eastbound off-ramp to the Rancheria. The provision of this auxiliary lane would result in acceptable LOS D or better operation for the eastbound off-ramp during all three peak hour scenarios during the cumulative year.

DIRECT IMPACT 6. CUMULATIVE PLUS PROJECT- PEAK HR FREEWAY MAINLINE OPERATIONS

Development of the project would increase peak hour congestion along US-50 between East Shingle Springs Road and Greenstone Road.

AA No Project Alternative

There would be *no impact* associated with the No Project Alternative.

AB Flyover Alternative

Table 30 shows the freeway mainline operations along the existing 4-lane freeway for all three peak hour scenarios during the cumulative year 2025 following the completion of the proposed interchange and proposed hotel/casino.

**Table 30
Cumulative Plus Project (4-lanes)
Peak Hour Freeway Mainline Level of Service**

Freeway Segment	Freeway Level of Service ⁽¹⁾					
	Cumulative (no project)			Cumulative plus Project		
	AM Pk Hr	PM Pk Hr	Sat Pk Hr	AM Pk Hr	PM Pk Hr	Sat Pk Hr
EB US-50 (btwn E. Shingle Springs & Rancheria)	D	E	E	D		E
EB US-50 (btwn Rancheria & Greenstone)	D	E	E	D	E	E
WB US-50 (btwn Greenstone & Rancheria)	E	D	D	E	D	E
WB US-50 (btwn Rancheria & E. Shingle Springs)	E	D	D	E	D	E

Notes:

(1) Free Flow speed varies - "Ideal" Free Flow Speed of 65 mph adjusted to account for specific hwy geometry.

As the above table shows, the freeway is projected to operate acceptably at LOS E or better for both east and west of the proposed interchange along both directions during AM and Saturday peak hour conditions. During the PM peak hour, the freeway is projected to operate acceptably at LOS E or better both east and west of the proposed interchange along the westbound direction, and east of the proposed interchange along the eastbound direction. Therefore, these are considered *less-than-significant impacts*.

However, the freeway is projected to operate unacceptably at LOS F west of the proposed interchange along the eastbound direction during the PM peak hour. Therefore, this is considered a *significant impact*.

Figure 7 provides a summary of Cumulative plus Project freeway volumes and levels of service for a 4-lane freeway.

Detailed analysis data for a 4-lane freeway segment is provided in **Appendix H**.

AC Diamond Alternative (tight diamond)

Impacts associated with Alternative AC are identical to those identified above for Alternative AB.

As the above table shows, the freeway is projected to operate acceptably at LOS E or better for both east and west of the proposed interchange along both directions during AM and Saturday peak hour conditions. During the PM peak hour, the freeway is projected to operate acceptably at LOS E or better both east and west of the proposed interchange along the westbound direction,

and east of the proposed interchange along the eastbound direction. Therefore, these are considered *less-than-significant impacts*.

However, the freeway is projected to operate unacceptably at LOS F west of the proposed interchange along the eastbound direction during the PM peak hour. Therefore, this is considered a *significant impact*.

MEASURES TO MINIMIZE IMPACT

AB Flyover Alternative

Provide an eastbound auxiliary lane between the eastbound East Shingle Springs Drive on-ramp and the eastbound off-ramp to the Rancheria. The provision of this auxiliary lane would result in acceptable LOS D or better operation along the 3-lane weaving section (which includes an eastbound auxiliary lane between the eastbound East Shingle Springs Drive on-ramp and the eastbound off-ramp to the Rancheria) during all three peak hour scenarios during the cumulative year.

AC Diamond Alternative (tight diamond)

Provide an eastbound auxiliary lane between the eastbound East Shingle Springs Drive on-ramp and the eastbound off-ramp to the Rancheria. The provision of this auxiliary lane would result in acceptable LOS D or better operation along the 3-lane weaving section (which includes an eastbound auxiliary lane between the eastbound East Shingle Springs Drive on-ramp and the eastbound off-ramp to the Rancheria) during all three peak hour scenarios during the cumulative year.

DIRECT IMPACT 7. CUMULATIVE PLUS PROJECT- INTERCHANGE INTERSECTION OPERATIONS

Development of the project would create two new intersections within the newly created interchange, each of which would experience newly created delays.

AA No Project Alternative

There would be *no impact* associated with the no project alternative

AB Flyover Alternative

Since there are no intersections associated with the Flyover Alternative, there would be *no impact* associated with the Flyover Alternative.


AC Diamond Alternative (tight diamond)

Interchange operations are analyzed only for Alternative AC, since the modified trumpet design of Alternative AB contains no intersections to analyze. If constructed as a diamond interchange as proposed for Alternative AC, the interchange would include two intersections.

Unsignalized Intersection Analysis

Table 31a provides a summary of cumulative plus project intersection operations following the completion of the interchange and casino/hotel for unsignalized intersections.

**Table 31a
Cumulative plus Project
Unsignalized Level of Service**

Intersection	AM Peak Hour		PM Peak Hour		Saturday Peak Hour	
	LOS	delay (sec)	LOS	delay (sec)	LOS	delay (sec)
New Shingle Springs Rancheria Road / WB ramps	A	(1.3)	A	(1.0)	A	(1.2)
New Shingle Springs Rancheria Road / EB ramps	B	(5.5)	C	(12.0)		

Notes:

Applicable only for Alternative AC since Alternative AB includes no intersections.

Delay for unsignalized intersections based on overall average vehicle delay.



As the above table shows, the eastbound ramp intersection would not operate at an acceptable level of service as an unsignalized intersection.

Signalized Intersection Analysis

Both of the intersections would meet Caltrans signal warrants for rural conditions, although the westbound ramps are warranted only for weekday PM peak hour and Saturday peak hour conditions, and the eastbound ramps are warranted only for Saturday peak hour conditions. It is thus recommended that at a minimum the eastbound ramp intersection be signalized as part of the interchange construction. Although not necessary to provide acceptable level of service operation, it is also recommended that the westbound ramp intersection also be signalized to provide efficient operation.

Table 31b below shows the level of service for the two intersections following signalization for cumulative conditions.

**Table 31b
Cumulative plus Project
Signalized Level of Service**

Intersection	AM Peak Hour		PM Peak Hour		Saturday Peak Hour	
	LOS	delay (sec)	LOS	delay (sec)	LOS	delay (sec)
New Shingle Springs Rancheria Road / WB ramps	A	(3.4)	A	(3.6)	B	(7.0)
New Shingle Springs Rancheria Road / EB ramps	B	(8.3)	B	(6.8)	B	(8.4)

Notes:

Applicable only for Alternative AC since Alternative AB includes no intersections.

The signalized analysis was conducted using SYNCHRO software, which considered the effects of each of the two intersections upon each other.

Detailed analysis data for both unsignalized and signalized intersections, as well as a signal warrant worksheet, are provided in **Appendix I**.

Interchange Queuing Analysis

Using the SYNCHRO software analysis described previously, the queuing along each of the intersection approaches were analyzed. **Table 32** summarizes the queuing which would occur along each of the intersection approaches for cumulative conditions following the completion of the interchange project for signalized intersections.

As the below shows, the queuing capacity for each of the approaches is expected to easily accommodate the projected queues for each of the cumulative peak hour scenarios for Alternative AC, with the exception of the southbound approach to the eastbound ramp intersection during Saturday peak hour conditions. Although the queue capacity is adequate for more than 50% of the Saturday peak hours (as shown by the 50th percentile queue), it would exceed the link capacity between the westbound and eastbound ramps during at least 5% of the Saturday peak hours (as shown by the 95th percentile queue). Even then, the excess queue would amount only to a single vehicle. This excess queue would in effect stretch through the westbound ramp intersection to the section of the new roadway north of the interchange. If signalized and coordinated with the signal for the eastbound ramps, the signal timing coordination could ensure that the eastbound ramp intersection would provide additional or offset green time to clear the westbound intersection. Additionally, even in a worse case scenario where traffic did extend through the westbound ramp intersection, these vehicles would not block any of the predominate movements through the intersection, since the other movements are associated primarily with southbound rights and westbound off-ramp right turns to the Rancheria.

**Table 32
Cumulative plus Project
Intersection Queuing for Signalized Intersections**

Movement	Available Queue Capacity (ft)	AM Peak Hour		PM Peak Hour		Saturday Peak Hour	
		Queue (ft)		Queue (ft)		Queue (ft)	
		50 th Percentile	95 th Percentile	50 th Percentile	95 th Percentile	50 th Percentile	95 th Percentile
<u>New Project Access / Westbound US-50 Ramps</u>							
NB	163	0	0	0	0	0	0
SB Thru	760+	8	28	25	65	46	91
SB Right	760+	21	59	53	133	134	340
WB	1,275	56	93	54	94	70	131
<u>New Project Access / Eastbound US-50 Ramps</u>							
SB	163	18	46	50	117	87	174 [11]
EB	983	92	140	125	172	134	216

Notes:

The only movements which would potentially be blocked would be associated with vehicles which intended to use the interchange to reverse their direction of travel along US-50. If the interchange were congested, it is doubtful anyone would negotiate this movement at this particular interchange.

Detailed queuing analysis is included within Appendix I.

Recommended Interchange Intersection Design

To summarize, it is recommended that the two newly created intersections be signalized, and that the signals be coordinated to assure that queues would not develop which would block the westbound ramp intersection. Therefore, this is considered a *less-than-significant impact*.

DIRECT IMPACT 8. CUMULATIVE PLUS PROJECT- RAMP METERING

Development of the project would create two new on-ramps, which would increase congestion along US-50 during periods when on-ramps experience high volumes warranting ramp metering.

AA No Project Alternative

There would be *no impact* associated with the no project alternative

AB Flyover Alternative

Ramp metering analysis was performed for 2010 conditions along the proposed eastbound and westbound on-ramps using analysis methodologies as described within Caltrans' *Ramp Meter Design Manual, 2000*. The analysis assumes that the proposed on-ramps would be metered in the cumulative condition. However, it should be noted that ramp metering is not proposed as part of the interchange project, or in any 20-year Transportation System Management Plans. The analysis was performed to insure that there is proper storage available on the ramps for expected queues, should metering be implemented. This was determined by investigating whether the minimum metering rate permitted by Caltrans can accommodate maximum vehicle queues within the available storage length of the on-ramps.

In performing this analysis, metered single-lane entrance ramps were considered for the on-ramps.

The peak hour volume for the Saturday conditions was used as a worst case condition since the volumes along the ramps are highest compared to other peak hour periods. The ramp storage length was determined by measuring the length directly from the design layout of the new interchange. Also, minimum vehicle spacing of 9 meters (29.5 ft) was used in considering storage length on metered ramps.

Table 33 provides a summary of the ramp metering conditions along the new on-ramps for Saturday peak hour conditions. Saturday peak hour conditions is when traffic volumes along the new on-ramps would be heaviest. The table shows that the ramp metering at the proposed on-ramps would operate without the queue exceeding the storage length if metering rates were as provided.

**Table 33
Ramp Metering**

On-Ramp	Storage Length	Storage Vehicles	Peak Hour Volumes	Metering Rate (vph)
Westbound on-ramp	472 ft (144 meters)	16	615	600
Eastbound on-ramp	640 ft (195 meters)	22	305	285

Provided the recommended metering rates are provided, this is considered a *less-than-significant impact*.

AC Diamond Alternative (tight diamond)

Impacts associated with Alternative AC are identical to those identified above for Alternative AB. Provided the recommended metering rates are provided, this is considered a *less-than-significant impact*.

DIRECT IMPACT 9. CUMULATIVE PLUS PROJECT- LOCAL ROADS ANALYSIS

Development of the project would add traffic to local roadways within El Dorado County.

AA No Project Alternative

There would be *no impact* associated with the no project alternative

AB Flyover Alternative

Table 34a provides a summary of all of the local roadway and highway segments within El Dorado County along which the project is projected to increase existing traffic volumes by 2% or more. The table also shows the resulting level of service along the roadway using capacities as established within spreadsheets associated with the El Dorado County model.

Roadway geometries, and corresponding capacities, for cumulative conditions are based on the roadway network geometries included within the 2022 CIP (Capitol Improvement Program) data network files contained with the 1999 El Dorado County travel demand model.

A local roadway is assumed to be significantly impacted by the project if it meets any of the following criteria:

- a) Degrading from acceptable LOS (A,B or C) without the project to an unacceptable LOS (D,E or F) with the project
- b) Degrading from unacceptable LOS D without project to unacceptable LOS E with the project, when the cumulative LOS is D or better (as defined within the 2015 CIP)
- c) Degrading from unacceptable LOS D without project to unacceptable LOS F with project
- d) Degrading from unacceptable LOS E w/out project to unacceptable LOS F with project
- e) Unacceptable LOS F both without and with the project

Table 34b provides a summary of the portion of US-50 within El Dorado County along which the project is projected to increase existing traffic volumes by 2% or more (with the exception of US-50 between East Shingle Springs Drive and Greenstone Road, which is analyzed elsewhere for peak hour conditions). Traffic volumes east of Greenstone Road are not projected to increase by 2% or more. The table also shows the resulting level of service along the roadway using capacities as established within spreadsheets associated with the El Dorado County model.

Based on the methodology and impact analysis criteria established above, the proposed project was found to not significantly impact any of the local roadways and highways (including SR-49 and SR-193, but excluding US-50) for cumulative conditions on an average weekday. Therefore, this is considered a *less-than-significant impact*.

Impacts along US-50 from the Sacramento/El Dorado County line to east of the project site are based on the concept level of service "E", established within *Caltrans' State Route 50 Transportation Concept Report* and additional information provided by Caltrans. Thus LOS F is considered an unacceptable level of service for US-50.

Table 34a
Cumulative Local Roadway Impact Summary
(Cumulative + Project Volumes 2% over Cumulative No Project)

Road	Segment	Geometry/Classification	Cumulative		Project		Cumulative + Project	
			Daily Vol	LOS	Daily Vol	% Inc	Daily Vol	LOS
BIG CUT RD								
	Benham St to Quarry Rd	2-Lane Rural Local Rd	1,388	B	28	2.0%	1,416	B
BUCKEYE RD								
	E. Shingle Springs to Mother Lode Rd (W)	2-Lane Rural Local Rd	469	B	59	12.6%	528	B
GREENSTONE RD.								
	Mother Lode to US 50	2-Lane Rural Minor Collector	922	B	322	34.9%	1,244	B
	US 50 to Green Vly Rd	2-Lane Rural Minor Collector	3,784	B	268	7.1%	4,052	B
GREEN VALLEY RD.								
	Lotus Rd to Greenstone	4-Lane Divided Urban Minor Arterial (Upgraded)	4,308	A	238	5.5%	4,546	A
LARSEN RD								
	Barkley Rd to North Canyon Rd	2-Lane Rural Local Rd	618	B	21	3.4%	639	B
LOTUS RD.								
	Green Vly Rd to GoldHill Rd	2-Lane Rural Minor Arterial	9,077	D	224	2.5%	9,301	D
	GoldHill Rd to Thompson Hill	2-Lane Rural Minor Arterial	6,360	C	207	3.3%	6,567	C
	Thompson Hill to Bassi Rd	2-Lane Rural Minor Arterial	6,360	E	207	3.3%	6,567	E
	Bassi Rd to SR 49	2-Lane Rural Minor Arterial	6,360	E	207	3.3%	6,567	E
MISSOURI FLAT RD.								
	MO Flat Conn to Pleasant Valley Rd	2-Lane Rural Minor Arterial	8,236	D	184	2.2%	8,420	D
MOTHER LODE DR.								
	Greenstone to Pleasant Vly Rd	2-Lane Rural Major Collector	11,372	E	279	2.5%	11,651	E
PLEASANT VALLEY RD.								
	Mother Lode Dr to El Dorado Rd	2-Lane Rural Minor Arterial	9,166	E	250	2.7%	9,416	E
	El Dorado Rd to SR 49	2-Lane Rural Minor Arterial	11,000	E	243	2.2%	11,243	E
SR 49								
	Mo Flat Rd to Pleasant Vly (DS)	2-Lane Rural Highway	5,792	D	132	2.3%	5,924	D
	US 50 to SR 193 (N)	2-Lane Rural Highway	6,200	E	152	2.5%	6,352	E
	SR 193 (N) to Gold Hill	2-Lane Rural Highway	4,487	E	108	2.4%	4,595	E
	Gold Hill Rd to Cold Springs	2-Lane Rural Highway	2,362	C	101	4.3%	2,463	C
	Cold Springs to Lotus Rd	2-Lane Rural Highway	3,545	D	112	3.2%	3,657	D
	Lotus Rd to Marshall Rd.	2-Lane Rural Highway	6,026	C	235	3.9%	6,261	C
	Marshall to Salmon Falls Rd	2-Lane Rural Highway	4,509	B	203	4.5%	4,712	B
	Salmon Falls Rd to SR 193	2-Lane Rural Highway	6,589	C	198	3.0%	6,787	C

Note: Cumulative daily volumes are 2022 year volumes as reported within the 1999 version of the "El Dorado County Travel Demand Forecasting Model."

Table 34b
Cumulative US-50 Impact Summary
(Cumulative + Project Volumes 2% over Cumulative No Project)

Segment	Geometry/ Classification	Cumulative		Project		Cumulative + Project		Target LOS E or Better? (2)
		Daily Vol ⁽¹⁾	LOS	Daily Vol	% Inc	Daily Vol	LOS	
County Line to EDHB/Latrobe	6-Lane Freeway			4,235	3.1%			
EDHB/Latrobe to Bass Lake Rd	6-Lane Freeway	111,347	D	4,377	3.9%	115,724	E	Yes
Bass Lake Rd to Cambridge Rd	6-Lane Freeway	98,896	D	4,397	4.4%	103,293	D	Yes
Cambridge Rd to Cameron Pk Dr	6-Lane Freeway	86,247	C	4,439	5.1%	90,686	C	Yes
Cameron Pk Dr to Shingle Springs	6-Lane Freeway	72,045	C	4,607	6.4%	76,652	C	Yes
Shingle Springs to E. Shingle Spr	4-Lane Freeway	63,285	D	4,807	7.6%	68,092	D	Yes
E. Shingle Spr to New Interchange	4-Lane Freeway	61,690	C	4,856	7.9%	66,546	D	Yes

Notes:

- (1) Cumulative daily volumes are 2022 year volumes as reported within the 1999 version of the "El Dorado County Travel Demand Forecasting Model," with exception of County Line to EDHB/Latrobe which was supplied by Caltrans.
- (2) Concept level of service for US-50 is LOS E based on Caltrans' State Route 50 Transportation Concept Report.

Based on the methodology and impact analysis criteria established above, the proposed project was found to not significantly impact US-50 within El Dorado County east of El Dorado Hills Boulevard. Therefore, this is considered a *less-than-significant impact*.

However, as noted, it is anticipated that the section of US-50 between the El Dorado County Line and El Dorado Hills Boulevard will operate at a deficient LOS F operation without the project. The addition of project traffic will add to the projected adverse operation within this section of US-50. To address this incremental cumulative impact, it is recommended that the project participate in a fair share contribution for future master planned improvements as identified by Caltrans and El Dorado County for this section of freeway.

AC Diamond Alternative (tight diamond)

Impacts associated with Alternative AC are identical to those identified above for Alternative AB.

Based on the methodology and impact analysis criteria established above, the proposed project was found to not significantly impact any of the local roadways and highways (including SR-49 and SR-193, but excluding US-50) for cumulative conditions on an average weekday. Therefore, this is considered a *less-than-significant impact*.

Based on the methodology and impact analysis criteria established above, the proposed project was found to not significantly impact US-50 within El Dorado County east of El Dorado Hills Boulevard. Therefore, this is considered a *less-than-significant impact*.

However, as noted, it is anticipated that the section of US-50 between the El Dorado County Line and El Dorado Hills Boulevard will operate at a deficient LOS F operation without the project.

The addition of project traffic will add to the projected adverse operation within this section of US-50. To address this incremental cumulative impact, it is recommended that the project participate in a fair share contribution for future master planned improvements as identified by Caltrans and El Dorado County for this section of freeway.

MEASURE TO MINIMIZE IMPACT

Pursuant to Section 10.8 of Tribal State Compact, the tribal government will contribute a fair share contribution to future master planned improvements as identified by Caltrans and El Dorado County for the section of US-50 between the El Dorado County Line and El Dorado Hills Boulevard.

SUMMARY OF DIRECT IMPACTS

This traffic operations analysis shows that the proposed interchange will result in acceptable levels of service along the freeway mainline and at the new ramp merge/diverge areas for Existing Conditions for both alternatives. For Alternative AC, which include new intersections, it is recommended that the new eastbound ramp intersection be signalized for existing conditions to accommodate trips generated by the casino/hotel. Although not necessary to provide acceptable level of service operation, it is also recommended that the westbound ramp intersection also be signalized to provide efficient operation. Both of the intersections would meet Caltrans signal warrants for rural conditions, although the westbound ramps are warranted only for weekday PM peak hour and Saturday peak hour conditions, and the eastbound ramps are warranted only for Saturday peak hour conditions.

The 2010 ramp metering analysis shows that both of the proposed on-ramps can easily accommodate the maximum number of peak hour trips generated by Rancheria and casino/hotel complex with allowable ramp metering rates.

For cumulative conditions, all of the study freeway segments are projected to operate acceptably at LOS E or better during AM and Saturday peak hour conditions. During the PM peak hour, the freeway is projected to operate acceptably at LOS E or better both east and west of the proposed interchange along the westbound direction; and east of the proposed interchange along the eastbound direction. However, the freeway is projected to operate unacceptably at LOS F west of the proposed interchange along the eastbound direction during the PM peak hour. This deficiency would also result in unacceptable levels of service within the ramp diverge area.

The operational problem along eastbound US-50 west of the interchange is due to the concentration of exiting traffic in the outer lane for the eastbound off-ramp. The provision of an auxiliary lane (or an additional mixed flow lane) will mitigate this conditions. It is thus recommended that an auxiliary lane be constructed along eastbound US-50 between the East Shingle Springs Drive on-ramp and the new interchange's eastbound off-ramp. To allow for the eventual widening of the freeway to a 6-lane facility, which Caltrans has stated they eventually plan to do, it is recommended that a third eastbound lane be constructed within the median between East Shingle Springs Drive and the new interchange. Following construction, it is

recommended that traffic in the current #1 lane be transitioned to the newly constructed lane within the median at East Shingle Springs Drive, and that traffic in the current #2 lane be transitioned to the old #1 lane (newly created center lane). East of the eastbound off-ramp of the new interchange, traffic would be transitioned back to the original two lanes. With this new transition, the current #2 lane would be restriped to provide an auxiliary lane between the East Shingle Springs Drive on-ramp and the new interchange's eastbound off-ramp. This alignment would easily accommodate the eventual widening of US-50 to 6-lanes.

The other ramp merge/diverge areas would operate acceptably along a 4-lane US-50 highway provided the westbound ramps provided a minimum of 250 ft. (76 meters) long acceleration/deceleration lanes, and the eastbound on-ramp provided a minimum of 300 ft. (91 meters) long acceleration lane. These recommendations are applicable to both alternatives. A lengthening of the acceleration/deceleration lanes as traffic volumes increase helps to improve the operation and safety of the merge/diverge areas. This improvement is due to the necessity of vehicles to use more of the acceleration/deceleration lane (or "speed-change" lane) to increase or decrease speeds between the freeway and ramp, then would be necessary with lower traffic volumes. When volumes are lower, vehicles are able to use more of the ramp and/or freeway mainline to increase or decrease their speed, but when traffic volumes are high, the need arises to use more of the acceleration/deceleration lane for this purpose, hence an increase in the length is warranted.

The traffic operations analysis within this report is for "typical" peak hour scenarios, even for peak month/seasonal conditions. It is possible that the proposed casino facility will have occasional special events which will result in higher traffic volumes. If special events are planned at the casino facility (or elsewhere within the Rancheria) which will generate traffic volumes than those reported within this analysis, it is recommended that a traffic management plan be established and implemented that will minimize traffic impacts to US-50. This could potentially include, but not necessarily be limited to, parking and traffic control personnel directing traffic, limiting or eliminating conflicting turning movements such as traffic turning left to the eastbound on-ramp, traffic cones to reduce or eliminate weaving and lane changes, etc.

It is anticipated that the section of US-50 between the El Dorado County Line and El Dorado Hills Boulevard will operate at a deficient LOS F operation without the project for daily conditions. The addition of project traffic will add to the projected adverse operation within this section of US-50. Pursuant to Section 10.8 of Tribal State Compact, the tribal government will contribute a fair share contribution to future master planned improvements as identified by Caltrans and El Dorado County for the section of US-50 between the El Dorado County Line and El Dorado Hills Boulevard.

Caltrans personnel have stated that due to uncertainty regarding trip generation and passer-by capture assumptions, the following measures should be taken:

- Loop detectors should be placed on all ramps
- Following completion of the project, traffic volumes should be counted and monitored at the new ramp interchange/intersections, freeway mainline, and driveways to the casinos to determine if future modifications are necessary.

- If possible, a license plate survey may also be needed to determine trip distribution in the area.

However, the trip generation assumptions used within this report are assumed to be conservative. It is expected that time periods where trip generation and conditions described within this report would be rare. The worst case scenario is the situation where crowds greater than those typically anticipated would cause the off-ramps to back up onto the freeway interfering with through traffic. However, this condition is unlikely provided queue detectors are installed for the off-ramp signals, which would provide green lights to off-ramp traffic before it backed up onto the freeway. Since the new roadway leading to the rancheria and casino would provide four lanes of travel, in essence each on-ramp and off-ramp leads to its own lane on the local road system. Thus, there are essentially no negative consequences to the local streets that might be caused by the extra traffic.

The proposed interchange is projected to work as well as it will because it will be comparatively lightly utilized and is custom designed for the anticipated conditions with a comfortable factor of safety.

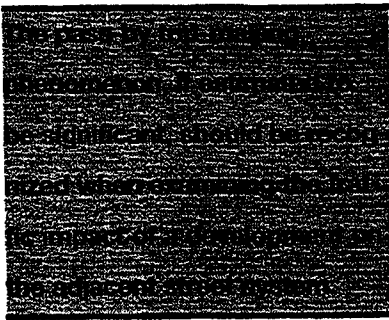
APPENDIX F

Pass-By Trips

Pass-by, Primary, and Diverted Linked Trips

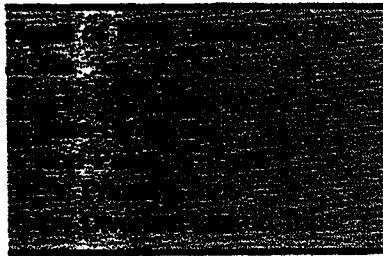
5.1 Background

The trip generation rates and equations contained in *Trip Generation* are derived from actual measurements of traffic generated by individual sites. These rates and equations represent vehicles entering and exiting a site at its driveways. Therefore, these volumes are appropriate for determining the total traffic to be accommodated by site driveways.

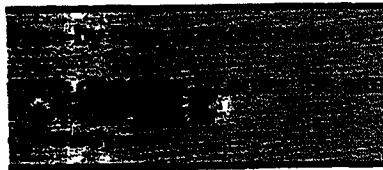


There are instances, however, when the total number of trips generated by a site is different from the amount of new traffic added to the street system by the generator. For example, retail-oriented developments such as shopping centers, discount stores, restaurants, banks, service stations, and convenience markets often locate adjacent to busy streets in order to attract the motorists already on the street. These sites attract a portion of their trips from traffic passing the site on the way from an origin to an ultimate destination. These retail trips may not add new traffic to the adjacent street system.

Trip-making can be broken down into two major categories: **pass-by trips** and **non-pass-by trips**. In some traffic impact study applications, it is necessary to further subdivide non-pass-by trips into **primary trips** and **diverted linked trips**. These trip types are illustrated in figure 5.1 and are defined below.



Pass-by trips are made as intermediate stops *on the way* from an origin to a primary trip destination without a route diversion. Pass-by trips are attracted from traffic passing the site *on an adjacent street* or roadway that offers direct access to the generator. **Pass-by trips are not diverted from another roadway.**

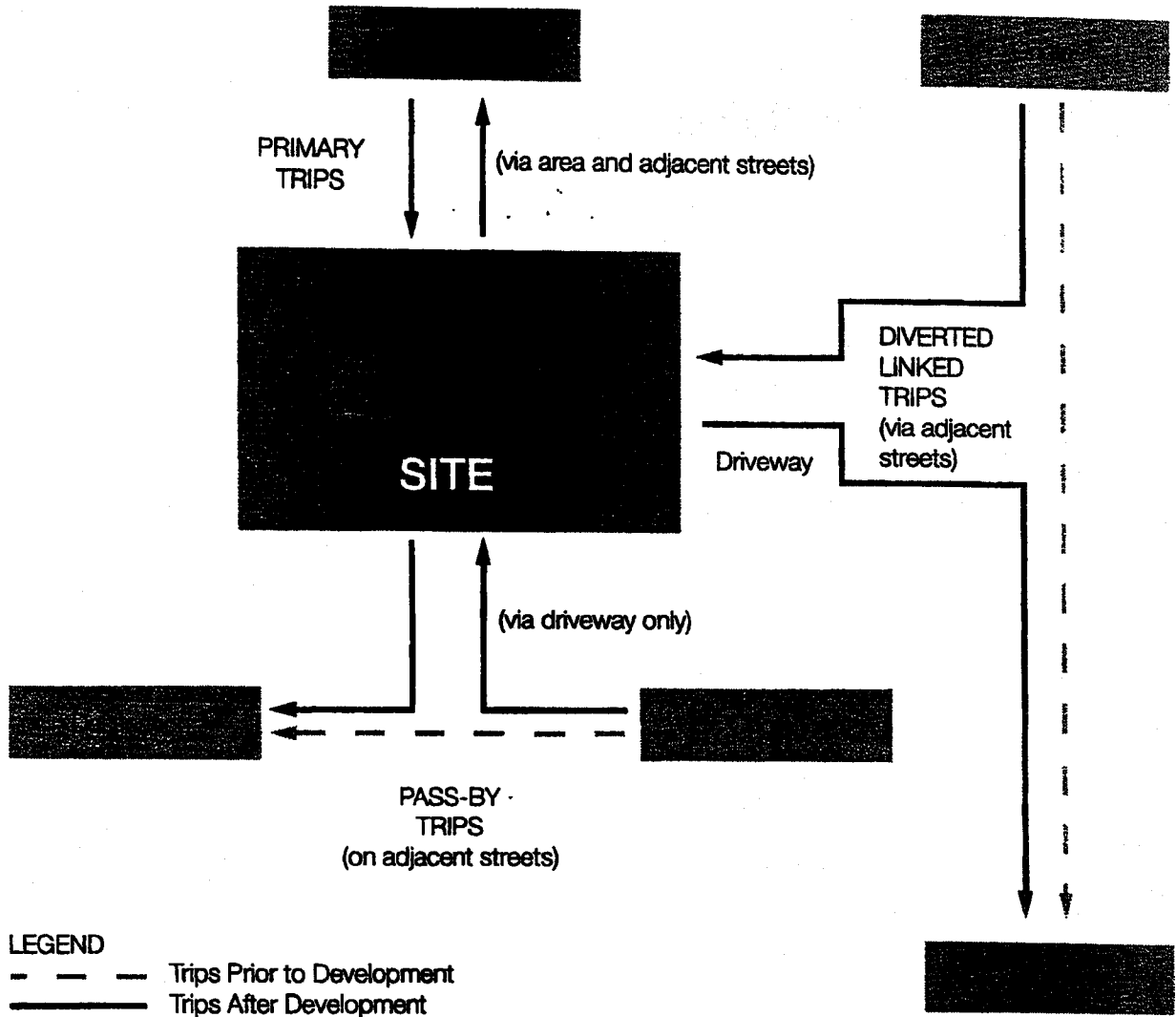


Non-pass-by trips are simply all trips generated by a site that are not pass-by trips. This term is sometimes used when diverted linked trips are not tabulated separately from primary trips.

Primary trips are trips made for the specific purpose of visiting the generator. The stop at the generator is the primary reason for the trip. The trip typically goes from origin to generator and then returns to the origin. For example, a home-to-shopping-to-home combination of trips is a primary trip set.

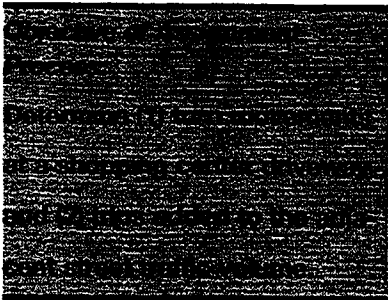
Diverted linked trips are trips that are attracted from the traffic volume on roadways within the vicinity of the generator but that require a diversion from that roadway to another roadway to gain access to the site. These trips could travel on highways or freeways adjacent to a generator, but without access to the generator. **Diverted linked trips add traffic to streets adjacent to a site, but may not add traffic to the area's major travel routes** (see figure 5.1). Both pass-by and diverted linked trips may be part of a multiple-stop chain of trips.

Figure 5.1 Types of Trips



5.2 Sample Application of Pass-By Trip Assignment Process

In this example, the objectives are to (1) estimate the number of new trips added to the adjacent street traffic volume with the development of a shopping center with 580,000 square feet of gross leasable area, and (2) determine the turn movements at the shopping center driveway. The forecasted two-way evening peak hour traffic on a street adjacent to the proposed shopping center is 1,200 vehicles, as shown in figure 5.2(A)—1,000 traveling west and 200 traveling east.



The shopping center is estimated to generate 2,000 evening peak hour trips (based on the fitted curve equation given for Land Use Code 820 on page 1,339 of *Trip Generation*, Sixth Edition). An assessment of the shopping center parking configuration and access points indicates that an estimated 20 percent of the site-generated traffic will use the driveway being analyzed in this example. Thus, the driveway volume is estimated to be 400 evening peak hour trips (i.e., 20 percent of 2,000 trips). For this

example, 50 percent enter and 50 percent exit the shopping center (as shown in figure 5.2(B)).

From data collected at other shopping centers, it is estimated (in this example) that about 15 percent of the driveway volume is pass-by (figure 5.2(B)). Therefore, 30 of the inbound vehicles (i.e., 15 percent of 200 vehicles) and 30 of the outbound vehicles are considered pass-by trips.

The assumed trip distribution for the non-pass-by trips is shown in figure 5.2(C). These values are based on local knowledge of expected trip patterns for primary and diverted linked trips to and from the shopping center (based on existing travel patterns, surrounding land uses, etc.). For example, 80 percent of the non-pass-by trips are expected to arrive from the east and to return to the east after the trip to the shopping center.

The distribution of the pass-by trips is based on the volume of traffic passing the driveway, as shown in figure 5.2(D). Because 83 percent of the traffic passing by the site comes from the east (i.e., 1,000 of the 1,200 shown previously in figure 5.2(A)), it is assumed that 83 percent of the pass-by trips will likewise arrive from the east and will depart toward the west.

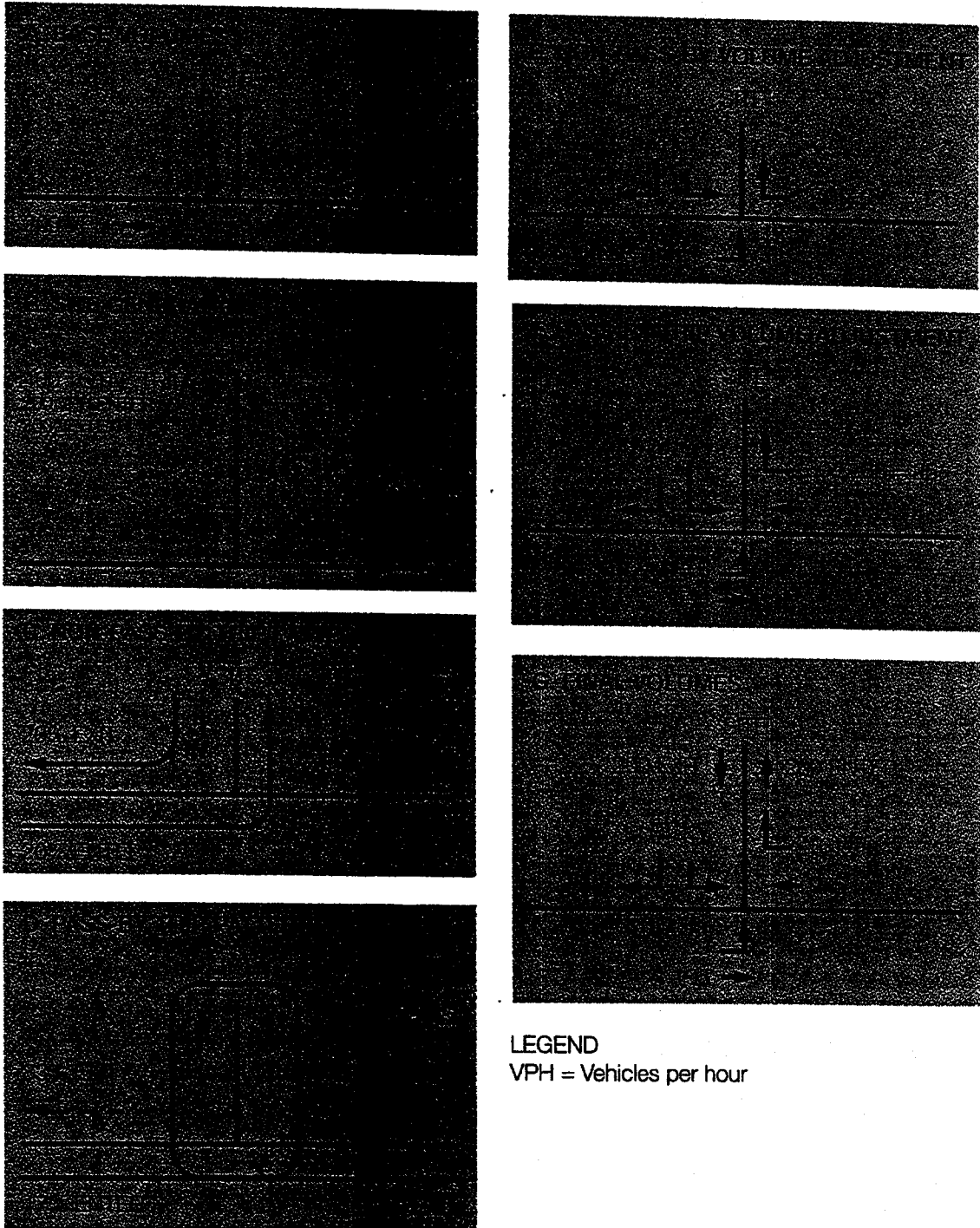
The assignment of the non-pass-by trips generated by the site is shown in figure 5.2(E). The total number

of non-pass-by trips destined to the site is 170 (the 200 total trips minus the 30 inbound pass-by trips shown earlier in figure 5.2(B)). Eighty percent (or 136) are expected to arrive from the east and to return to the east.

The assignment of the pass-by trips is shown in figure 5.2(F). Of the 30 pass-by trips, 83 percent (or 25) arrive from the east and depart to the west. Likewise, 17 percent (or 5) arrive from the west and depart to the east. Note that the calculation also shows the expected through-trip reductions as the trips passing the site turn into the new driveway. For example, the new westbound right-turn volume of 25 causes a reduction in the westbound through movement.

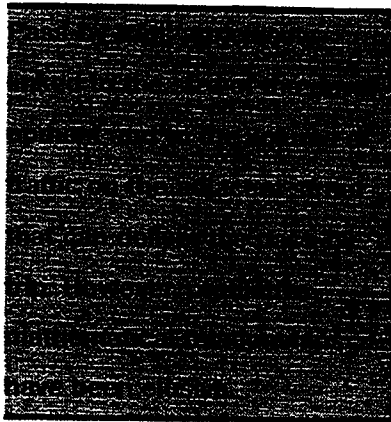
The final assignment of all trips entering and leaving the shopping center driveway, as well as passing the driveway, is shown in figure 5.2(G). These values are simply the sum of the base volumes (from figure 5.2(A)), the non-pass-by trips generated by the site (from figure 5.2(E)), and the pass-by trips generated by the site (from figure 5.2(F)). Note that the through-traffic volumes in both directions on the major street are reduced as a result of the pass-by trip analysis.

Figure 5.2 Application of Pass-By Trips



5.3 Cautions

Statistical analysis and correlation of the pass-by data collected by the profession continue to evolve. However, due to the limited amount of pass-by data available and the inherent variability in surveyed site characteristics, it has still proven difficult to obtain high correlation indices.



Traditional pass-by trip analyses have attempted to correlate pass-by trip percentages (i.e., percent of the total number of trips generated by a site) with units of occupied site development (such as gross leasable area, gross floor area, seats in a restaurant, or fueling positions at a gas/service station). Limited results for some land uses show that this correlation can be enhanced further

by including the magnitude of the traffic passing the site on the adjacent roadways.

The analyst should exercise caution in the use of pass-by and diverted linked data presented in this chapter to ensure that the following aspects of pass-by trip characteristics are handled appropriately in the analysis process.

Diverted linked trips are clearly different from pass-by trips.

Diverted linked trips add trips to the adjacent roads at a proposed or expanded site, but may not add trips to nearby major highways or freeways.

Diverted linked trips are often difficult to identify. Therefore, **diverted-linked trips should be treated similarly to primary trips**, unless: (1) all three (primary, pass-by, and diverted linked) categories are being analyzed and processed separately, and (2) the travel routes for diverted linked trips can be clearly established.

Pass-by trips are drawn from the passing traffic stream, **but are always included in the site driveway movements**. In traffic analyses, summation of driveway

volumes must equal the total external site generation (i.e., the sum of primary, pass-by, and diverted linked trips). Pass-by trips are not included in (and thus, subtracted from) the through-volumes passing a given site access point on an adjacent road. Standard methodologies for assessing the traffic impacts of site development typically require that diverted linked trips be included as additional trips within the confines of local impact assessment studies.

In a multi-use development, it is likely that there will be trips internal to the site (refer to chapter 7 for guidance). Before applying the pass-by reduction, the internal trips should be removed from the total number of trips generated by the multi-use site. **Pass-by trips are only applicable to trips that enter or exit the site, not internal trips.**

Overall, diverted linked trips represent a change in local area travel patterns but constitute no new increase on a *macroscopic* scale.

Within the immediate study area, diverted linked trips do represent additional traffic on individual streets and should be analyzed that way.

5.4 Data Base on Pass-By, Primary, and Diverted Linked Trips

Listed in table 5.1 are 19 land uses for which ITE has received and compiled pass-by and diverted linked trip data. The table denotes whether the data are presented in this handbook in a table or a figure (in a data plot similar to those presented in *Trip Generation* for trip end data). Table 5.1 also identifies the time periods for which the data have been reported.

Tables 5.2 through 5.26 present the values for percentage of site generation that is accounted for by pass-by, non-pass-by, primary, and diverted linked trips.

Figures 5.3 through 5.15 plot the average *pass-by* trip percentages associated with the various land uses. No plots are provided for *diverted linked* trips. These figures are provided to enable the user to visualize the data scatter provided in tables 5.2 through 5.26.

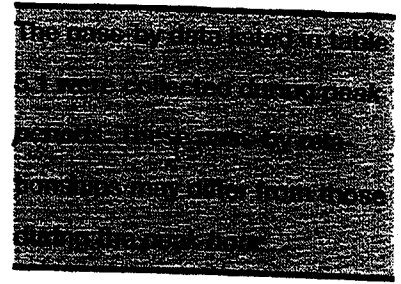
Data plots are provided for each land use where nine or more data points are available for a specific independent variable.

For all land uses except shopping centers, data are plotted for only one independent variable. For shopping centers, data are plotted for GLA and peak hour traffic on adjacent streets for the weekday evening peak period; GLA is also used as the independent variable for shopping centers during the midday Saturday time period.

A regression equation is shown on the data plot if there are more than 10 points and the R^2 is greater than 0.25 (which only occurs on two of the Land Use Code 820 data plots). Note that this threshold is less than the 0.5 threshold for R^2 used for data plots in *Trip Generation*.

Recommended guidelines for using the data presented in these figures and tables are provided in section 5.5 of this chapter. In particular, the guidelines recommend when to use the data and how to select a pass-by percentage.

Users of the data are cautioned that the number and geographic distribution of sites are limited. Little or no data on adjacent street traffic volumes have been collected for uses other than shopping centers. The actual pass-by and diverted



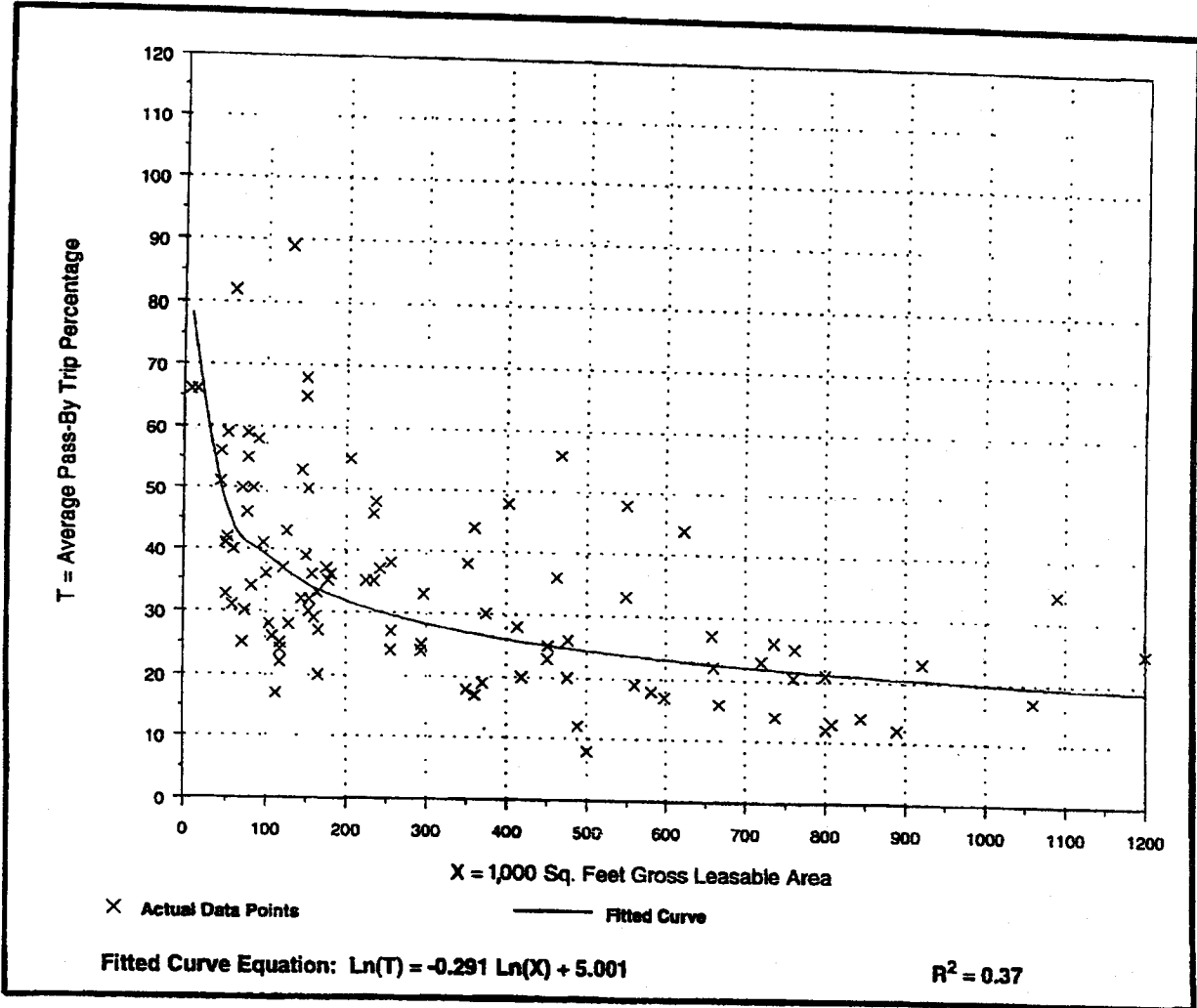
linked trip percentages may vary by site due to the specific influences of the characteristics of passing traffic, area roadway network patterns, specific businesses in the site being analyzed, other nearby development, and so forth. Surveys of similar developments near the analysis site are encouraged.

Because data are limited for many of the land uses, the analyst is encouraged to collect pass-by trip data and transmit the data to ITE. Section 5.6 of this chapter describes how to collect the appropriate data and provides sample forms to use.

Figure 5.5 Shopping Center (820)

Average Pass-By Trip Percentage vs: 1,000 Sq. Feet Gross Leasable Area
On a: Weekday, P.M. Peak Period
Number of Studies: 100
Average 1,000 Sq. Feet GLA: 329

Data Plot



APPENDIX G

Freeway Analysis

Table 1

Existing Intersection Delay and Level of Service

Intersection	Traffic Control ³	Intersection Approach Lanes ¹												Peak Hour Delay (Secs.) - LOS ²	
		Northbound			Southbound			Eastbound			Westbound			Morning	Evening
		L	T	R	L	T	R	L	T	R	L	T	R		
I-215 Freeway SB Ramps (NS) at: Bonnie Drive (EW) ⁴	CSS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	52.7-F	41.3-E
I-215 Freeway NB Ramps (NS) at: SR-74 (EW) ⁴	CSS	0	0	0	0	1	0	1	2	0	0	2	1>>	57.8-F	40.0-E
Beaumont Avenue (SR-79) (NS) at: I-10 Freeway WB Ramps (EW)	TS	1	2	0	0	2	0	0	0	0	0.5	1	0.5	31.4-C	43.9-D
I-10 Freeway EB Ramps (EW)	TS	0	2	0	1	2	0	0.5	1	0.5	0	0	0	27.3-C	32.8-C

¹ When a right turn lane is designated, the lane can either be striped or unstriped. To function as a right turn lane there must be sufficient width for right turning vehicles to travel outside the through lanes.

L = Left; T = Through; R = Right; >> = Free Right Turn

² Delay and level of service has been calculated using the following analysis software: Traffix, Version 7.9.0215 (2008). Per the 2000 Highway Capacity Manual, overall average intersection delay and level of service are shown for intersections with traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown.

³ CSS = Cross Street Stop; TS = Traffic Signal

⁴ A traffic signal is warranted for Existing traffic conditions.

Table 2

**Opening Year (2010) Without Project - No Action
Intersection Delay and Level of Service**

Intersection	Traffic Control ³	Intersection Approach Lanes ¹												Peak Hour Delay (Secs.) - LOS ²	
		Northbound			Southbound			Eastbound			Westbound			Morning	Evening
		L	T	R	L	T	R	L	T	R	L	T	R		
I-215 Freeway SB Ramps (NS) at: Bonnie Drive (EW)															
- Without Improvements	CSS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	81.9-F	56.5-F
- With Improvements	TS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	16.5-B	14.4-B
I-215 Freeway NB Ramps (NS) at: SR-74 (EW)															
- Without Improvements	CSS	0	0	0	0	1	0	1	2	0	0	2	1>>	99.9-F ⁴	73.5-F
- With Improvements	TS	0	0	0	0	1	0	1	2	0	0	2	1>>	9.2-A	9.6-A
Beaumont Avenue (SR-79) (NS) at: I-10 Freeway WB Ramps (EW)	TS	1	2	0	0	2	0	0	0	0	0.5	1	0.5	35.1-D	53.7-D
I-10 Freeway EB Ramps (EW)	TS	0	2	0	1	2	0	0.5	1	0.5	0	0	0	31.0-C	38.4-D

¹ When a right turn lane is designated, the lane can either be striped or unstriped. To function as a right turn lane there must be sufficient width for right turning vehicles to travel outside the through lanes.

L = Left; T = Through; R = Right; >> = Free Right Turn; 1 = Improvement

² Delay and level of service has been calculated using the following analysis software: Traffix, Version 7.9.0215 (2008). Per the 2000 Highway Capacity Manual, overall average intersection delay and level of service are shown for intersections with traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown.

³ CSS = Cross Street Stop; TS = Traffic Signal

⁴ 99.9-F = Delay High, Intersection Unstable, Level of Service F.

Table 3

Opening Year (2010) With Project - Proposed Action "A"
Intersection Delay and Level of Service

Intersection	Traffic Control ³	Intersection Approach Lanes ¹												Peak Hour Delay (Secs.) - LOS ²	
		Northbound			Southbound			Eastbound			Westbound			Morning	Evening
		L	T	R	L	T	R	L	T	R	L	T	R		
I-215 Freeway SB Ramps (NS) at: Bonnie Drive (EW)															
- Without Improvements	CSS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	87.2-F	65.2-F
- With Improvements	TS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	17.2-B	16.2-B
I-215 Freeway NB Ramps (NS) at: SR-74 (EW)															
- Without Improvements	CSS	0	0	0	0	1	0	1	2	0	0	2	1>>	99.9-F ⁴	99.9-F
- With Improvements	TS	0	0	0	0	1	0	1	2	0	0	2	1>>	11.5-B	12.8-B
Beaumont Avenue (SR-79) (NS) at: I-10 Freeway WB Ramps (EW)															
- Without Improvements	TS	1	2	0	0	2	0	0	0	0	0.5	1	0.5	40.0-D	99.9-F
- With Improvements	TS	1	2	0	0	2	0	0	0	0	1.5	1	0.5	28.4-C	33.0-C
I-10 Freeway EB Ramps (EW)	TS	0	2	0	1	2	0	0.5	1	0.5	0	0	0	38.3-D	54.9-D

¹ When a right turn lane is designated, the lane can either be striped or unstriped. To function as a right turn lane there must be sufficient width for right turning vehicles to travel outside the through lanes.

L = Left; T = Through; R = Right; >> = Free Right Turn; **1** = Improvement

² Delay and level of service has been calculated using the following analysis software: Traffix, Version 7.9.0215 (2008). Per the 2000 Highway Capacity Manual, overall average intersection delay and level of service are shown for intersections with traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown.

³ CSS = Cross Street Stop; TS = Traffic Signal

⁴ 99.9-F = Delay High, Intersection Unstable, Level of Service F.

Table 4

**Opening Year (2010) With Project - Proposed Action "B"
Intersection Delay and Level of Service**

Intersection	Traffic Control ³	Intersection Approach Lanes ¹												Peak Hour Delay (Secs.) - LOS ²	
		Northbound			Southbound			Eastbound			Westbound			Morning	Evening
		L	T	R	L	T	R	L	T	R	L	T	R		
I-215 Freeway SB Ramps (NS) at: Bonnie Drive (EW)															
- Without Improvements	CSS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	87.0-F	64.8-F
- With Improvements	TS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	17.2-B	16.2-B
I-215 Freeway NB Ramps (NS) at: SR-74 (EW)															
- Without Improvements	CSS	0	0	0	0	1	0	1	2	0	0	2	1>>	99.9-F ⁴	99.9-F
- With Improvements	TS	0	0	0	0	1	0	1	2	0	0	2	1>>	11.4-B	12.7-B
Beaumont Avenue (SR-79) (NS) at: I-10 Freeway WB Ramps (EW)															
- Without Improvements	TS	1	2	0	0	2	0	0	0	0	0.5	1	0.5	39.8-D	99.9-F
- With Improvements	TS	1	2	0	0	2	0	0	0	0	1.5	1	0.5	28.3-C	32.9-C
I-10 Freeway EB Ramps (EW)	TS	0	2	0	1	2	0	0.5	1	0.5	0	0	0	38.0-D	54.2-D

¹ When a right turn lane is designated, the lane can either be striped or unstriped. To function as a right turn lane there must be sufficient width for right turning vehicles to travel outside the through lanes.

L = Left; T = Through; R = Right; >> = Free Right Turn; 1 = Improvement

² Delay and level of service has been calculated using the following analysis software: Traffix, Version 7.9.0215 (2008). Per the 2000 Highway Capacity Manual, overall average intersection delay and level of service are shown for intersections with traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown.

³ CSS = Cross Street Stop; TS = Traffic Signal

⁴ 99.9-F = Delay High, Intersection Unstable, Level of Service F.

Table 5

**Opening Year (2010) With Project - Alternative 1
Intersection Delay and Level of Service**

Intersection	Traffic Control ³	Intersection Approach Lanes ¹												Peak Hour Delay (Secs.) - LOS ²	
		Northbound			Southbound			Eastbound			Westbound			Morning	Evening
		L	T	R	L	T	R	L	T	R	L	T	R		
I-215 Freeway SB Ramps (NS) at: Bonnie Drive (EW)															
- Without Improvements	CSS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	85.7-F	62.8-F
- With Improvements	TS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	16.8-B	15.8-B
I-215 Freeway NB Ramps (NS) at: SR-74 (EW)															
- Without Improvements	CSS	0	0	0	0	1	0	1	2	0	0	2	1>>	99.9-F ⁴	99.9-F
- With Improvements	TS	0	0	0	0	1	0	1	2	0	0	2	1>>	10.9-B	12.0-B
Beaumont Avenue (SR-79) (NS) at: I-10 Freeway WB Ramps (EW)															
- Without Improvements	TS	1	2	0	0	2	0	0	0	0	0.5	1	0.5	38.5-D	99.9-F
- With Improvements	TS	1	2	0	0	2	0	0	0	0	1.5	1	0.5	27.8-C	31.6-D
I-10 Freeway EB Ramps (EW)	TS	0	2	0	1	2	0	0.5	1	0.5	0	0	0	36.1-D	49.5-D

¹ When a right turn lane is designated, the lane can either be striped or unstriped. To function as a right turn lane there must be sufficient width for right turning vehicles to travel outside the through lanes.

L = Left; T = Through; R = Right; >> = Free Right Turn; 1 = Improvement

² Delay and level of service has been calculated using the following analysis software: Traffix, Version 7.9.0215 (2008). Per the 2000 Highway Capacity Manual, overall average intersection delay and level of service are shown for intersections with traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown.

³ CSS = Cross Street Stop; TS = Traffic Signal

⁴ 99.9-F = Delay High, Intersection Unstable, Level of Service F.

Table 6

**Opening Year (2010) With Project - Alternative 2
Intersection Delay and Level of Service**

Intersection	Traffic Control ³	Intersection Approach Lanes ¹												Peak Hour Delay (Secs.) - LOS ²	
		Northbound			Southbound			Eastbound			Westbound			Morning	Evening
		L	T	R	L	T	R	L	T	R	L	T	R		
I-215 Freeway SB Ramps (NS) at: Bonnie Drive (EW)															
- Without Improvements	CSS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	83.7-F	58.5-F
- With Improvements	TS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	16.6-B	14.5-B
I-215 Freeway NB Ramps (NS) at: SR-74 (EW)															
- Without Improvements	CSS	0	0	0	0	1	0	1	2	0	0	2	1>>	99.9-F ⁴	99.9-F
- With Improvements	TS	0	0	0	0	1	0	1	2	0	0	2	1>>	9.9-F	10.1-B
Beaumont Avenue (SR-79) (NS) at: I-10 Freeway WB Ramps (EW)															
- Without Improvements	TS	1	2	0	0	2	0	0	0	0	0.5	1	0.5	36.6-D	57.9-E
- With Improvements	TS	1	2	0	0	2	0	0	0	0	1.5	1	0.5	26.9-C	29.0-C
I-10 Freeway EB Ramps (EW)	TS	0	2	0	1	2	0	0.5	1	0.5	0	0	0	33.0-C	40.8-D

¹ When a right turn lane is designated, the lane can either be striped or unstriped. To function as a right turn lane there must be sufficient width for right turning vehicles to travel outside the through lanes.

L = Left; T = Through; R = Right; >> = Free Right Turn; **1** = Improvement

² Delay and level of service has been calculated using the following analysis software: Traffix, Version 7.9.0215 (2008). Per the 2000 Highway Capacity Manual, overall average intersection delay and level of service are shown for intersections with traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown.

³ CSS = Cross Street Stop; TS = Traffic Signal

⁴ 99.9-F = Delay High, Intersection Unstable, Level of Service F.

Table 7

**Opening Year (2010) With Project - Alternative 3
Intersection Delay and Level of Service**

Intersection	Traffic Control ³	Intersection Approach Lanes ¹												Peak Hour Delay (Secs.) - LOS ²	
		Northbound			Southbound			Eastbound			Westbound			Morning	Evening
		L	T	R	L	T	R	L	T	R	L	T	R		
I-215 Freeway SB Ramps (NS) at: Bonnie Drive (EW)															
- Without Improvements	CSS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	84.2-F	60.8-F
- With Improvements	TS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	16.6-B	14.8-B
I-215 Freeway NB Ramps (NS) at: SR-74 (EW)															
- Without Improvements	CSS	0	0	0	0	1	0	1	2	0	0	2	1>>	99.9-F ⁴	99.9-F
- With Improvements	TS	0	0	0	0	1	0	1	2	0	0	2	1>>	9.8-A	11.1-B
Beaumont Avenue (SR-79) (NS) at: I-10 Freeway WB Ramps (EW)															
- Without Improvements	TS	1	2	0	0	2	0	0	0	0	0.5	1	0.5	36.4-D	99.9-F
- With Improvements	TS	1	2	0	0	2	0	0	0	0	1.5	1	0.5	26.9-C	30.3-C
I-10 Freeway EB Ramps (EW)	TS	0	2	0	1	2	0	0.5	1	0.5	0	0	0	32.8-C	44.8-D

¹ When a right turn lane is designated, the lane can either be striped or unstriped. To function as a right turn lane there must be sufficient width for right turning vehicles to travel outside the through lanes.

L = Left; T = Through; R = Right; >> = Free Right Turn; 1 = Improvement

² Delay and level of service has been calculated using the following analysis software: Traffix, Version 7.9.0125 (2008). Per the 2000 Highway Capacity Manual, overall average intersection delay and level of service are shown for intersections with traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown.

³ CSS = Cross Street Stop; TS = Traffic Signal

⁴ 99.9-F = Delay High, Intersection Unstable, Level of Service F.

Table 8

**Year 2025 Without Project - No Action
Intersection Delay and Level of Service**

Intersection	Traffic Control ³	Intersection Approach Lanes ¹												Peak Hour	
		Northbound			Southbound			Eastbound			Westbound			Delay (Secs.) - LOS ²	
		L	T	R	L	T	R	L	T	R	L	T	R	Morning	Evening
I-215 Freeway SB Ramps (NS) at:															
Bonnie Drive (EW)															
- Without Improvements															
	CSS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	99.9-F ⁴	99.9-F
- With Improvements															
	TS	<u>2</u>	1	0	0	<u>2</u>	1>>	1	0	1>>	0	0	0	19.6-B	17.0-B
I-215 Freeway NB Ramps (NS) at:															
SR-74 (EW)															
- Without Improvements															
	CSS	0	0	0	0	1	0	1	2	0	0	2	1>>	99.9-F	99.9-F
- With Improvements															
	TS	0	0	0	<u>1</u>	0	<u>1</u>	1	2	0	0	2	1>>	13.5-B	30.8-C
Beaumont Avenue (SR-79) (NS) at:															
I-10 Freeway WB Ramps (EW)															
- Without Improvements															
	TS	1	2	0	0	2	0	0	0	0	0.5	1	0.5	99.9-F	99.9-F
- With Improvements															
	TS	2	2	0	0	3	0	0	0	0	<u>1.5</u>	1	0.5	26.5-C	37.7-D
I-10 Freeway EB Ramps (EW)															
- Without Improvements															
	TS	0	2	0	1	2	0	0.5	1	0.5	0	0	0	99.9-F	99.9-F
- With Improvements															
	TS	0	1.5	<u>1.5</u>	<u>2</u>	2	0	<u>1</u>	0.5	<u>1.5</u>	0	0	0	29.0-C	36.4-D

¹ When a right turn lane is designated, the lane can either be striped or unstriped. To function as a right turn lane there must be sufficient width for right turning vehicles to travel outside the through lanes.

L = Left; T = Through; R = Right; >> = Free Right Turn; 1 = Improvement

² Delay and level of service has been calculated using the following analysis software: Traffix, Version 7.9.0215 (2008). Per the 2000 Highway Capacity Manual, overall average intersection delay and level of service are shown for intersections with traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown.

³ CSS = Cross Street Stop; TS = Traffic Signal

⁴ 99.9-F = Delay High, Intersection Unstable, Level of Service F.

Table 9

**Year 2025 With Project - Proposed Action "A"
Intersection Delay and Level of Service**

Intersection	Traffic Control ³	Intersection Approach Lanes ¹												Peak Hour	
		Northbound			Southbound			Eastbound			Westbound			Delay (Secs.) - LOS ²	
		L	T	R	L	T	R	L	T	R	L	T	R	Morning	Evening
I-215 Freeway SB Ramps (NS) at:															
Bonnie Drive (EW)															
- Without Improvements	CSS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	99.9-F ⁴	99.9-F
- With Improvements	TS	2	1	0	0	2	1>>	1	0	1>>	0	0	0	21.2-C	18.3-B
I-215 Freeway NB Ramps (NS) at:															
SR-74 (EW)															
- Without Improvements	CSS	0	0	0	0	1	0	1	2	0	0	2	1>>	99.9-F	99.9-F
- With Improvements	TS	0	0	0	2	0	1	1	2	0	0	2	1>>	9.3-A	18.1-B
Beaumont Avenue (SR-79) (NS) at:															
I-10 Freeway WB Ramps (EW)															
- Without Improvements	TS	1	2	0	0	2	0	0	0	0	0.5	1	0.5	99.9-F	99.9-F
- With Improvements	TS	2	2	0	0	3	0	0	0	0	1.5	1	0.5	27.7-C	43.8-D
I-10 Freeway EB Ramps (EW)															
- Without Improvements	TS	0	2	0	1	2	0	0.5	1	0.5	0	0	0	99.9-F	99.9-F
- With Improvements	TS	0	1.5	1.5	2	2	0	1	0.5	1.5	0	0	0	32.3-C	43.4-D

¹ When a right turn lane is designated, the lane can either be striped or unstriped. To function as a right turn lane there must be sufficient width for right turning vehicles to travel outside the through lanes.

L = Left; T = Through; R = Right; >> = Free Right Turn; **1** = Improvement

² Delay and level of service has been calculated using the following analysis software: Traffix, Version 7.9.0215 (2008). Per the 2000 Highway Capacity Manual, overall average intersection delay and level of service are shown for intersections with traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown.

³ CSS = Cross Street Stop; TS = Traffic Signal

⁴ 99.9-F = Delay High, Intersection Unstable, Level of Service F.

Table 10

**Year 2025 With Project - Proposed Action "B"
Intersection Delay and Level of Service**

Intersection	Traffic Control ³	Intersection Approach Lanes ¹												Peak Hour Delay (Secs.) - LOS ²	
		Northbound			Southbound			Eastbound			Westbound			Morning	Evening
		L	T	R	L	T	R	L	T	R	L	T	R		
I-215 Freeway SB Ramps (NS) at: Bonnie Drive (EW)															
- Without Improvements	CSS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	99.9-F ⁴	99.9-F
- With Improvements	TS	2	1	0	0	2	1>>	1	0	1>>	0	0	0	21.2-C	18.0-B
I-215 Freeway NB Ramps (NS) at: SR-74 (EW)															
- Without Improvements	CSS	0	0	0	0	1	0	1	2	0	0	2	1>>	99.9-F	99.9-F
- With Improvements	TS	0	0	0	2	0	1	1	2	0	0	2	1>>	9.2-A	18.0-B
Beaumont Avenue (SR-79) (NS) at: I-10 Freeway WB Ramps (EW)															
- Without Improvements	TS	1	2	0	0	2	0	0	0	0	0.5	1	0.5	99.9-F	99.9-F
- With Improvements	TS	2	2	0	0	3	0	0	0	0	1.5	1	0.5	27.7-C	43.5-D
I-10 Freeway EB Ramps (EW)															
- Without Improvements	TS	0	2	0	1	2	0	0.5	1	0.5	0	0	0	99.9-F	99.9-F
- With Improvements	TS	0	1.5	1.5	2	2	0	1	0.5	1.5	0	0	0	32.2-C	43.1-D

¹ When a right turn lane is designated, the lane can either be striped or unstriped. To function as a right turn lane there must be sufficient width for right turning vehicles to travel outside the through lanes.

L = Left; T = Through; R = Right; >> = Free Right Turn; **1** = Improvement

² Delay and level of service has been calculated using the following analysis software: Traffix, Version 7.9.0215 (2008). Per the 2000 Highway Capacity Manual, overall average intersection delay and level of service are shown for intersections with traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown.

³ CSS = Cross Street Stop; TS = Traffic Signal

⁴ 99.9-F = Delay High, Intersection Unstable, Level of Service F.

Table 11

**Year 2025 With Project - Alternative 1
Intersection Delay and Level of Service**

Intersection	Traffic Control ³	Intersection Approach Lanes ¹												Peak Hour Delay (Secs.) - LOS ²	
		Northbound			Southbound			Eastbound			Westbound			Morning	Evening
		L	T	R	L	T	R	L	T	R	L	T	R		
I-215 Freeway SB Ramps (NS) at: Bonnie Drive (EW)															
- Without Improvements	CSS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	99.9-F ⁴	99.9-F
- With Improvements	TS	<u>2</u>	1	0	0	<u>2</u>	1>>	1	0	1>>	0	0	0	20.6-C	17.7-B
I-215 Freeway NB Ramps (NS) at: SR-74 (EW)															
- Without Improvements	CSS	0	0	0	0	1	0	1	2	0	0	2	1>>	99.9-F	99.9-F
- With Improvements	TS	0	0	0	<u>2</u>	0	<u>1</u>	1	2	0	0	2	1>>	8.9-A	17.1-B
Beaumont Avenue (SR-79) (NS) at: I-10 Freeway WB Ramps (EW)															
- Without Improvements	TS	1	2	0	0	2	0	0	0	0	0.5	1	0.5	99.9-F	99.9-F
- With Improvements	TS	2	2	0	0	3	0	0	0	0	<u>1.5</u>	1	0.5	27.4-C	42.0-D
I-10 Freeway EB Ramps (EW)															
- Without Improvements	TS	0	2	0	1	2	0	0.5	1	0.5	0	0	0	99.9-F	99.9-F
- With Improvements	TS	0	1.5	<u>1.5</u>	<u>2</u>	2	0	<u>1</u>	0.5	<u>1.5</u>	0	0	0	31.4-C	41.3-D

¹ When a right turn lane is designated, the lane can either be striped or unstriped. To function as a right turn lane there must be sufficient width for right turning vehicles to travel outside the through lanes.

L = Left; T = Through; R = Right; >> = Free Right Turn; 1 = Improvement

² Delay and level of service has been calculated using the following analysis software: Traffix, Version 7.9.0215 (2008). Per the 2000 Highway Capacity Manual, overall average intersection delay and level of service are shown for intersections with traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown.

³ CSS = Cross Street Stop; TS = Traffic Signal

⁴ 99.9-F = Delay High, Intersection Unstable, Level of Service F.

Table 12

**Year 2025 With Project - Alternative 2
Intersection Delay and Level of Service**

Intersection	Traffic Control ³	Intersection Approach Lanes ¹												Peak Hour Delay (Secs.) - LOS ²	
		Northbound			Southbound			Eastbound			Westbound			Morning	Evening
		L	T	R	L	T	R	L	T	R	L	T	R		
I-215 Freeway SB Ramps (NS) at: Bonnie Drive (EW)															
- Without Improvements	CSS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	99.9-F ⁴	99.9-F
- With Improvements	TS	2	1	0	0	2	1>>	1	0	1>>	0	0	0	20.0-B	17.1-B
I-215 Freeway NB Ramps (NS) at: SR-74 (EW)															
- Without Improvements	CSS	0	0	0	0	1	0	1	2	0	0	2	1>>	99.9-F	99.9-F
- With Improvements	TS	0	0	0	1	0	1	1	2	0	0	2	1>>	14.8-B	32.8-C
Beaumont Avenue (SR-79) (NS) at: I-10 Freeway WB Ramps (EW)															
- Without Improvements	TS	1	2	0	0	2	0	0	0	0	0.5	1	0.5	99.9-F	99.9-F
- With Improvements	TS	2	2	0	0	3	0	0	0	0	1.5	1	0.5	26.9-C	38.8-D
I-10 Freeway EB Ramps (EW)															
- Without Improvements	TS	0	2	0	1	2	0	0.5	1	0.5	0	0	0	99.9-F	99.9-F
- With Improvements	TS	0	1.5	1.5	2	2	0	1	0.5	1.5	0	0	0	30.1-C	37.6-D

¹ When a right turn lane is designated, the lane can either be striped or unstriped. To function as a right turn lane there must be sufficient width for right turning vehicles to travel outside the through lanes.

L = Left; T = Through; R = Right; >> = Free Right Turn; **1** = Improvement

² Delay and level of service has been calculated using the following analysis software: Traffix, Version 7.9.0215 (2008). Per the 2000 Highway Capacity Manual, overall average intersection delay and level of service are shown for intersections with traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown.

³ CSS = Cross Street Stop; TS = Traffic Signal

⁴ 99.9-F = Delay High, Intersection Unstable, Level of Service F.

Table 13

Year 2025 With Project - Alternative 3
Intersection Delay and Level of Service

Intersection	Traffic Control ³	Intersection Approach Lanes ¹												Peak Hour Delay (Secs.) - LOS ²	
		Northbound			Southbound			Eastbound			Westbound			Morning	Evening
		L	T	R	L	T	R	L	T	R	L	T	R		
I-215 Freeway SB Ramps (NS) at: Bonnie Drive (EW)															
- Without Improvements	CSS	1	1	0	0	1	1>>	1	0	1>>	0	0	0	99.9-F ⁴	99.9-F
- With Improvements	TS	2	1	0	0	2	1>>	1	0	1>>	0	0	0	19.9-B	17.5-B
I-215 Freeway NB Ramps (NS) at: SR-74 (EW)															
- Without Improvements	CSS	0	0	0	0	1	0	1	2	0	0	2	1>>	99.9-F	99.9-F
- With Improvements	TS	0	0	0	2	0	1	1	2	0	0	2	1>>	8.3-A	16.1-B
Beaumont Avenue (SR-79) (NS) at: I-10 Freeway WB Ramps (EW)															
- Without Improvements	TS	1	2	0	0	2	0	0	0	0	0.5	1	0.5	99.9-F	99.9-F
- With Improvements	TS	2	2	0	0	3	0	0	0	0	1.5	1	0.5	26.9-C	40.3-D
I-10 Freeway EB Ramps (EW)															
- Without Improvements	TS	0	2	0	1	2	0	0.5	1	0.5	0	0	0	99.9-F	99.9-F
- With Improvements	TS	0	1.5	1.5	2	2	0	1	0.5	1.5	0	0	0	29.9-C	39.9-D

¹ When a right turn lane is designated, the lane can either be striped or unstriped. To function as a right turn lane there must be sufficient width for right turning vehicles to travel outside the through lanes.

L = Left; T = Through; R = Right; >> = Free Right Turn; **1** = Improvement

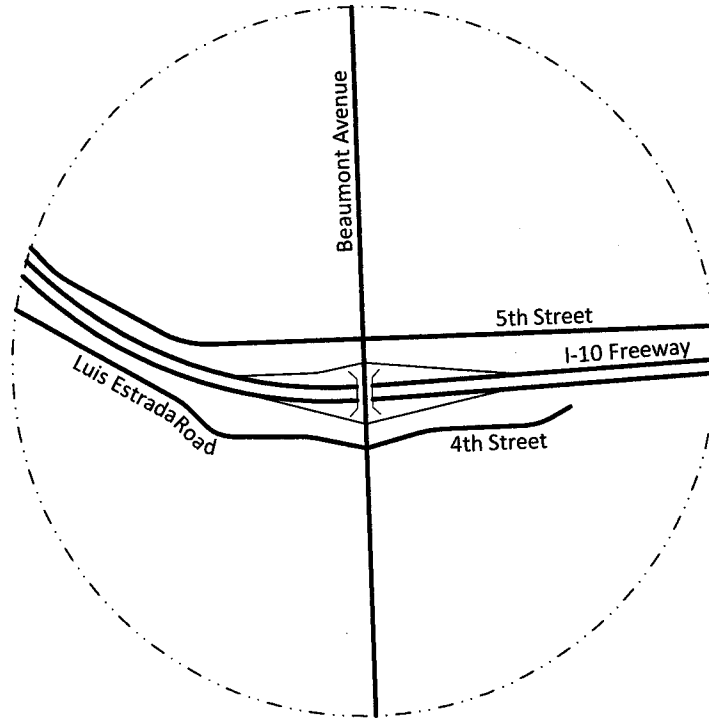
² Delay and level of service has been calculated using the following analysis software: Traffix, Version 7.9.0215 (2008). Per the 2000 Highway Capacity Manual, overall average intersection delay and level of service are shown for intersections with traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown.

³ CSS = Cross Street Stop; TS = Traffic Signal

⁴ 99.9-F = Delay High, Intersection Unstable, Level of Service F.

Figure 1
Freeway Analysis Location Map

I-10 Freeway and Beaumont Avenue (SR-79) Interchange



I-215 Freeway and Bonnie Drive/SR-74 Interchange

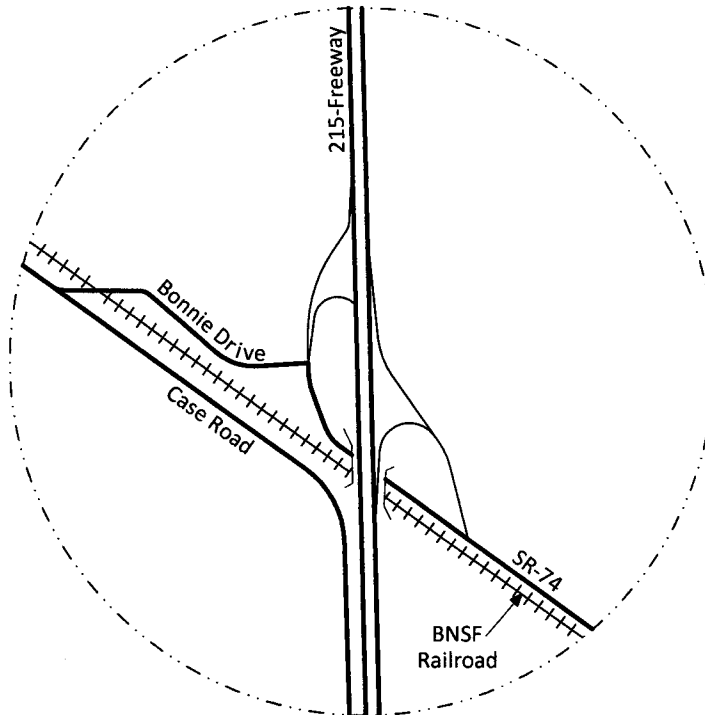
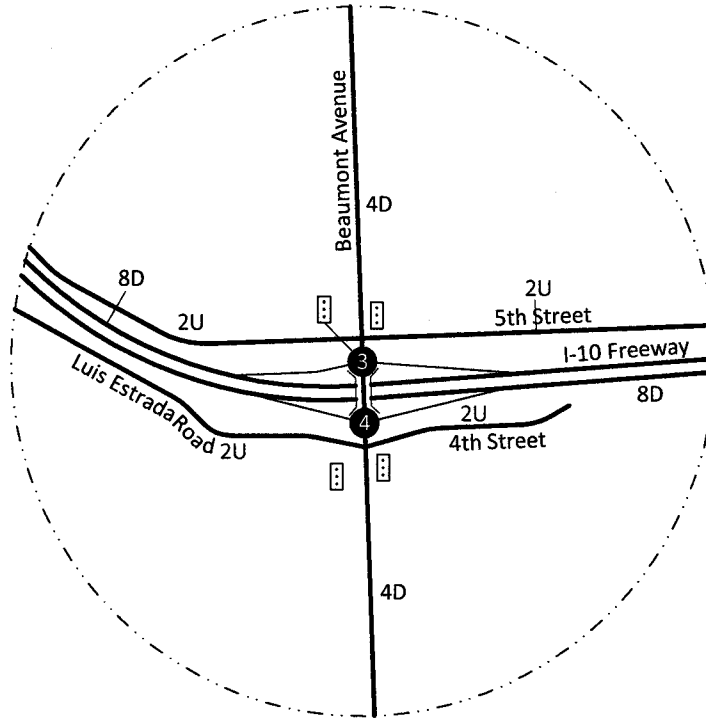
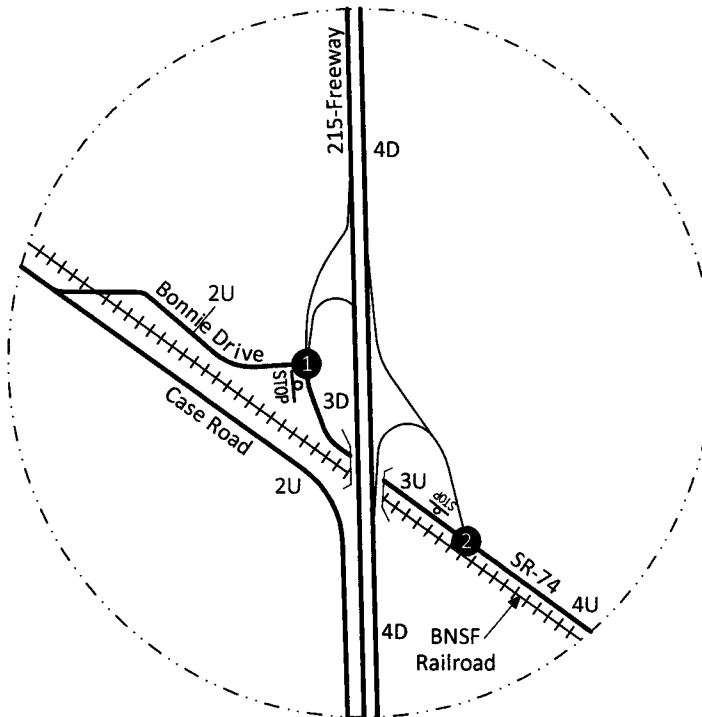


Figure 2
Existing Through Travel Lanes and Intersection Controls

I-10 Freeway and Beaumont Avenue (SR-79) Interchange



I-215 Freeway and Bonnie Drive/SR-74 Interchange



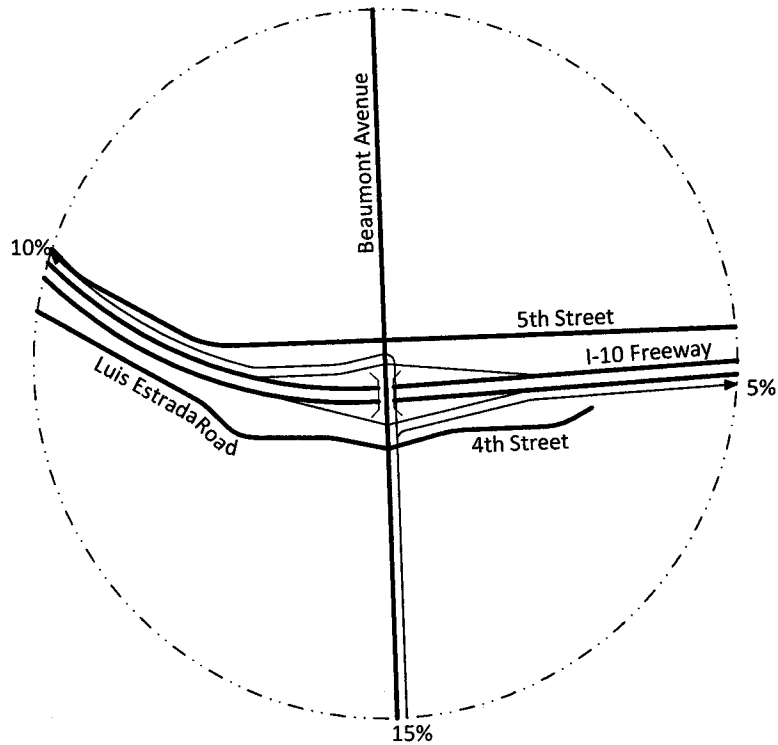
Legend

- = Traffic Signal
- = Stop Sign
- 4 = Through Travel Lanes
- D = Divided
- U = Undivided

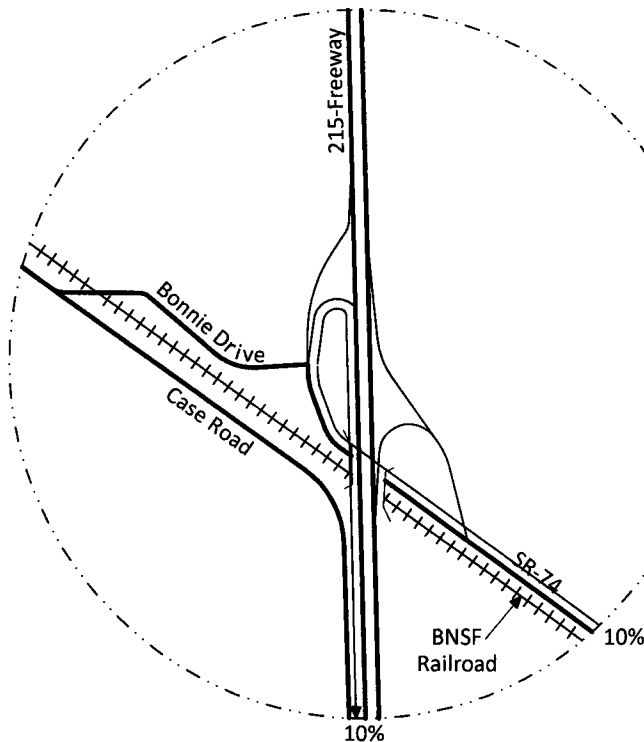
1		2	
3		4	



Figure 3
Proposed Action "A" Project Traffic Distribution
(Casino, Event Arena, Convention Center and Hotel)
I-10 Freeway and Beaumont Avenue (SR-79) Interchange



I-215 Freeway and Bonnie Drive/SR-74 Interchange

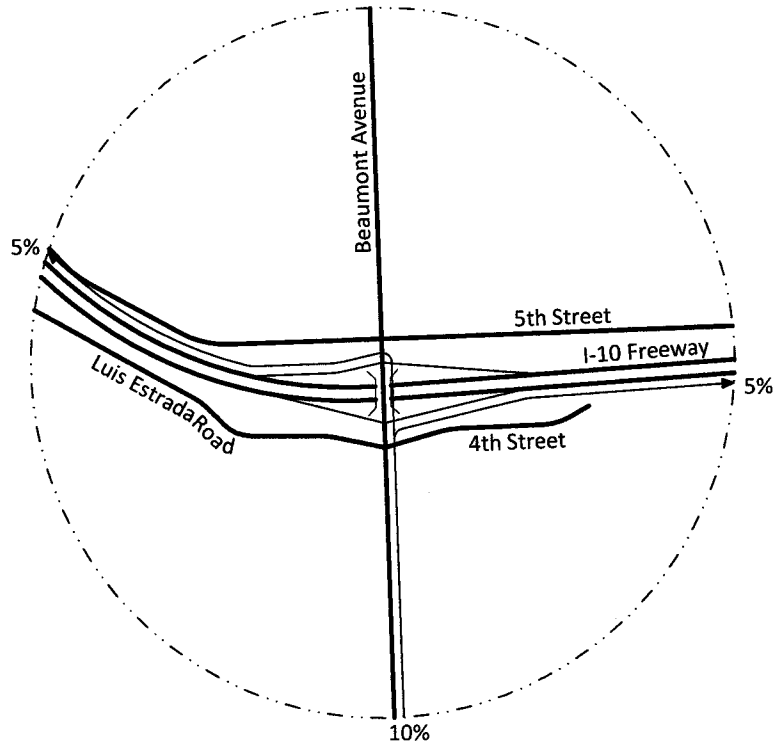


Legend

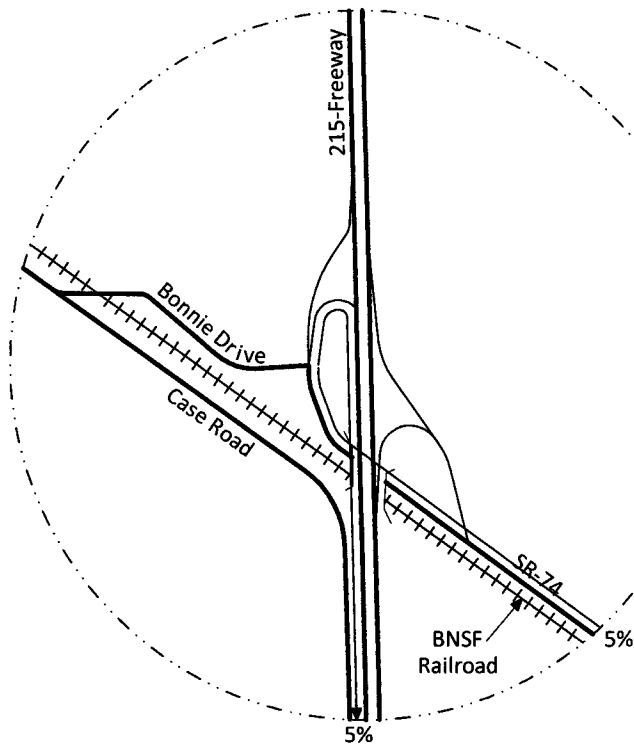
10% = Percent To/From Project



Figure 4
Proposed Action "A" Project Traffic Distribution
(Service Station and Fire Station)
I-10 Freeway and Beaumont Avenue (SR-79) Interchange



I-215 Freeway and Bonnie Drive/SR-74 Interchange

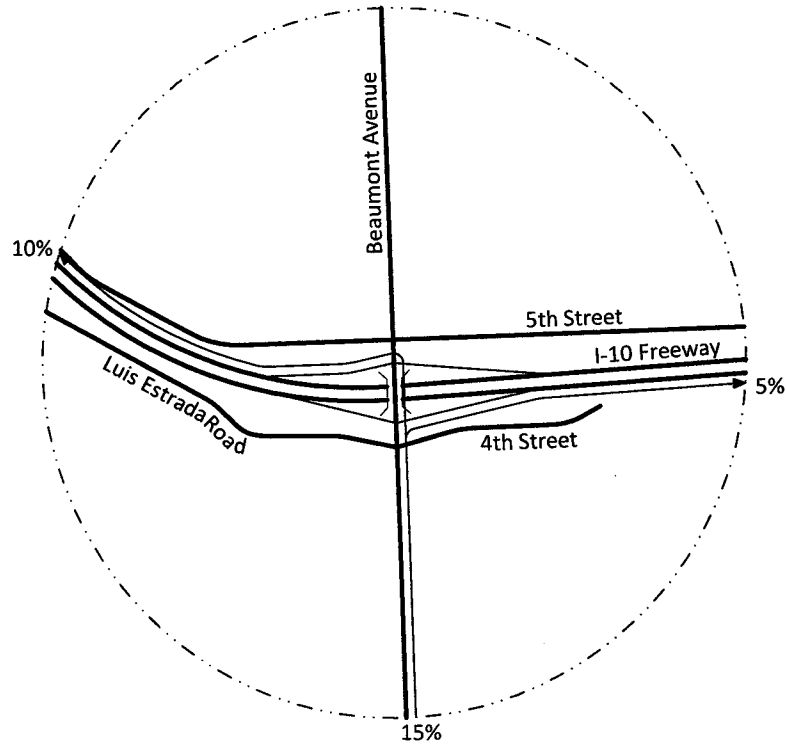


Legend

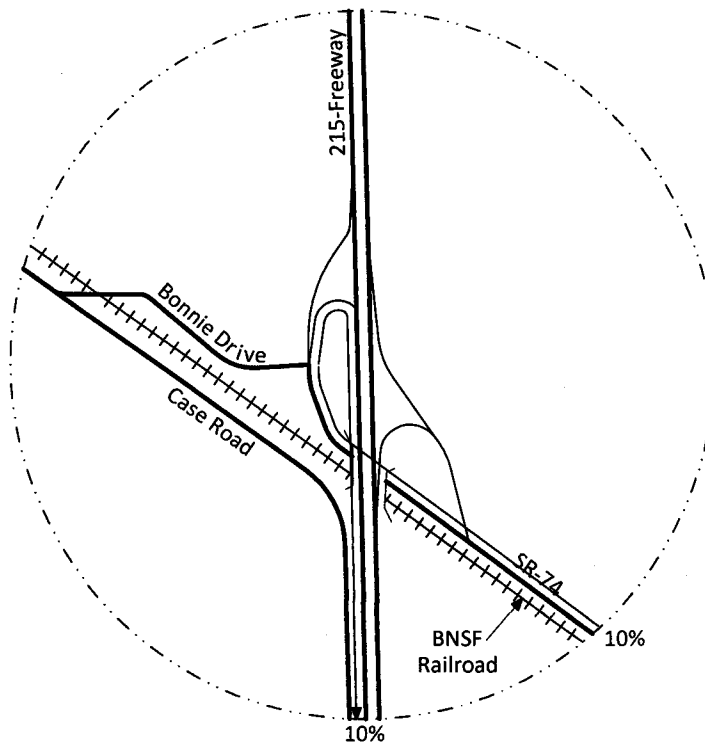
10% = Percent To/From Project



Figure 5
Proposed Action "B" Project Traffic Distribution
(Casino, Event Arena, Convention Center and Hotel)
I-10 Freeway and Beaumont Avenue (SR-79) Interchange



I-215 Freeway and Bonnie Drive/SR-74 Interchange

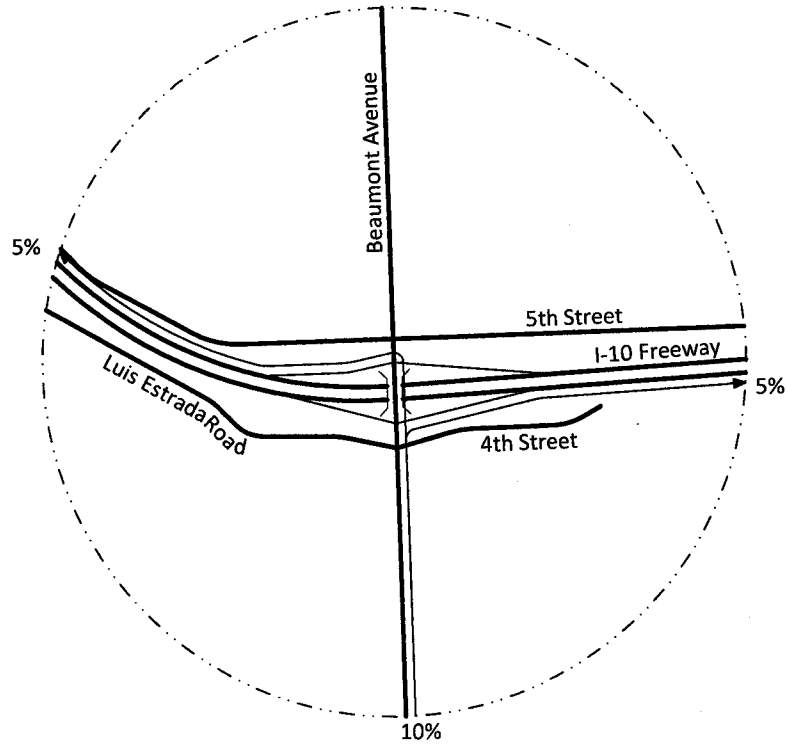


Legend

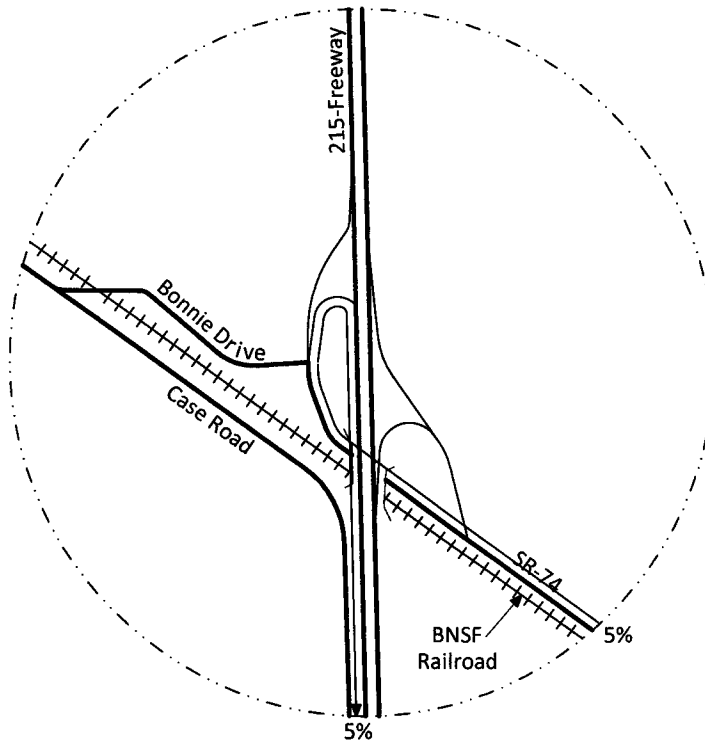
10% = Percent To/From Project



Figure 6
Proposed Action "B" Project Traffic Distribution
(Service Station and Fire Station)
I-10 Freeway and Beaumont Avenue (SR-79) Interchange



I-215 Freeway and Bonnie Drive/SR-74 Interchange

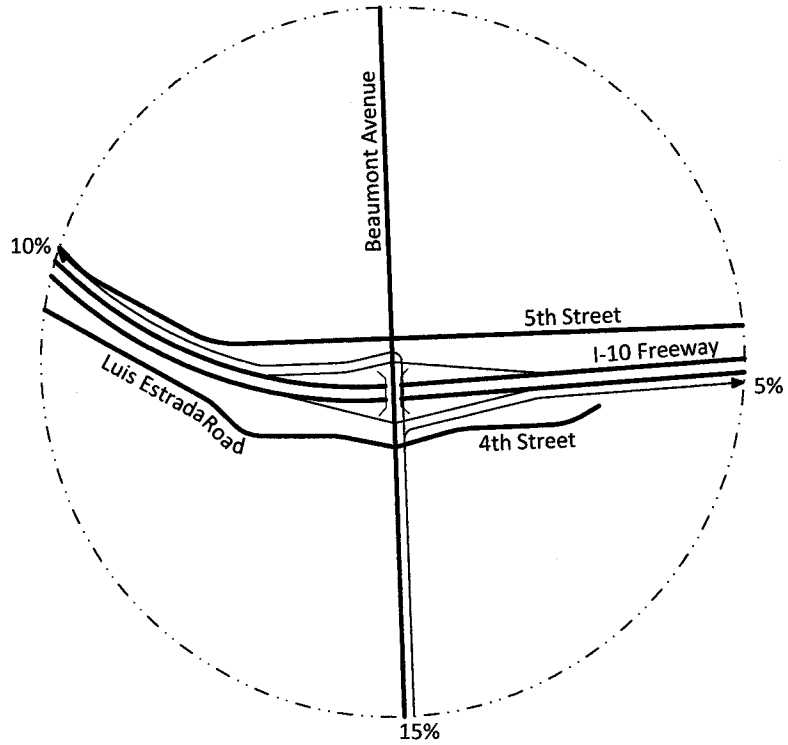


Legend

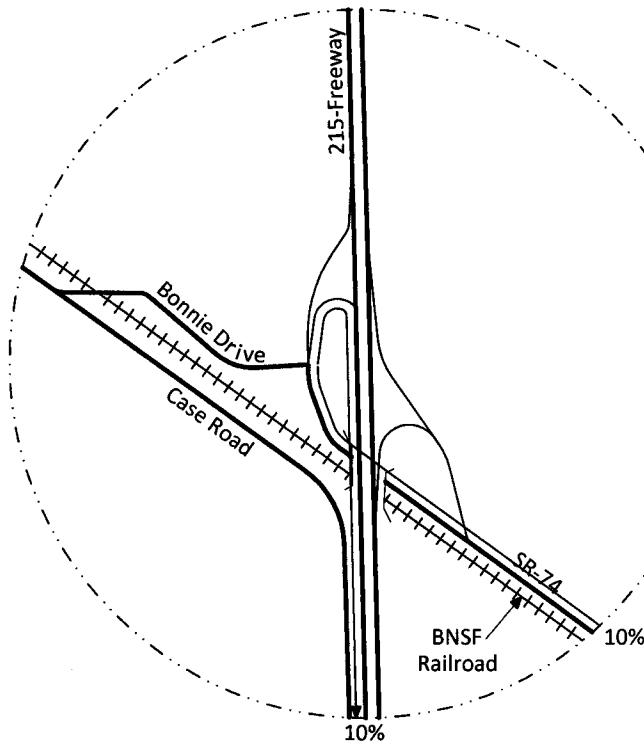
10% = Percent To/From Project



Figure 7
Alternative 1 Project Traffic Distribution
(Casino, Event Arena, Convention Center and Hotel)
I-10 Freeway and Beaumont Avenue (SR-79) Interchange



I-215 Freeway and Bonnie Drive/SR-74 Interchange

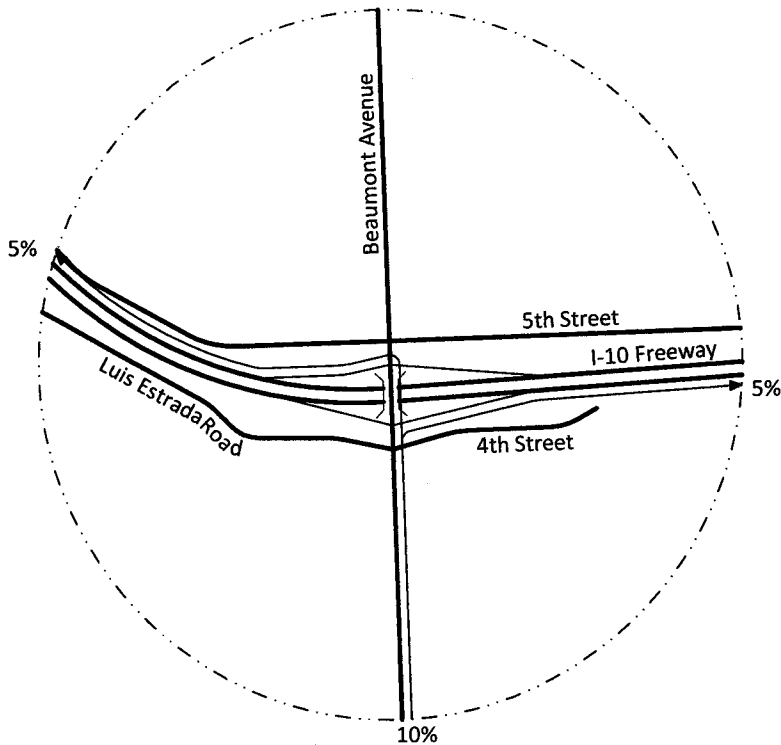


Legend

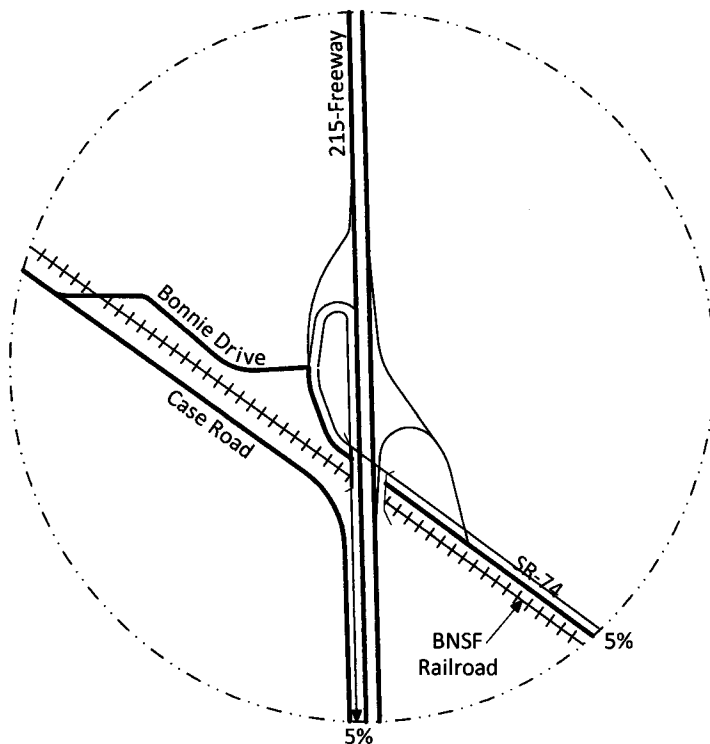
10% = Percent To/From Project



Figure 8
Alternative 1 Project Traffic Distribution
(Service Station and Fire Station)
I-10 Freeway and Beaumont Avenue (SR-79) Interchange



I-215 Freeway and Bonnie Drive/SR-74 Interchange

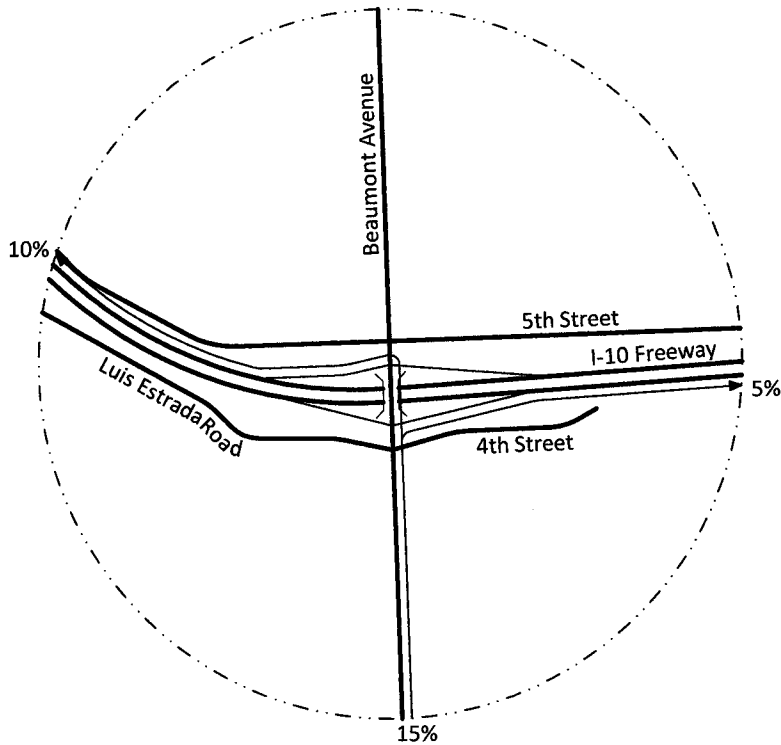


Legend

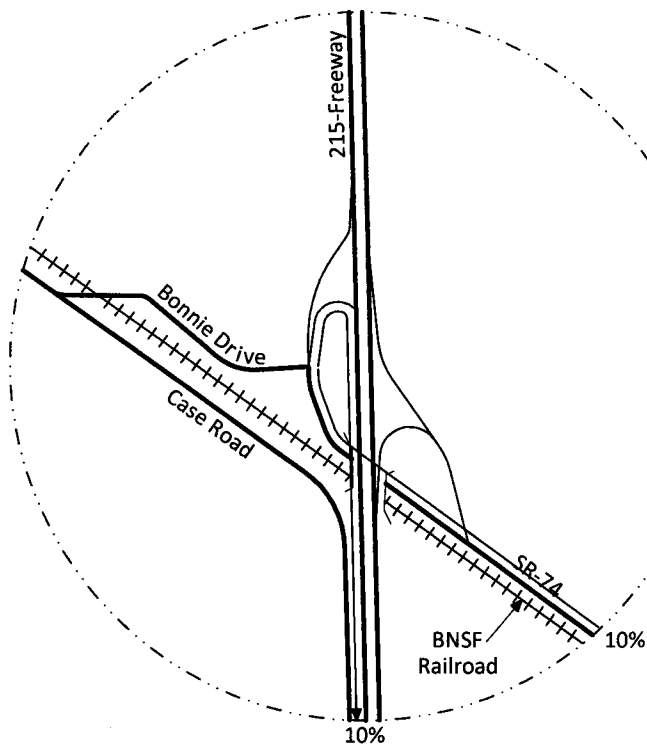
10% = Percent To/From Project



Figure 9
Alternative 2 Project Traffic Distribution
(Hotel and Convention Center)
I-10 Freeway and Beaumont Avenue (SR-79) Interchange



I-215 Freeway and Bonnie Drive/SR-74 Interchange

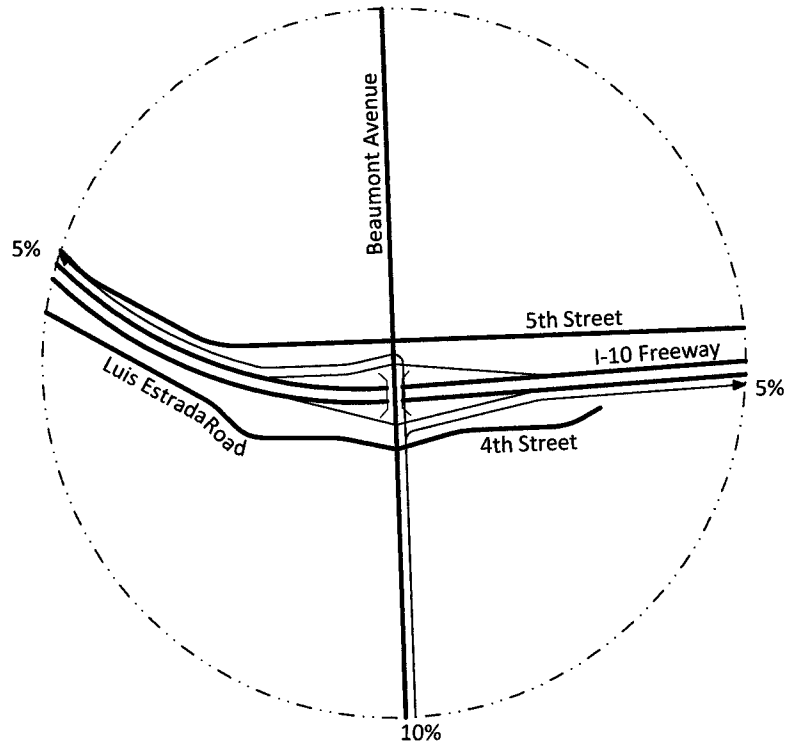


Legend

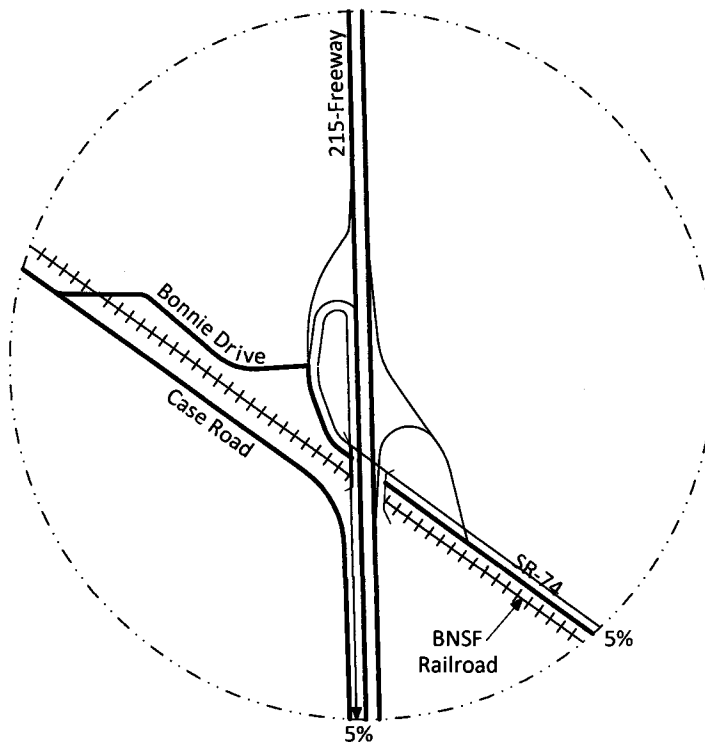
10% = Percent To/From Project



Figure 10
Alternative 2 Project Traffic Distribution
(Service Station and Fire Station)
I-10 Freeway and Beaumont Avenue (SR-79) Interchange



I-215 Freeway and Bonnie Drive/SR-74 Interchange

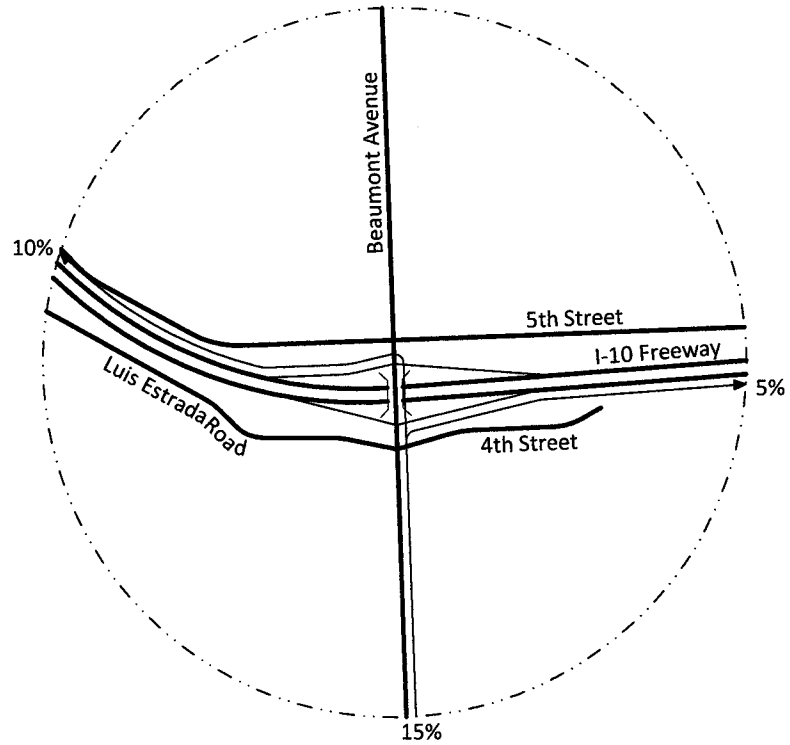


Legend

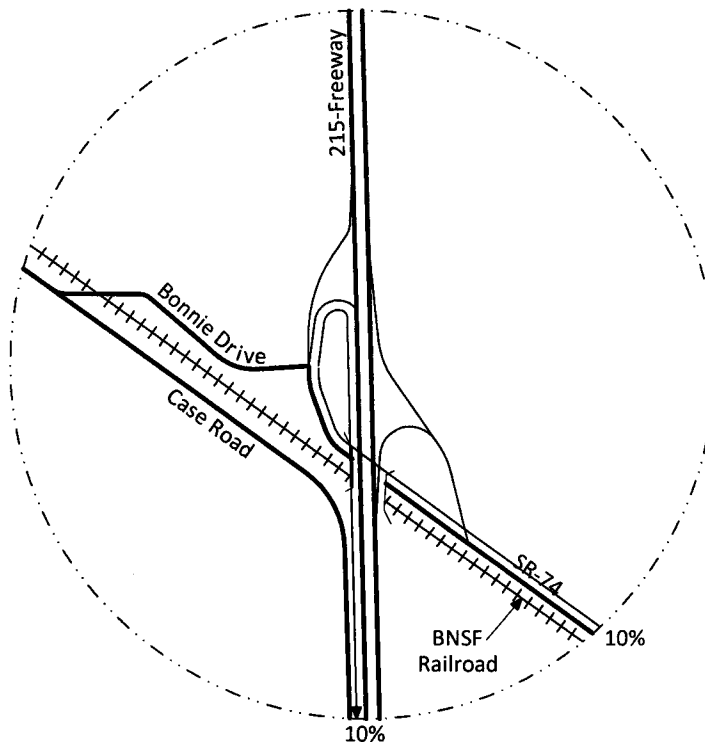
10% = Percent To/From Project



Figure 11
Alternative 3 Project Traffic Distribution
I-10 Freeway and Beaumont Avenue (SR-79) Interchange



I-215 Freeway and Bonnie Drive/SR-74 Interchange



Legend

10% = Percent To/From Project



APPENDIX H

Line of Sight Analysis

Figure 4
Alternative 2 Sight Distance

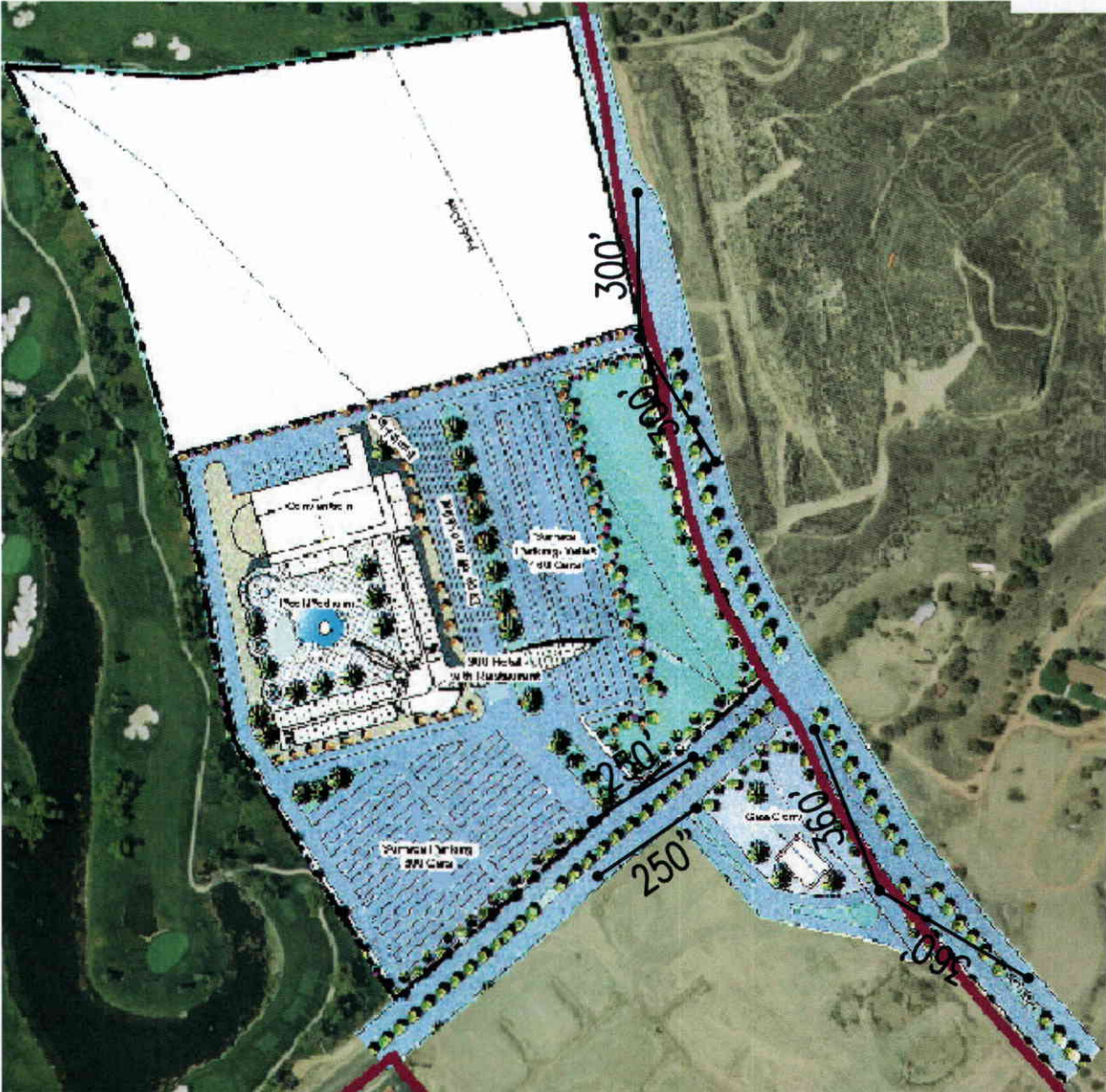


Figure 5
Alternative 3 Sight Distance



Legend	
[Orange Box]	Floodplain
[Red Box]	Gas Station
[Green Box]	Habitat
[Blue Box]	Lake
[Pink Box]	Lake with Private Lot
[Dark Green Box]	Major Road
[Dark Blue Box]	Pool and R/C Area
[Light Green Box]	Recreation Area
[Yellow Box]	RIVERFRONT
[Grey Box]	Site and/or Parking Lot
[Black Box]	Deck or - Pool/Trough System
[Dark Brown Box]	Deck or - Pool/Trough System
[Light Brown Box]	Deck or - Pool/Trough System
[Dark Blue Box]	View Corridor
[Light Blue Box]	View Corridor
[Red Line]	Proposed Site



KUNZMAN ASSOCIATES, INC.

OVER 30 YEARS OF EXCELLENT SERVICE

4030d/5